# Meeting of the Central Valley Flood Protection Board May 29, 2015

#### **Staff Report**

Reclamation District No. 341 – Sherman Island Landside Levee Improvements and Modifications Sacramento County

#### <u>1.0 – ITEM</u>

Consider approval of Draft Permit No. 18985-1 (Attachment B).

#### 2.0 - APPLICANT

Reclamation District No. 341 (RD-341)

#### 3.0 - LOCATION

The project is located on the left (east) bank levee of the Sacramento River approximately 8-miles downstream from Rio Vista on Sherman Island. (Sacramento River, Sacramento County, see Attachment A).

#### 4.0 – PROJECT DESCRIPTION

The applicant proposes to construct landside levee improvements and realign a county road (West Sherman Island Road) on Sherman Island to accommodate two (2) new fish release stations referred to as Little Baja (18985-3) and Manzo Ranch (18985-2), near Sacramento River Miles 4 and 4.5 respectively. Work includes removal of existing gates and fencing, adding fill to the landside levee slope, widening the levee crown to approximately 50-feet at each fish release site to include asphalt-concrete operation pads that will facilitate truck access, asphalt paving of the levee crown between the two (2) fish release sites; construction of a 0.75-mile long, approximately 100-foot wide landside levee toe berm; and the trenching of four (4) 2-inch diameter and two (2) 4-inch diameter electrical conduits at each fish release site above the design water surface elevation.

#### 5.0 - AUTHORITY OF THE BOARD

California Water Code § 8534, 8590 – 8610.5, and 8700 - 8710

California Code of Regulations, Title 23 (Title 23)

- § 6, Need for a Permit
- § 13, Evidentiary Hearings
- § 112, Streams Regulated and Nonpermissible Work Periods
- § 120, Levees
- § 126, Fences and Gates

#### <u>6.0 – PROJECT ANALYSIS</u>

The California Department of Water Resources proposes to construct the Little Baja and Manzo Ranch fish release sites on Sherman Island to comply with the National Marine and Fisheries Services' (NMFS) Biological Opinion (BiOp) on the Long-term Operations of the Central Valley Project and State Water Project (2009) and the California Department of Fish and Wildlife's (CDFW) Longfin Smelt Incidental Take Permit (ITP) for the California State Water Project Delta Facilities and Operations (2009). Specific requirements are to reduce predation of salvaged fish at the fish release sites and increase salvaged fish survival rates.

Prior to the construction of the two (2) fish release sites, landside levee improvements and county road realignment will be required to accommodate the facilities. These improvements are being made due to concerns regarding levee stability, settlement, seepage, and safety. Improvements to address these concerns include widening the levee crown at the location of the fish release sites, placement of a landside seepage/stability berm, and the relocation of West Sherman Island Road to accommodate the two (2) fish release sites and to improve traffic safety. The seepage/stability berm will increase the factor-of-safety for levee stability and reduce seepage through the levee. DWR and RD 341 have collaborated on this project to ensure that all aspects of the entire project are compatible. All of the proposed landside levee improvements meet Title 23 standards.

#### 6.1 - Hydraulic Analysis

A hydraulic analysis was not required as there will be no work performed on the waterside of the levee.

#### 6.2 - Geotechnical Analysis

The significant geotechnical considerations for the project levee reach on Sherman Island are lack of erosion protection for waterside slopes, the presence of marsh soils below the levee, and ongoing settlement of the levee crest. The existing county road along the levee toe will need to be relocated to accommodate the levee rehabilitation and the two (2) new fish release stations. DWR borings from the 1950's described the levee as having landside slope failures and described the presence of numerous active seeps and slope failures. The levee has been widened and is not currently showing evidence of seepage or instability. Extensive filling was required for the USACE improvements shown in the 1954 plans. The fill may have overstressed the peat and caused failure of the levee slopes.

A seepage analysis was performed at six (6) locations along the project levee reach. Locations were chosen based on the surface and subsurface conditions encountered and are intended to represent the range of conditions along the reach. The computer program SEEP/W was used to perform the seepage analysis. The model is a two-dimension model that considers horizontal and vertical seepage in the model plane. The permeability values were estimated based on available data with similar soil conditions and the presumptive values presented in the Guidance Document for DWR Geotechnical Analyses (Revision 9, 2011).

The seepage analysis indicates that the exit gradients at the levee meet the USACE criteria for levees of 0.5 or lower. The use of flatter slopes and a toe berm will provide protection from high seepage through the levee and from slope movements. The levee on Sherman Island is founded on weak marsh deposits consisting predominately of peat and organic soil. To buttress the levee between Stations 700 and 745, where the landside slope is steeper and the island interior is lower than the remainder of the levee reach, it was determined that a 150 feet wide toe berm measured from the new landside hinge is needed to control seepage and to increase the factor of safety for slope stability. The landside levee slope will be inclined at 4H:1V.

A settlement analysis was performed to estimate settlement of the levee based on the theory of one-dimensional consolidation. The ground settles from a combination of primary consolidation and secondary compression (creep). Primary consolidation

occurs from compression of the marsh soils beginning when weight is placed on the soil. The initial weight is transferred to the water within the soil. The water builds up pressure causing flow to occur. As the water flows out of the soil, the soil structure compresses and continues to compress until the water flow is complete and the water pressure returns to hydro-static levels. Secondary compression is deformation without flow of water. With most soils, the amount of secondary compression is small relative to the primary consolidation and is not a concern. With peat, secondary compression is a significant phenomenon and is a primary cause of long-term settlement of Delta levees and their loss of freeboard. The secondary compression will continue for many years at a diminishing rate with time.

It is estimated that the toe berm and slopes will settle approximately 2 to 3 feet under the weight of 5 feet of new fill. The landside edge of the toe berm will settle about 1.0 to 1.3 feet under the weight of 2 feet of fill. The levee crown is expected to settle about 0.3 to 1.7 feet. The settlement estimates are expected within 10 years after fill placement. Design of the landside levee improvements have accounted for this anticipated settlement.

#### 7.0 – AGENCY COMMENTS AND ENDORSEMENTS

The comments and endorsements associated with this project, from all pertinent agencies are shown below:

- The local maintain agency for the project area is RD-341, the applicant for this project.
- The U.S. Army Corps of Engineers 208.10 comment letter <u>has not been</u> <u>received</u> for this application. Staff anticipates receipt of a letter indicating that the USACE District Engineer has no objection to the project, subject to conditions. Upon receipt of the letter, staff will review to ensure conformity with the permit language and incorporate it into the permit as Exhibit A.

#### 8.0 – CEQA ANALYSIS

Board staff has prepared the following CEQA findings:

The Board, as a responsible agency under CEQA, has reviewed an Initial Study/Mitigated Negative Declaration (IS/MND) (SCH Number: 2014052035, July 2014)

and Mitigation Monitoring and Reporting Program for the Sherman Island "Little Baja and Manzo Ranch" Fish Release Sites Project prepared by the lead agency, the Department of Water Resources. These documents, including project design, may be viewed or downloaded from the Central Valley Flood Protection Board website at <a href="http://www.cvfpb.ca.gov/meetings/2015/05-29-2015.cfm">http://www.cvfpb.ca.gov/meetings/2015/05-29-2015.cfm</a> under a link for this agenda item. These documents are also available for review in hard copy at the Board and the Department of Water Resources offices.

The Department of Water Resources determined that the project would not have a significant effect on the environment on November 10, 2014 and filed a Notice of Determination on November 12, 2014 with the State. Board staff finds that although the proposed project could have a potentially significant effect on the environment, there will not be a significant effect in this case because revisions in the project have been made by or agreed to by the project proponent. The project proponent has incorporated mandatory mitigation measures into the project plans to avoid identified impacts or to mitigate such impacts to a point where no significant impacts will occur. These mitigation measures are included in the project proponent's IS/MND and address impacts to air quality, biological resources and hazards and hazardous materials. The description of the mitigation measures are further described in the adopted IS/MND.

#### 9.0 - WATER CODE SECTION 8610.5 CONSIDERATIONS

 Evidence that the Board admits into its record from any party, State or local public agency, or nongovernmental organization with expertise in flood or flood plain management:

The Board has considered all the evidence presented in this matter, including the original and updated applications and supporting documents, this Staff Report and attachments, all other evidence presented by any individual or group, and all letters and other correspondence received by the Board and in the Board's files related to this matter.

The best available science that related to the scientific issues presented by the executive officer, legal counsel, the Department or other parties that raise credible scientific issues.

The accepted industry standards for the earth work proposed and water delivery channels under this permit as regulated by Title 23 have been applied to the review of this permit.

3. Effects of the decision on facilities of the State Plan of Flood Control, and consistency of the proposed project with the Central Valley Flood Protection Plan as adopted by Board Resolution 2012-25 on June 29, 2012.

The proposed project will improve the structural stability of the levee; therefore there will be no adverse effect on facilities of the State Plan of Flood Control. In addition the project is consistent with the 2012 Central Valley Flood Protection Plan.

The Delta Stewardship Council (DSC), and its authorizing statutes, requires that any actions in the Delta be consistent with the Delta Plan. DWR staff and DSC staff met to discuss the entire Sherman Island "Little Baja and Manzo Ranch" Fish Release Sites Project (Project) and it was determined that (under Step 3 of the DSC review process) the Project was not covered by any of the regulatory policies of the Delta Plan since the primary purpose of the Project was for releasing fish. It was further agreed that construction of the levee berm was an ancillary benefit to the Project and not the Project's primary purpose. Therefore, policies ER P4 (proposed actions that would construct or rehabilitate levees) and RR P1 (proposed actions that involve discretionary State investments in Delta flood risk management) would not be applicable to this Project.

4. Effects of reasonable projected future events, including, but not limited to, changes in hydrology, climate, and development within the applicable watershed:

There will be no effects of reasonable projected future events as the project will be located on the landside of the levee. Rising sea levels due to global warming may result in the need to raise the crown elevation of the levee.

#### 10.0 – STAFF RECOMMENDATION

Staff recommends that the Board:

#### Adopt:

the CEQA findings;

#### Approve:

draft Encroachment Permit No. 18985-1 in substantially the form provided; and

#### Direct:

 the Executive officer to take the necessary actions to execute the permit and file a Notice of Determination pursuant to CEQA with the State Clearinghouse.

Application No. 18985-1

#### 11.0 - LIST OF ATTACHMENTS

A. Location Maps and Photos

B. Draft Permit No. 18985-1

C. Project Drawings

D. Geotechnical Report

Design Review: Gary W. Lemon P.E.

Environmental Review: Andrea Buckley, Senior Environmental Scientist

Document Review: Mitra Emami P.E., Branch Chief - Permitting and Enforcement

Len Marino P.E., Chief Engineer

Legal Review: Nicole Rinke, Counsel

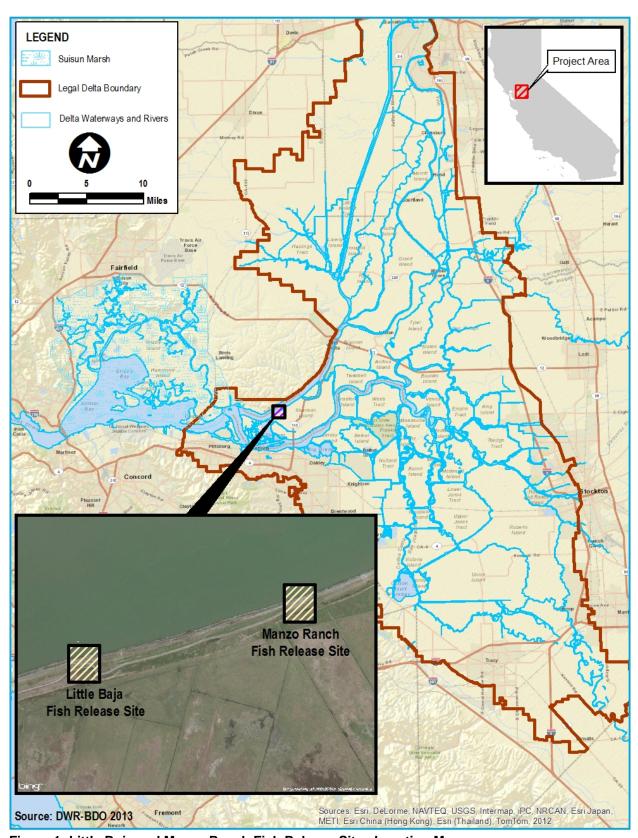
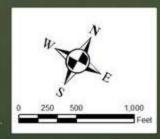
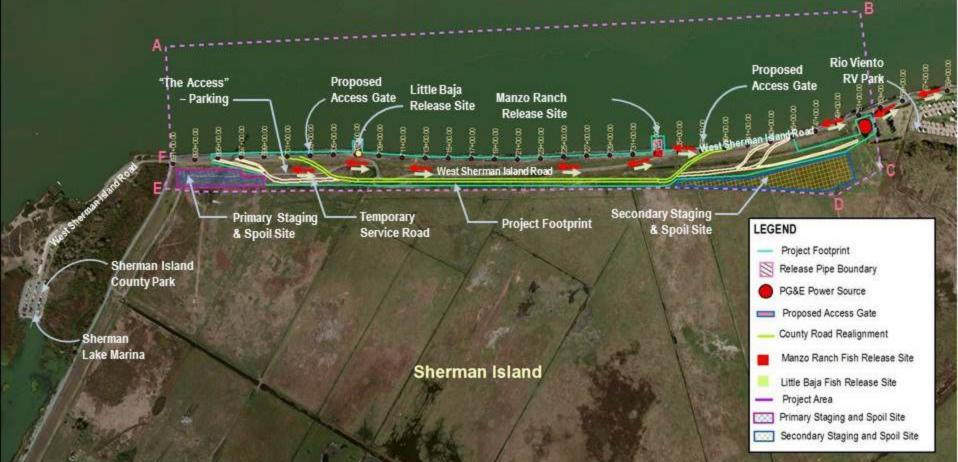


Figure 1: Little Baja and Manzo Ranch Fish Release Sites Location Map



# Sacramento River

Project Area Boundary Points					
point	Attach	honginde			
A	38° 3'47.90"N	121°47'10.36"W			
В	38° 4'19.61"N	121°46'6.95"W			
С	38° 4'8.68"N	121°45'56.45"W			
D	38° 4'4.75"N	121°45'59.44"W			
E	38° 3'37.16"N	121°47'3.41"W			
F	38° 3'39.55"N	121°47'4,21"W			



#### **DRAFT**

# STATE OF CALIFORNIA THE RESOURCES AGENCY

#### THE CENTRAL VALLEY FLOOD PROTECTION BOARD

PERMIT NO. 18985-1 BD

This Permit is issued to:

Reclamation District 341 (Sherman Island) 18419 Highway 160 Rio Vista, California 94571

To construct landside levee improvements and realign a county road (West Sherman Island Road) on Sherman Island to accommodate two (2) new fish release stations referred to as Manzo Ranch (18985-2) and Little Baja (18985-3), near Sacramento River Miles 4.5 and 4.0 respectively. Work includes removal and replacement of existing gates and fencing, adding fill to the landside levee slope, widening the levee crown to approximately 50-feet at each fish release site to include asphalt-concrete operation pads that will facilitate truck access, asphalt paving of the levee crown between the two (2) fish release sites; construction of a 0.75-mile long, approximately 100-foot wide landside levee toe berm; and the trenching of four (4) 2-inch diameter and two (2) 4-inch diameter electrical conduits at each fish release site above the design water surface elevation.

The project is located on the left (east) bank levee of the Sacramento River approximately 8-miles downstream from Rio Vista on Sherman Island (Section 28, 32, 33, T3N, R2E, MDB&M, Reclamation District 341, Sacramento River, Sacramento County).

NOTE: Special Conditions have been incorporated herein which may place limitations on and/or require modification of your proposed project as described above.

	(SEAL)			
Dated:				

Executive Officer

**GENERAL CONDITIONS:** 

#### **Attachment B**

**ONE**: This permit is issued under the provisions of Sections 8700 – 8723 of the Water Code.

**TWO**: Only work described in the subject application is authorized hereby.

THREE: This permit does not grant a right to use or construct works on land owned by the Sacramento and San Joaquin Drainage District or on any other land.

**FOUR**: The approved work shall be accomplished under the direction and supervision of the State Department of Water Resources, and the permittee shall conform to all requirements of the Department and The Central Valley Flood Protection Board.

**FIVE**: Unless the work herein contemplated shall have been commenced within one year after issuance of this permit, the Board reserves the right to change any conditions in this permit as may be consistent with current flood control standards and policies of The Central Valley Flood Protection Board.

**SIX**: This permit shall remain in effect until revoked. In the event any conditions in this permit are not complied with, it may be revoked on 15 days' notice.

**SEVEN**: It is understood and agreed to by the permittee that the start of any work under this permit shall constitute an acceptance of the conditions in this permit and an agreement to perform work in accordance therewith.

**EIGHT**: This permit does not establish any precedent with respect to any other application received by The Central Valley Flood Protection Board.

NINE: The permittee shall, when required by law, secure the written order or consent from all other public agencies having jurisdiction.

**TEN**: The permittee is responsible for all personal liability and property damage which may arise out of failure on the permittee's part to perform the obligations under this permit. If any claim of liability is made against the State of California, or any departments thereof, the United States of America, a local district or other maintaining agencies and the officers, agents or employees thereof, the permittee shall defend and shall hold each of them harmless from each claim.

**ELEVEN**: The permittee shall exercise reasonable care to operate and maintain any work authorized herein to preclude injury to or damage to any works necessary to any plan of flood control adopted by the Board or the Legislature, or interfere with the successful execution, functioning or operation of any plan of flood control adopted by the Board or the Legislature.

**TWELVE**: Should any of the work not conform to the conditions of this permit, the permittee, upon order of The Central Valley Flood Protection Board, shall in the manner prescribed by the Board be responsible for the cost and expense to remove, alter, relocate, or reconstruct all or any part of the work herein approved.

#### SPECIAL CONDITIONS FOR PERMIT NO. 18985-1 BD

#### LIABILITY AND INDEMNIFICATION

THIRTEEN: The permittee is responsible for all liability associated with construction, operation, and maintenance of the permitted facilities and shall defend, indemnify, and hold the Central Valley Flood Protection Board and the State of California; including its agencies, departments, boards, commissions, and their respective officers, agents, employees, successors and assigns (collectively, the "State"), safe and harmless, of and from all claims and damages arising from the project undertaken pursuant to this permit, all to the extent allowed by law. The State expressly reserves the right to supplement or take over its defense, in its sole discretion.

FOURTEEN: The permittee shall defend, indemnify, and hold the Central Valley Flood Protection Board and the State of California, including its agencies, departments, boards, commissions, and their respective officers, agents, employees, successors and assigns (collectively, the "State"), safe and harmless, of and from all claims and damages related to the Central Valley Flood Protection Board's approval of this permit, including but not limited to claims filed pursuant to the California Environmental Quality Act. The State expressly reserves the right to supplement or take over its

defense, in its sole discretion.

FIFTEEN: The Central Valley Flood Protection Board and Department of Water Resources shall not be held liable for any damages to the permitted encroachment(s) resulting from flood fight, operation, maintenance, inspection, or emergency repair.

#### **AGENCY CONDITIONS**

SIXTEEN: All work approved by this permit shall be in accordance with the submitted drawings and specifications except as modified by special permit conditions herein. No further work, other than that approved by this permit, shall be done in the area without prior approval of the Central Valley Flood Protection Board.

SEVENTEEN: Construction work of any type shall not be done on the levee or within the levee section during the flood season from November 1st to April 15th.

EIGHTEEN: The permittee shall be responsible for repair of any damages to the project levee and other flood control facilities due to construction, operation, or maintenance of the proposed project.

NINETEEN: The permittee shall con	nply with all conditions set forth in the	letter from the Department
of the Army (U.S. Army Corps of En	gineers, Sacramento District) dated _	, which is
attached to this permit as Exhibit	and is incorporated by reference.	

#### PRE-CONSTRUCTION

TWENTY: Upon receipt of a signed copy of the issued permit the permittee shall contact the Central Valley Flood Protection Board by telephone at (916) 574-0609, and submit the enclosed postcard, to schedule a preconstruction conference with the inspector that is assigned to your project. Failure to do so at least 10 working days prior to start of work may result in a delay of the project.

#### CONSTRUCTION

TWENTY-ONE: All cleared trees and brush shall be completely burned or removed from the project area and off all Project Works.

TWENTY-TWO: Prior to placement of fill for the toe berm and against the levee slope all surface vegetation shall be removed to a depth of 6 inches. Organic soil and roots larger than 1-1/2 inches in diameter shall be removed to a depth of 3 feet.

TWENTY-THREE: All fill material shall be imported impervious material with 20 percent or more passing the No. 200 sieve, a plasticity index of 8 or more, and a liquid limit of less than 50 and free of lumps or stones exceeding 3 inches in greatest dimension, vegetative matter, or other unsatisfactory material. Fill material shall be compacted in 4- to 6-inch layers to a minimum of 90 percent relative compaction as measured by ASTM Method D1557-91 or equivalent.

#### Attachment B

TWENTY-FOUR: Fill on the levee slope shall be keyed into the existing levee section with each lift.

TWENTY-FIVE: Positive drainage away from the levee toe shall be provided in all fill areas.

TWENTY-SIX: The invert of the electrical conduit through the levee section shall be above the design flood plane elevation of 8.7 feet, NGVD 29.

TWENTY-SEVEN: The electrical conduit shall be buried at least 12 inches below the landward levee slope.

TWENTY-EIGHT: Only impervious material shall be used for pipe bedding within the levee section.

#### **VEGETATION / ENVIRONMENTAL MITIGATION**

TWENTY-NINE: The mitigation measures approved by the CEQA lead agency and the permittee are found in its Mitigation and Monitoring Reporting Program (MMRP) adopted by the CEQA lead agency. The permittee shall implement all such mitigation measures.

#### POST-CONSTRUCTION

THIRTY: All debris generated by this project shall be disposed of outside the Project Works.

THIRTY-ONE: The project area shall be restored to at least the condition that existed prior to commencement of work.

THIRTY-TWO: The permittee shall replant or reseed the levee slopes to restore sod, grass, or other non-woody ground covers if damaged during project work.

THIRTY-THREE: Upon completion of the project, the permittee shall submit as-constructed drawings to: Department of Water Resources, Flood Project Inspection Section, 3310 El Camino Avenue, Suite 256, Sacramento, California 95821.

#### **OPERATIONS AND MAINTENANCE**

THIRTY-FOUR: The permittee shall maintain the permitted encroachment(s) and the project works within the utilized area in the manner required and as requested by the authorized representative of the Department of Water Resources or any other agency responsible for maintenance.

THIRTY-FIVE: The permitted encroachment(s) shall not interfere with operation and maintenance of the flood control project. If the permitted encroachment(s) are determined by any agency responsible for operation or maintenance of the flood control project to interfere, the permittee shall be required, at permittee's cost and expense, to modify or remove the permitted encroachment(s) under direction of the Central Valley Flood Protection Board or Department of Water Resources. If the permittee does not comply, the Central Valley Flood Protection Board may modify or remove the encroachment(s) at the permittee's expense.

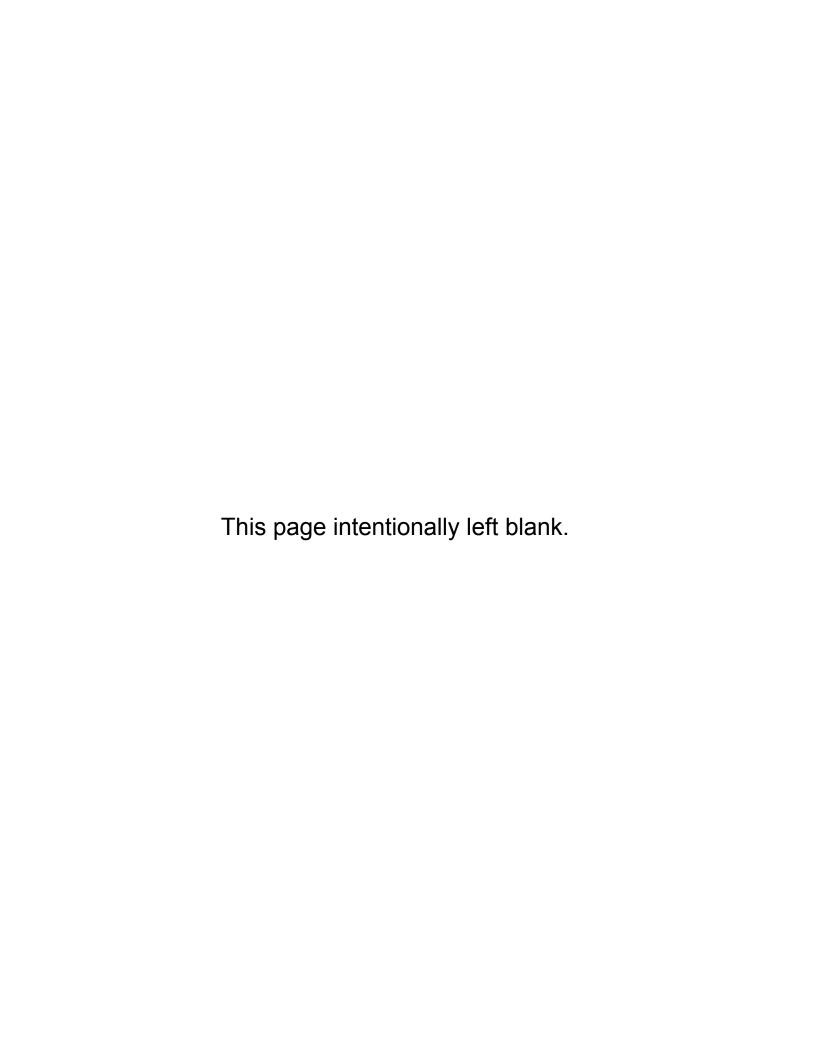
#### **Attachment B**

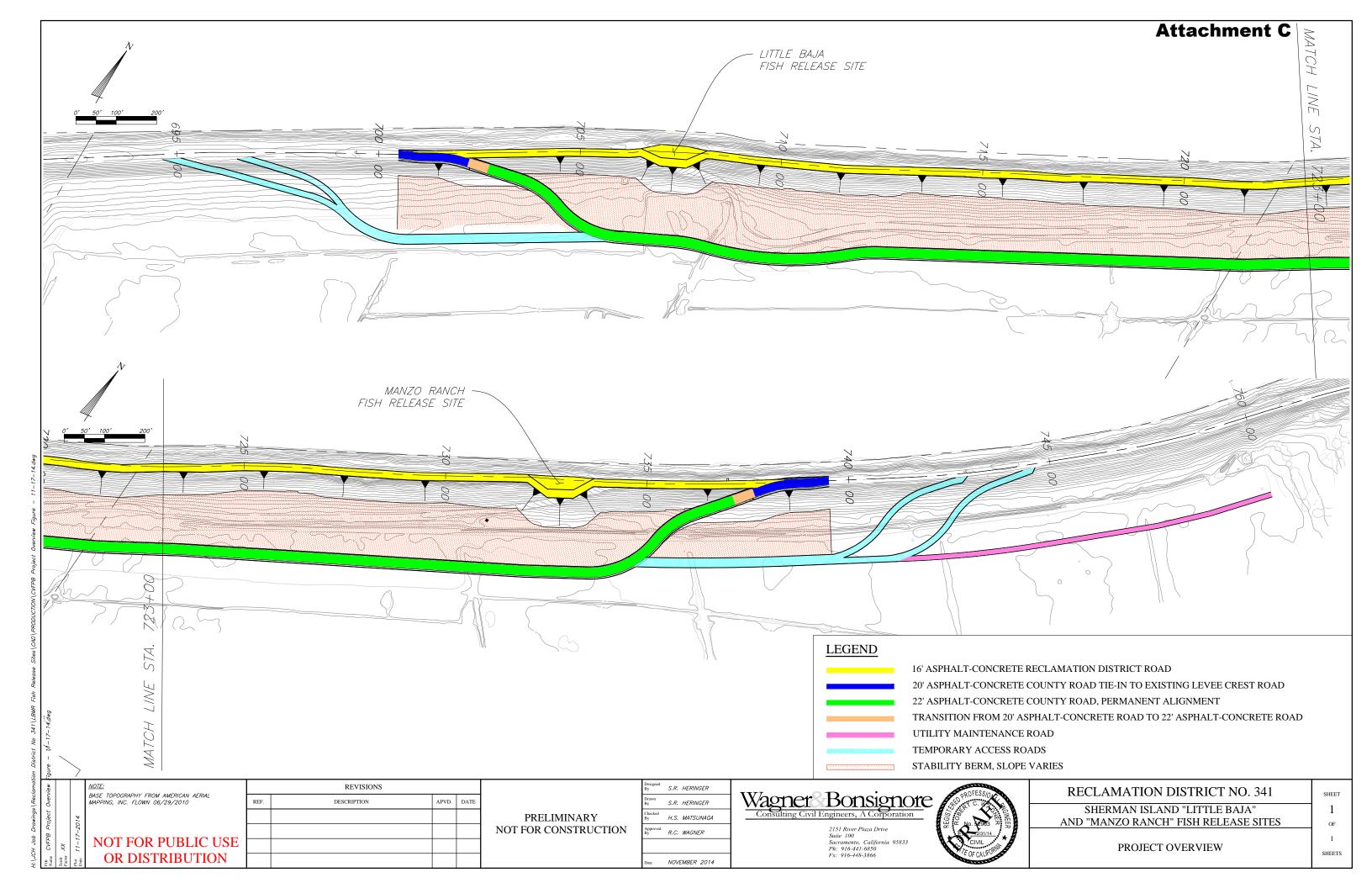
#### PROJECT ABANDONMENT / CHANGE IN PLAN OF FLOOD CONTROL

THIRTY-SIX: If the project, or any portion thereof, is to be abandoned in the future, the permittee or successor shall abandon the project under direction of the Central Valley Flood Protection Board and Department of Water Resources, at the permittee's or successor's cost and expense.

THIRTY-SEVEN: The permittee may be required, at permittee's cost and expense, to remove, alter, relocate, or reconstruct all or any part of the permitted encroachment(s) if removal, alteration, relocation, or reconstruction is necessary as part of or in conjunction with any present or future flood control plan or project or if damaged by any cause. If the permittee does not comply, the Central Valley Flood Protection Board may remove the encroachment(s) at the permittee's expense.

#### **END OF CONDITIONS**







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# RECLAMATION DISTRICT NO. 341

# SHERMAN ISLAND "LITTLE BAJA" AND "MANZO RANCH" FISH RELEASE SITES

**Sacramento County** 

California

WAGNER & BONSIGNORE • CONSULTING CIVIL ENGINEERS • A CORPORATION

NOT FOR PUBLIC USE

2151 RIVER PLAZA DRIVE, SUITE 100 • SACRAMENTO, CALIFORNIA

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OR DISTRIBUTION

REVISIONS APVD. DATE NOT FOR PUBLIC USE

PRELIMINARY NOT FOR CONSTRUCTION

Designed By	S.R. HERINGER		
Drawn By	S.R. HERINGER		
Checked By	H.S. MATSUNAGA		
Approved By	R.C. WAGNER		
Date	NOVEMBER 2014		



RECLAMATION DISTRICT NO. 34	RECLA	LAMA	ATION	DIS'	TRICT	NO.	341
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SHERMAN ISLAND "LITTLE BAJA" AND "MANZO RANCH" FISH RELEASE SITES

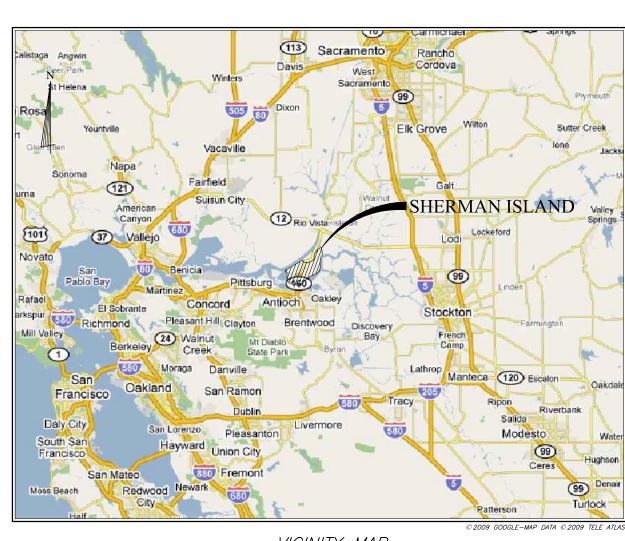
Attachment C

SHEET INDEX

SHEET OF

45 SHEETS

### Attachment C



<u>VICINITY MAP</u> N.T.S.

LOCATION MAP
0' 1000' 2000' 4000'

PROJECT LOCATION

			REVISIONS			
		REF.	DESCRIPTION	APVD.	DATE	
+ 10						
7/47	NOT FOR BUILDING LICE					ľ
///	NOT FOR PUBLIC USE					
ate	OR DISTRIBUTION					1

BASE MAP PER USGS 7.5 MINUTE QUADRANGLE MAP FOR ANTIOCH NORTH AND JERSEY ISLAND, CALIFORNIA

6660

620

PRELIMINARY NOT FOR CONSTRUCTION

Designed By	S.R. HERINGER	١.	
Drawn By	S.R. HERINGER		
Checked By	H.S. MATSUNAGA		
Approved By	R.C. WAGNER		
Date	NOVEMBER 2014		

150

002

078

220 + 00

00

Pumping Station

00

00



2151 River Plaza Drive Suite 100 Sacramento, California 95833 Ph: 916-441-6850 Fx: 916-779-3120

AD ROFESSIO	
No. 12903 RE RE 2007	H
CIVIL	
JE OF CALIFORNIA	

RECLAMATION DISTRICT NO. 341

SHERMAN ISLAND "LITTLE BAJA"

AND "MANZO RANCH" FISH RELEASE SITES

LOCATION MAP AND VICINITY MAP

SHEET

2

45

SHEETS

Attac	hmen	t (	

Section   Communication   Co		Levee Eml	hankment		Stability Toe Be	erm	New County Road	Utility Easement Road	Misc.
State   Relaxer   Lampa   Companies   Companies   Companies   Relaxer   Lampa   December & Companies   Companies									
Carbon   C	Station								Construction Reference Line to
1			Embankment Control Point				CL of New County Road		Waterside Hinge of Existing Levee
1999	694+00	-	-		-	-	-	-	20.5 *
99-90	695+00	-	-		-	-	-	-	
	696+00	-	-		-	-	-	-	26.4 *
Color	697+00	-	-		-	-	-	-	27.0 *
Match String Leve	698+00	-	-		-	-	-	-	25.6 *
Table   Tabl	699+00	-	-		-	-	-	-	22.8 *
1930   120   1318   138   160   160   173   160   173   160   173   160   173   17	700+00	Match Existing Levee	Match Existing Levee		50	-	-	-	15.8 *
133-00	701+00	21.4	13.8		53	167	5	-	11.7 *
194-00   120	702+00	32.7	13.8		58	160	16	-	10.6 *
135-90   12.0	703+00	12.0	13.8		59	160	49	-	15.9 *
135-00   12.0   11.8	704+00	12.0	13.8		68	160	92	-	12.2 *
196-10   12.0   13.8   6.2   156   209   .   150°   78.70°   78.	705+00	12.0	13.8				178	-	10.1 *
1975-00								-	19.0 *
198-0    193								-	
199-00   120   128   75   160   217								_	
10-10   12-0   11-8   10-8   10-8   180   190   21-1   1-   17-3   18-8   17-3   17-3   18-8   17-3   17-3   18-8   17-3   18-8   17-3   18-8   17-3   18-8   17-3   18-8   17-3   18-8   17-3   18-8   17-3   18-8   17-3   18-9   17-3   18-9   17-3   18-8   17-3   18-9   18-8   17-3   18-9   18-8   17-3   18-9   18-8   18-9   18-8   18-9   18-									
171-00   120									
12-06   12-06   13-8   11-16   16-06   20-56   17-9   17									
134-00   12.0									
11-10									
15.50									
11-00   12-0									
177-00   16.0   118.8   116   164   205   .   6.7°   178-00   12.0   13.8   112   160   204   .   .   11.6°   179-00   12.0   13.8   100   160   203   .   8.8°   179-00   12.0   13.8   100   160   203   .   .   8.8°   179-00   12.0   13.8   100   160   203   .   .   .   .   .   .   .   .   .									
18-90   12.0   13.8   112   190   204   .   11.6*   18-7   19-90   12.0   13.8   102   160   203   .     8.8*   19-90   12.0   13.8   100   160   203   .     9.0*   19.0*									
199-00   12.0   13.8   10.0   160   20.3   -									
120									
10									
123+00									
123-400									
12.00   12.00   13.8   94   160   217   - 8.66°     72600   12.0   13.8   92   160   220   - 10.5°     727-00   12.0   13.8   105   160   220   - 9.0°     728+00   12.0   13.8   99   160   219   - 9.0°     728+00   12.0   13.8   99   160   219   - 9.0°     729-00   13.0   13.8   93   161   219   - 7.3°     739-00   12.0   13.8   85   160   222   - 10.0°     731-00   12.0   13.8   103   160   227   - 10.0°     731-00   12.0   13.8   103   160   227   - 10.0°     731-00   13.0   13.8   103   160   227   - 10.0°     731-00   13.0   13.8   103   160   95   232   - 7.2°     733+00   45.4   16.3   110   193   228   - 110.0°     735-00   13.0   13.8   89   168   222   - 126°     735-00   12.0   13.8   81   160   168   195   15.6°     735-00   12.0   13.8   81   160   80   195   15.6°     736-00   12.0   13.8   81   160   80   195   15.6°     736-00   12.0   13.8   72   160   45   195   15.6°     738-00   28.5   13.8   98   160   111   195   19.0°     738-00   28.5   13.8   98   160   111   195   19.0°     738-00   19.8   13.8   98   160   111   195   19.0°     738-00   19.8   13.8   99   165   165   3   195   13.8°     740-00   100   100   100   100   11.5°     741-00   -									
T25400   12.0   13.8   92   160   220     10.5 °									
127-00   12.0   13.8   105   160   220   -   9.0 °     728-00   12.0   13.8   99   160   219   -   9.0 °     729-00   13.0   13.8   99   161   219   -   7.3 °     739-00   12.0   13.8   85   160   222   -   100 °     731-00   12.0   13.8   85   160   222   -   100 °     731-00   12.0   13.8   103   160   227   -   100 °     731-00   13.0   13.8   106   195   232   -   7.2 °     733-00   13.0   13.8   106   195   232   -   11.0 °     734-00   13.0   13.8   89   168   222   -   11.0 °     734-00   13.0   13.8   89   168   222   -   12.6 °     734-00   13.0   13.8   81   160   168   195   15.6 °     734-00   12.0   13.8   81   160   80   195   15.6 °     734-00   12.0   13.8   81   160   80   195   15.6 °     734-00   12.0   13.8   81   160   80   195   15.6 °     738-00   28.5   13.8   98   160   11   195   19.0 °     739-00   19.8   13.8   98   160   11   195   19.0 °     739-00   19.8   13.8   96   165   3   195   13.8 °     740-00   19.8   13.8   96   165   3   195   13.8 °     740-00   19.8   13.8   96   165   3   195   13.8 °     740-00   19.8   13.8   96   165   3   3   195   13.8 °     740-00   19.8   13.8   96   165   3   195   13.8 °     740-00   19.8   13.8   96   165   3   195   13.8 °     740-00   19.8   13.8   96   165   3   195   13.8 °     740-00   19.8   13.8   96   165   3   195   13.8 °     740-00   19.8   13.8   96   165   3   195   13.8 °     740-00   19.8   13.8   96   165   3   195   13.8 °     740-00   19.8   13.8   96   165   3   195   13.8 °     740-00   19.8   13.8   96   165   165   3   195   13.8 °     740-00   19.8   13.8   13.8   96   165									
\$\frac{728+00}{729+00}									
T29+00									
730+00   12.0   13.8   8.5   160   2222   -   10.0 °   731+00   12.0   13.8   103   160   2277   -   10.0 °   72 °   72 °   72 °   72 °   72 °   72 °   72 °   72 °   72 °   72 °   72 °   72 °   72 °   72 °   72 °   72 °   72 °   72 °   72 °   73 °   72 °   73 °   72 °   73									
731+00   12.0   13.8   103   160   227     10.0 °     732+00   13.0   13.8   106   195   232     7.2 °     733+00   45.4   16.3   1110   193   228     11.0 °     734+00   13.0   13.8   89   168   222     12.6 °     735+00   12.0   13.8   81   160   168   195   15.6 °     736+00   12.0   13.8   81   160   80   195   15.6 °     737+00   12.0   13.8   81   160   80   195   13.6 °     737+00   12.0   13.8   72   160   45   195   15.2 °     738+00   28.5   13.8   98   160   11   195   190 °     739+00   19.8   13.8   98   160   11   195   190 °     739+00   19.8   13.8   96   165   3   195   13.8 °     740+00   Match Existing Levee   Match Existing Levee   94   .   .   .   200   11.5 °     741+00   -								-	
732+00         13.0         13.8         106         195         232         -         7.2 *           733+00         45.4         16.3         110         193         228         -         11.0 *           734+00         13.0         13.8         89         168         222         -         12.6 *           735+00         12.0         13.8         81         160         168         195         15.6 *           737+00         12.0         13.8         81         160         80         195         13.6 *           737+00         12.0         13.8         81         160         45         195         15.6 *           738+00         12.0         13.8         98         160         45         195         15.2 *           738+00         12.0         13.8         98         160         11         195         19.2 *           739+00         12.8         13.8         98         160         11         195         13.8 *           740+00         19.8         13.8         98         165         3         195         13.8 *           741+00         -         -         -         -								-	
733+00         45.4         16.3         110         193         228         -         11.0 *           734+00         13.0         13.8         89         168         222         -         12.6 *           735+00         12.0         13.8         81         160         168         195         15.6 *           736+00         12.0         13.8         81         160         80         195         13.6 *           737+00         12.0         13.8         72         160         45         195         15.2 *           738+00         12.0         13.8         98         160         11         195         19.0 *           738+00         12.0         13.8         98         160         11         195         19.0 *           738+00         12.8         13.8         98         160         11         195         19.0 *           739+00         19.8         13.8         98         160         11         195         19.0 *           749+00         19.8         13.8         96         165         3         195         13.8 *           740+00         -         -         -         -								-	
734+00         13.0         13.8         89         168         222         -         12.6 °           735+00         12.0         13.8         81         160         168         195         15.6 °           736+00         12.0         13.8         81         160         80         195         13.6 °           737+00         12.0         13.8         81         160         45         195         15.2 °           738+00         28.5         13.8         98         160         11         195         19.0 °           739+00         19.8         13.8         96         165         3         195         13.8 °           740+00         Match Existing Leve         94         -         -         200         11.5 °           741+00         -         -         -         -         -         -         200         17.6 °           742+00         -         -         -         -         -         -         200         15.3 °           744+00         -         -         -         -         -         200         15.3 °           744+00         -         -         -         -									
735+00         12.0         13.8         81         160         168         195         15.6 *           736+00         12.0         13.8         81         160         80         195         13.6 *           737+00         12.0         13.8         72         160         45         195         15.2 *           738+00         28.5         13.8         98         160         11         195         19.9 *           739+00         19.8         13.8         96         165         3         195         13.8 *           740+00         Match Existing Levee         Match Existing Levee         94         -         -         200         11.5 *           741+00         -         -         -         -         200         17.6 *           742+00         -         -         -         -         200         15.3 *           744+00         -         -         -         -         200         15.3 *           744+00         -         -         -         -         -         200         13.6 *           745+00         -         -         -         -         -         200         13.9 *									
736+00         12.0         13.8         81         160         80         195         13.6 *           737+00         12.0         13.8         72         160         45         195         15.2 *           738+00         28.5         13.8         98         160         11         195         19.0 *           739+00         19.8         13.8         96         165         3         195         13.8 *           740+00         Match Existing Levee         Match Existing Levee         94         -         -         200         11.5 *           741+00         -         -         -         200         17.6 *         17.6 *           742+00         -         -         -         -         200         12.6 *           743+00         -         -         -         -         200         15.3 *           744+00         -         -         -         -         200         13.6 *           745+00         -         -         -         -         200         13.9 *           747+00         -         -         -         -         200         12.9 *           748+00         - <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>									
737+00         12.0         13.8         72         160         45         195         15.2 *           738+00         28.5         13.8         98         160         11         195         19.0 *           739+00         19.8         13.8         96         165         3         195         13.8 *           740+00         Match Existing Leve         Match Existing Leve         94         -         -         200         11.5 *           741+00         -         -         -         -         200         17.6 *           742+00         -         -         -         -         200         17.6 *           743+00         -         -         -         -         200         15.3 *           744+00         -         -         -         -         200         13.6 *           745+00         -         -         -         -         200         14.2 *           747+00         -         -         -         -         200         13.9 *           748+00         -         -         -         -         200         11.1 *           749+00         -         -         -									
738+00         28.5         13.8         98         160         11         195         19.0 *           739+00         19.8         13.8         96         165         3         195         13.8 *           740+00         Match Existing Leve         Match Existing Leve         94         -         -         200         11.5 *           741+00         -         -         -         -         200         17.6 *           742+00         -         -         -         -         200         12.6 *           743+00         -         -         -         -         200         15.3 *           744+00         -         -         -         -         200         13.6 *           745+00         -         -         -         -         200         13.6 *           745+00         -         -         -         -         200         13.9 *           747+00         -         -         -         -         200         13.9 *           748+00         -         -         -         -         200         11.1 *           749+00         -         -         -         -         200 <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>									
739+00         19.8         13.8         96         165         3         195         13.8 *           740+00         Match Existing Levee         Match Existing Levee         94         -         -         200         11.5 *           741+00         -         -         -         -         200         17.6 *           742+00         -         -         -         200         12.6 *           743+00         -         -         -         200         15.3 *           744+00         -         -         200         15.3 *           745+00         -         -         200         13.6 *           745+00         -         -         200         13.9 *           747+00         -         -         -         200         12.9 *           748+00         -         -         -         200         11.1 *           749+00         -         -         -         200         11.1 *           750+00         -         -         -         200         10.3 *									
740+00         Match Existing Levee         Match Existing Levee         94         -         -         200         11.5 *           741+00         -         -         -         -         200         17.6 *           742+00         -         -         -         -         200         12.6 *           743+00         -         -         -         -         200         15.3 *           744+00         -         -         -         -         200         13.6 *           745+00         -         -         -         -         200         14.2 *           746+00         -         -         -         -         200         13.9 *           747+00         -         -         -         -         200         12.9 *           748+00         -         -         -         -         200         11.1 *           749+00         -         -         -         -         200         61.*           750+00         -         -         -         -         200         10.3 *									
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$									
742+00         -         -         200         12.6 *           743+00         -         -         200         15.3 *           744+00         -         -         200         13.6 *           745+00         -         -         200         14.2 *           746+00         -         -         200         13.9 *           747+00         -         -         200         12.9 *           748+00         -         -         200         11.1 *           749+00         -         -         200         61.*           750+00         -         -         -         200         10.3 *		Match Existing Levee	Match Existing Levee		94	-	-		
743+00         -         -         200         15.3 *           744+00         -         -         200         13.6 *           745+00         -         -         200         14.2 *           746+00         -         -         200         13.9 *           747+00         -         -         200         12.9 *           748+00         -         -         200         11.1 *           749+00         -         -         200         6.1 *           750+00         -         -         200         10.3 *		-	-		-	-	-		
744+00       -       -       200       13.6 *         745+00       -       -       200       14.2 *         746+00       -       -       200       13.9 *         747+00       -       -       200       12.9 *         748+00       -       -       200       11.1 *         749+00       -       -       200       6.1 *         750+00       -       -       200       10.3 *		-	-		-	-	-		
745+00         -         -         200         14.2 *           746+00         -         -         200         13.9 *           747+00         -         -         200         12.9 *           748+00         -         -         200         11.1 *           749+00         -         -         200         6.1 *           750+00         -         -         200         10.3 *		-	-		-	-	-		
746+00         -         -         200         13.9 *           747+00         -         -         200         12.9 *           748+00         -         -         -         200         11.1 *           749+00         -         -         -         200         6.1 *           750+00         -         -         -         200         10.3 *	744+00	-	-		-	-	-		
747+00         -         -         200         12.9 *           748+00         -         -         -         200         11.1 *           749+00         -         -         -         200         6.1 *           750+00         -         -         200         10.3 *	745+00	-	-		-	-	-		
748+00         -         -         200         11.1 *           749+00         -         -         -         200         6.1 *           750+00         -         -         -         200         10.3 *	746+00	-	-		-	-	-		
749+00         -         -         -         200         6.1 *           750+00         -         -         -         200         10.3 *	747+00	-	-		-	-	-		
750+00 200 10.3 *	748+00	-			-	-	-	200	
	749+00	-	-		-	-	-	200	
PROJECT CONTROL	750+00				-	-	-	200	10.3 *
						PROJECT CONTR	<u></u>		* WATER

\* WATERSIDE LIMIT OF CONSTRUCTION. FIELD VERIFY LOCATION PRIOR TO CONSTRUCTION ACTIVITIES.

	REVISIONS				
	REF.	DESCRIPTION	APVD.	DATE	
NOT FOR BURLICIUSE					
NOT FOR PUBLIC USE					
OR DISTRIBUTION					

PRELIMINARY NOT FOR CONSTRUCTION

	Designed By	S.R. HERINGER
	Drawn By	S.R. HERINGER
	Checked By	H.S. MATSUNAGA
1	Approved By	R.C. WAGNER

NOVEMBER 2014

Wagner Bonsignore
Consulting Civil Engineers, A Corporation



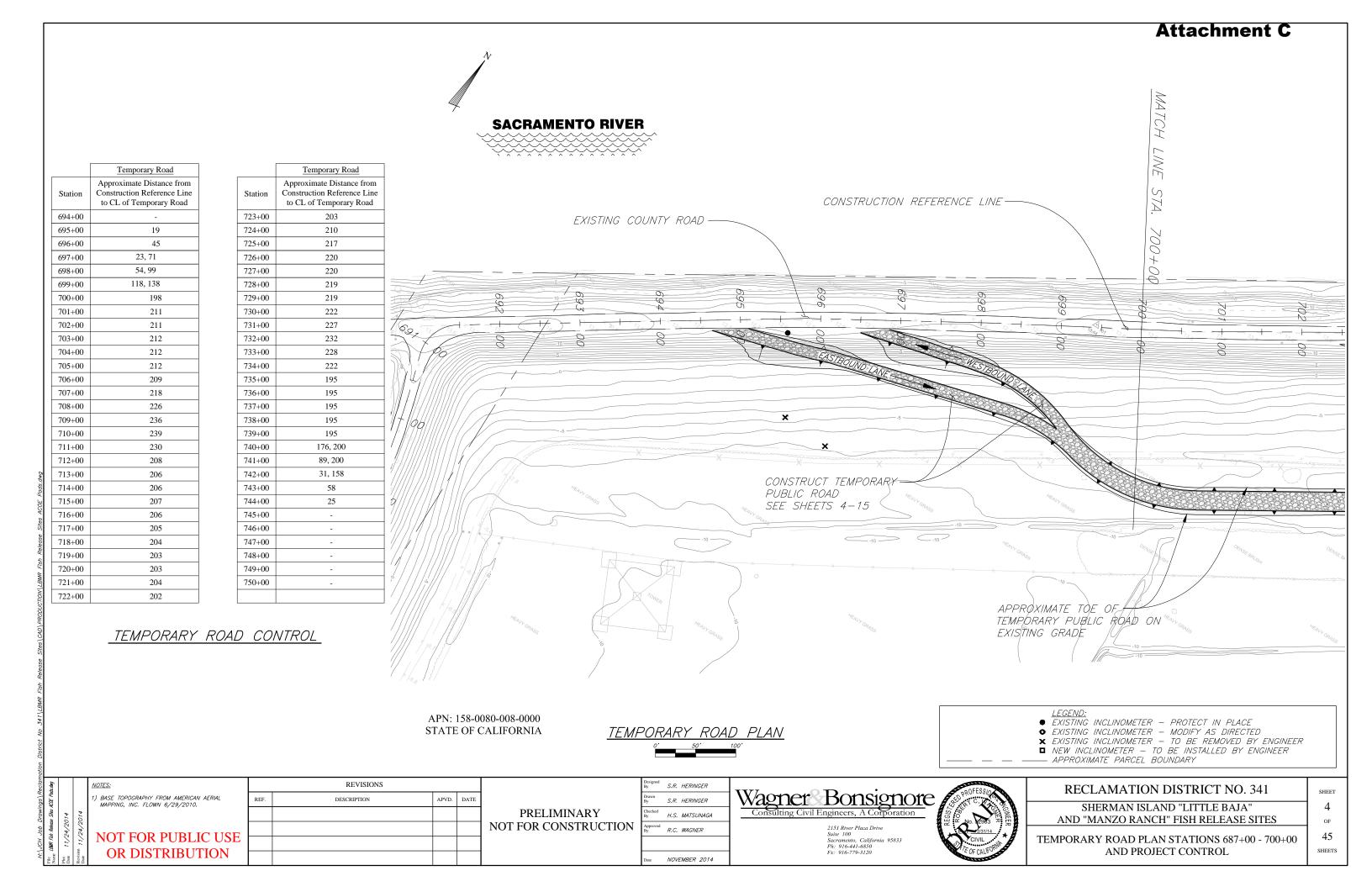
**RECLAMATION DISTRICT NO. 341** 

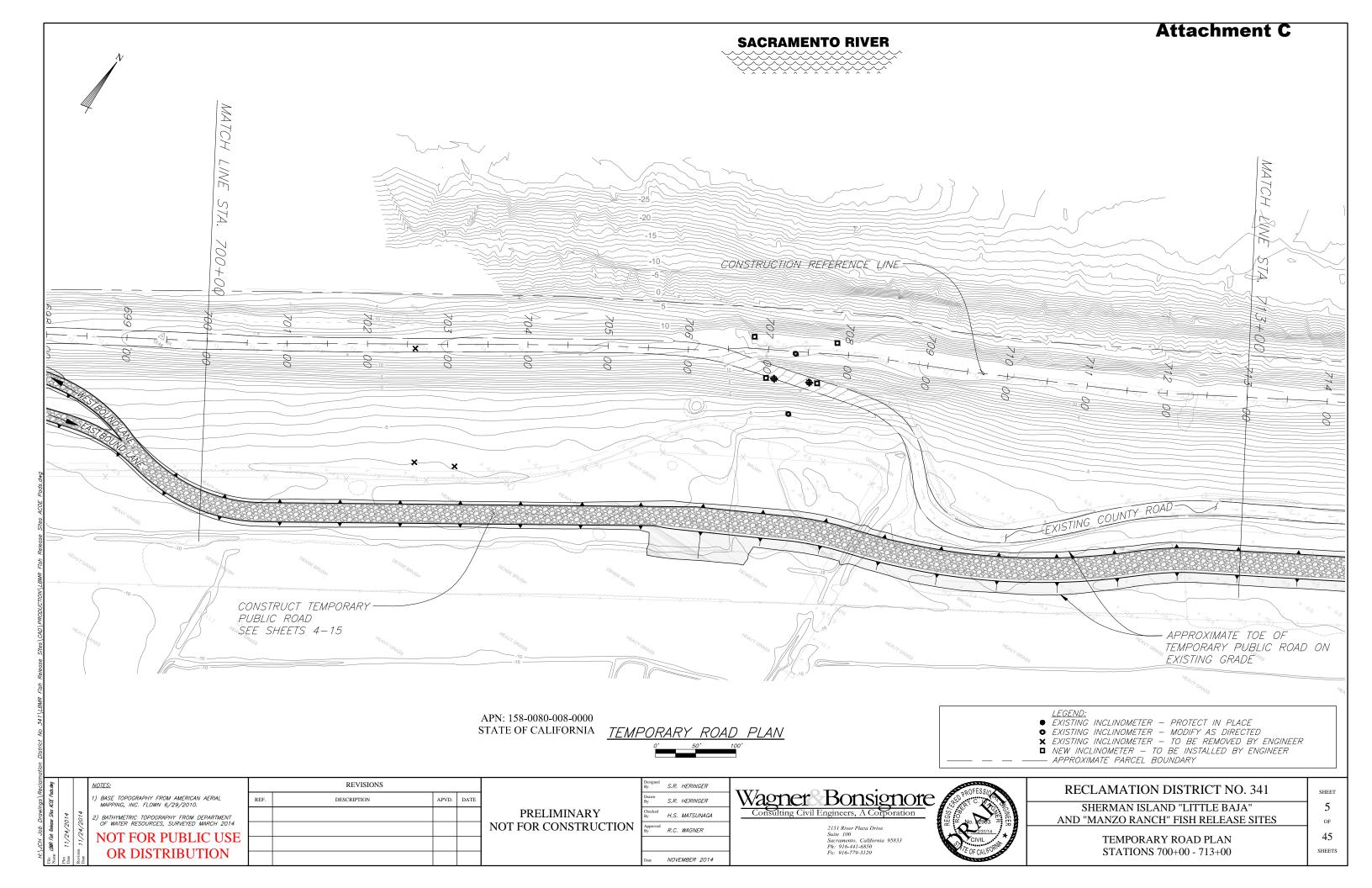
SHERMAN ISLAND "LITTLE BAJA" AND "MANZO RANCH" FISH RELEASE SITES

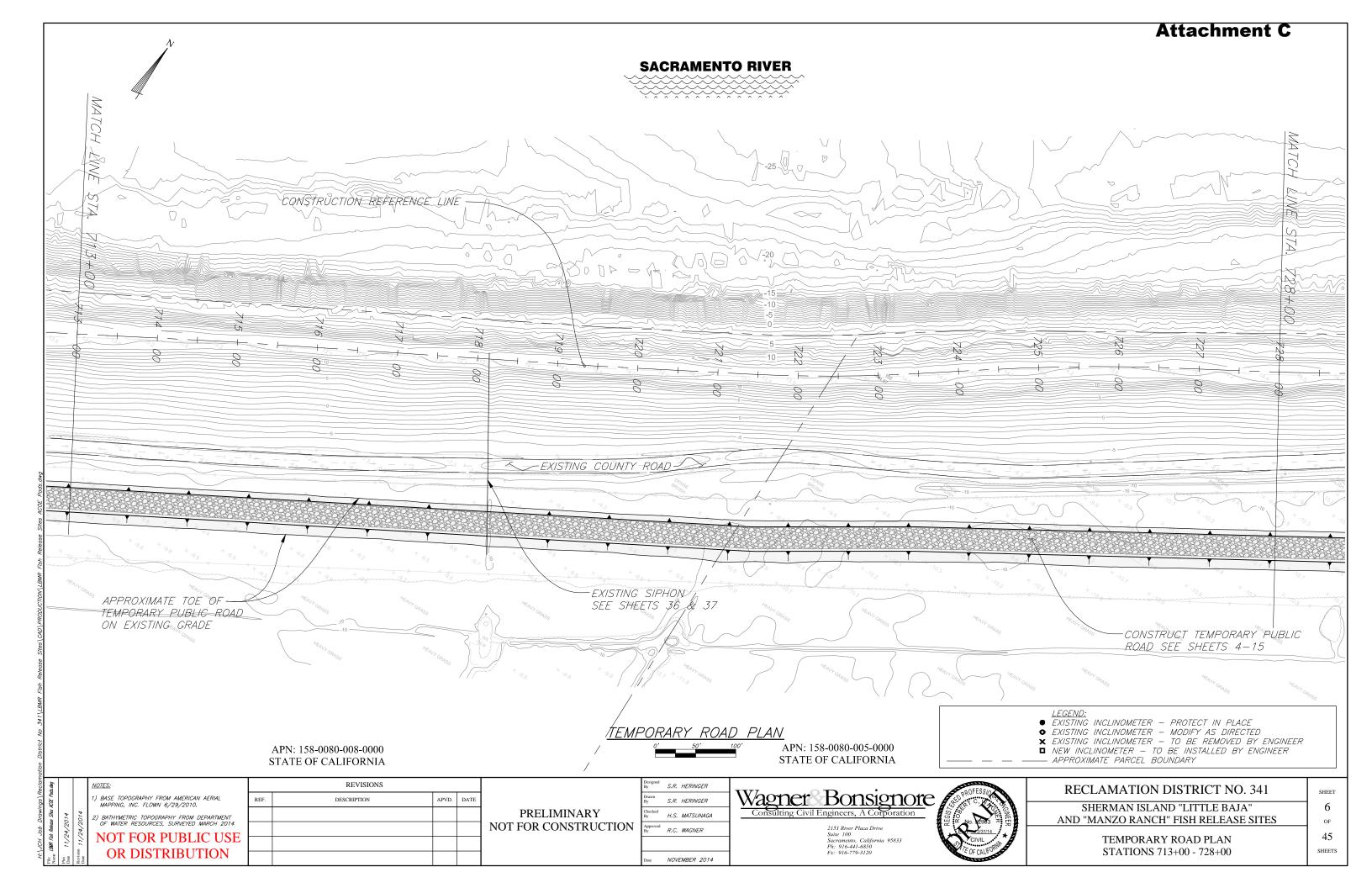
PROJECT CONTROL

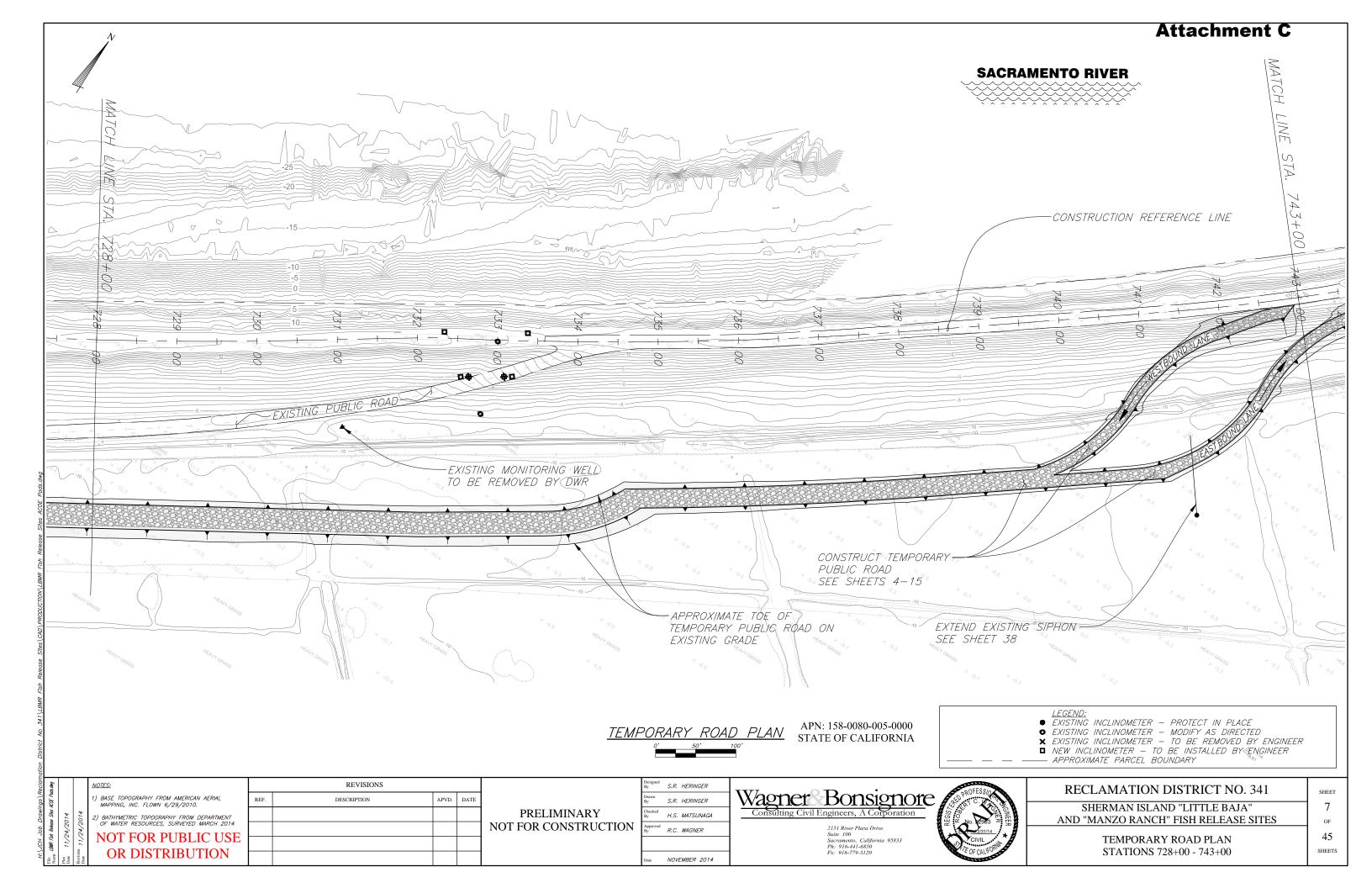
SHEET 3

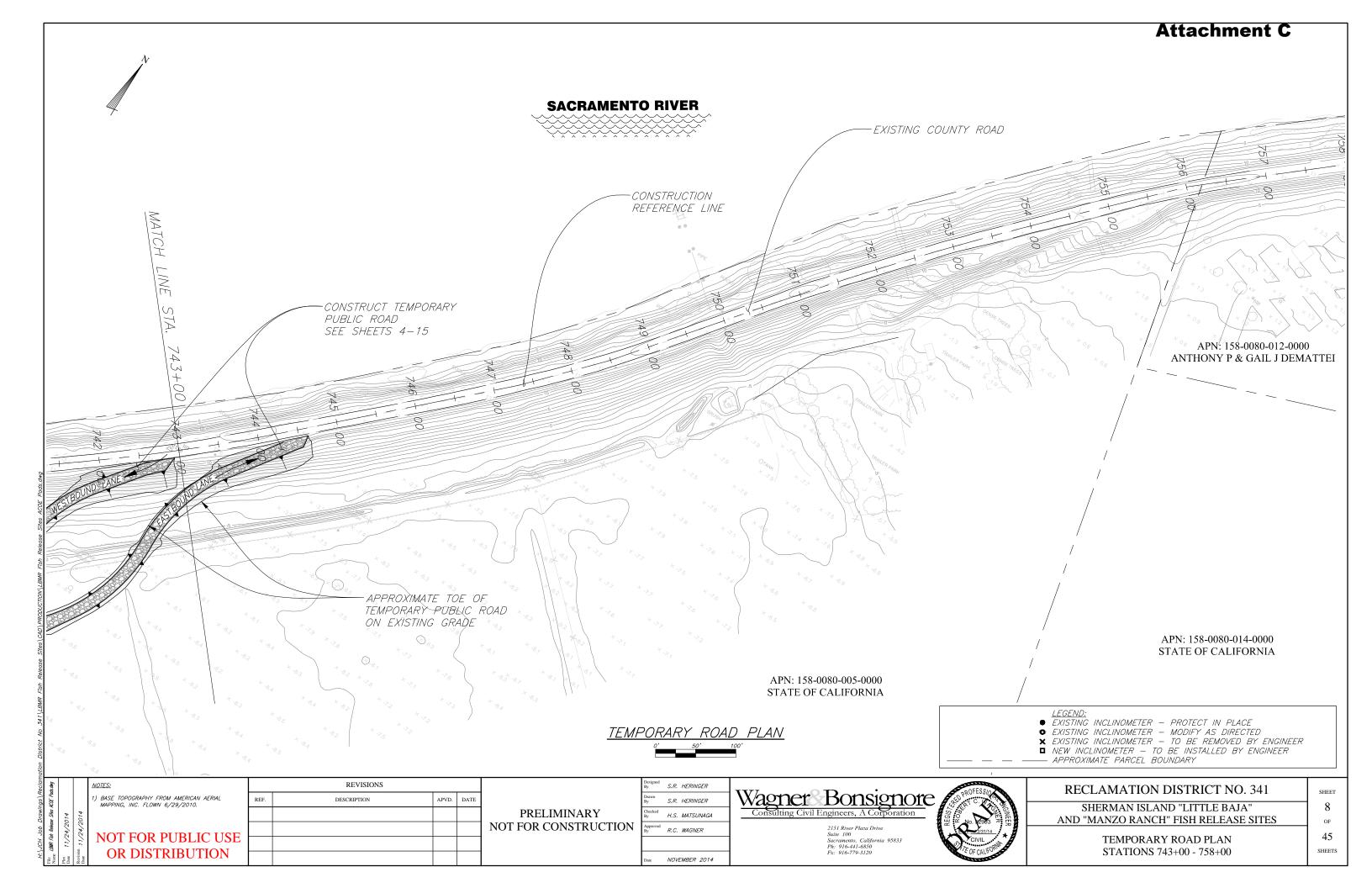
45 SHEETS

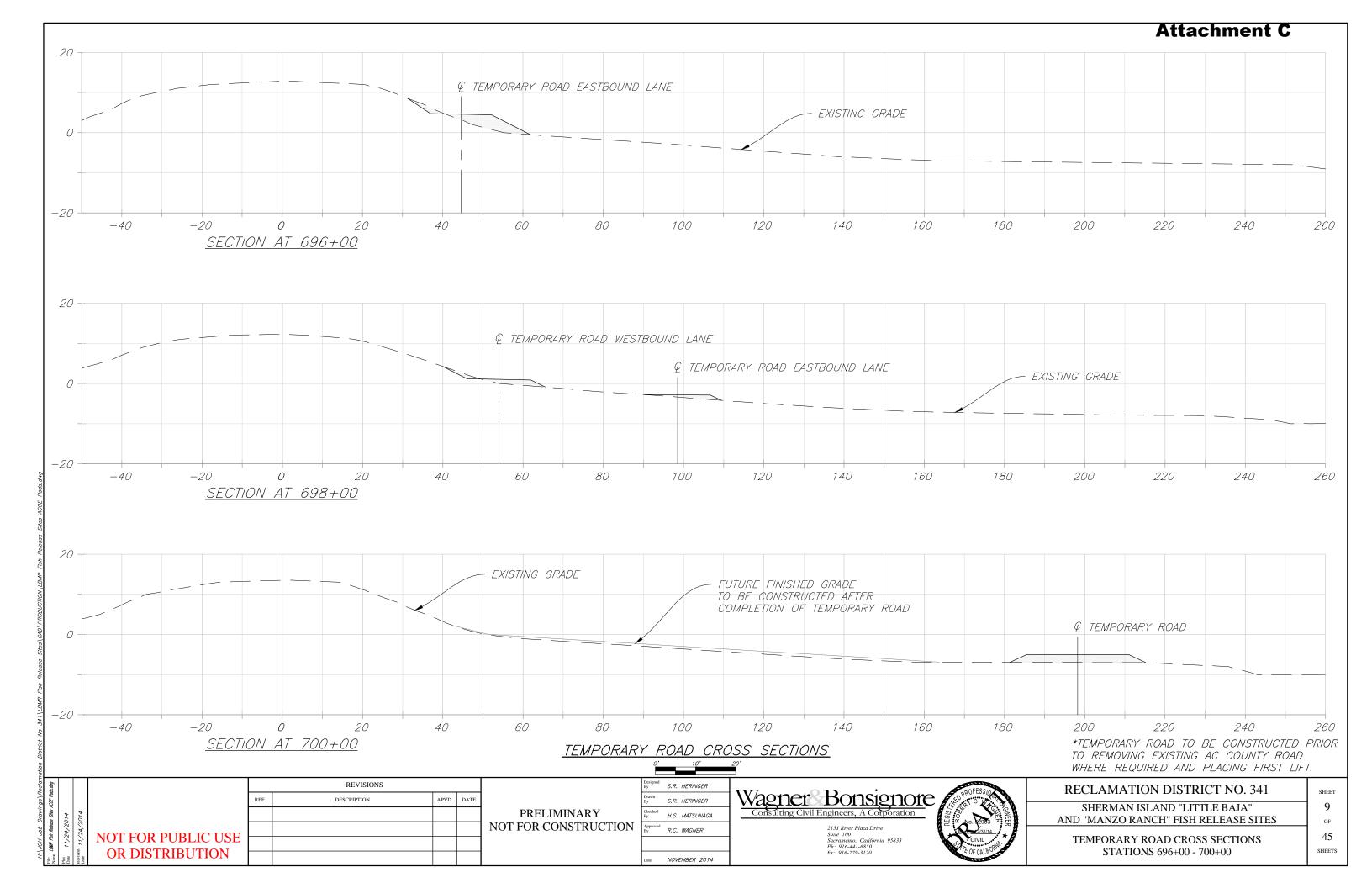


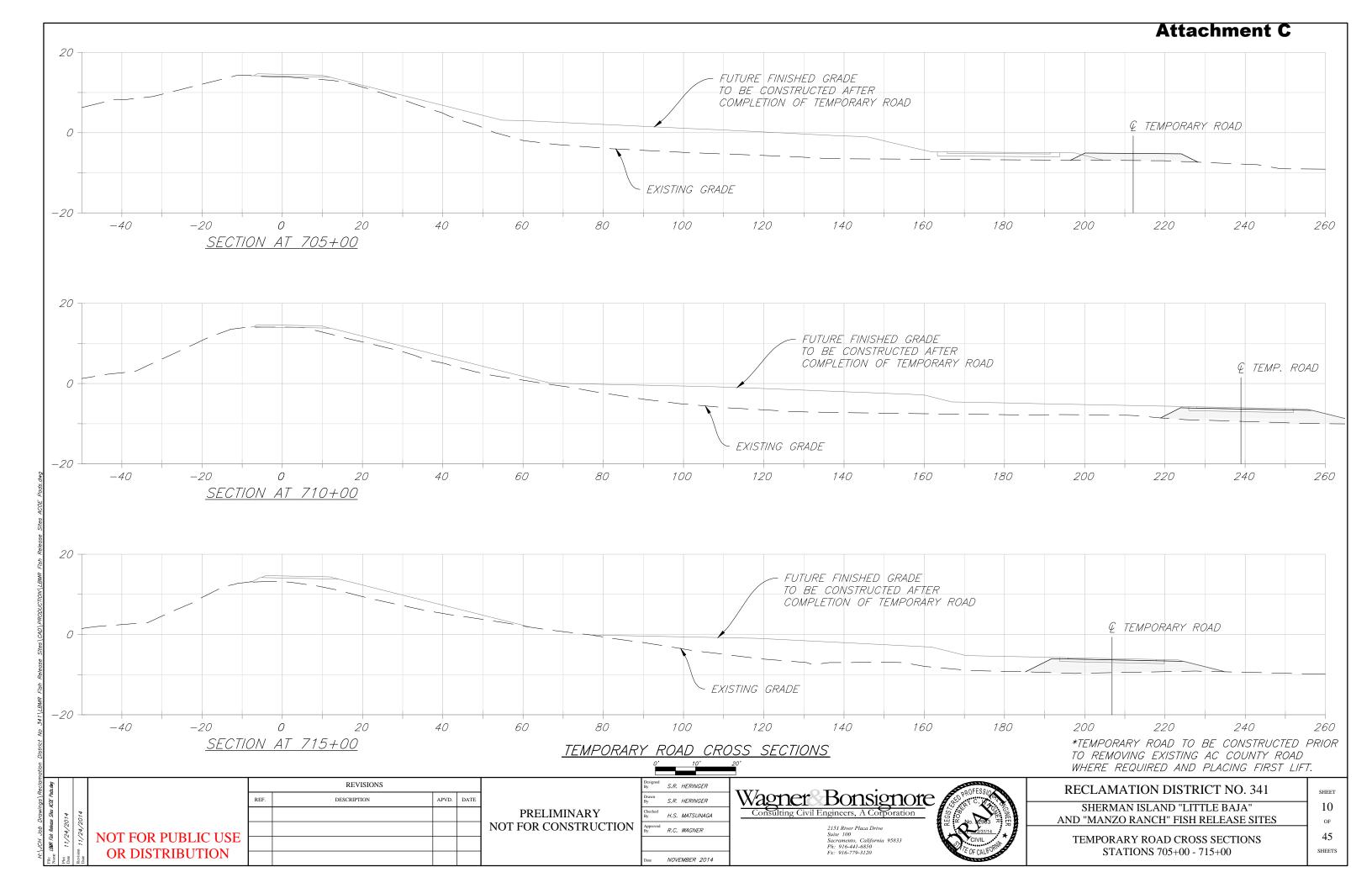


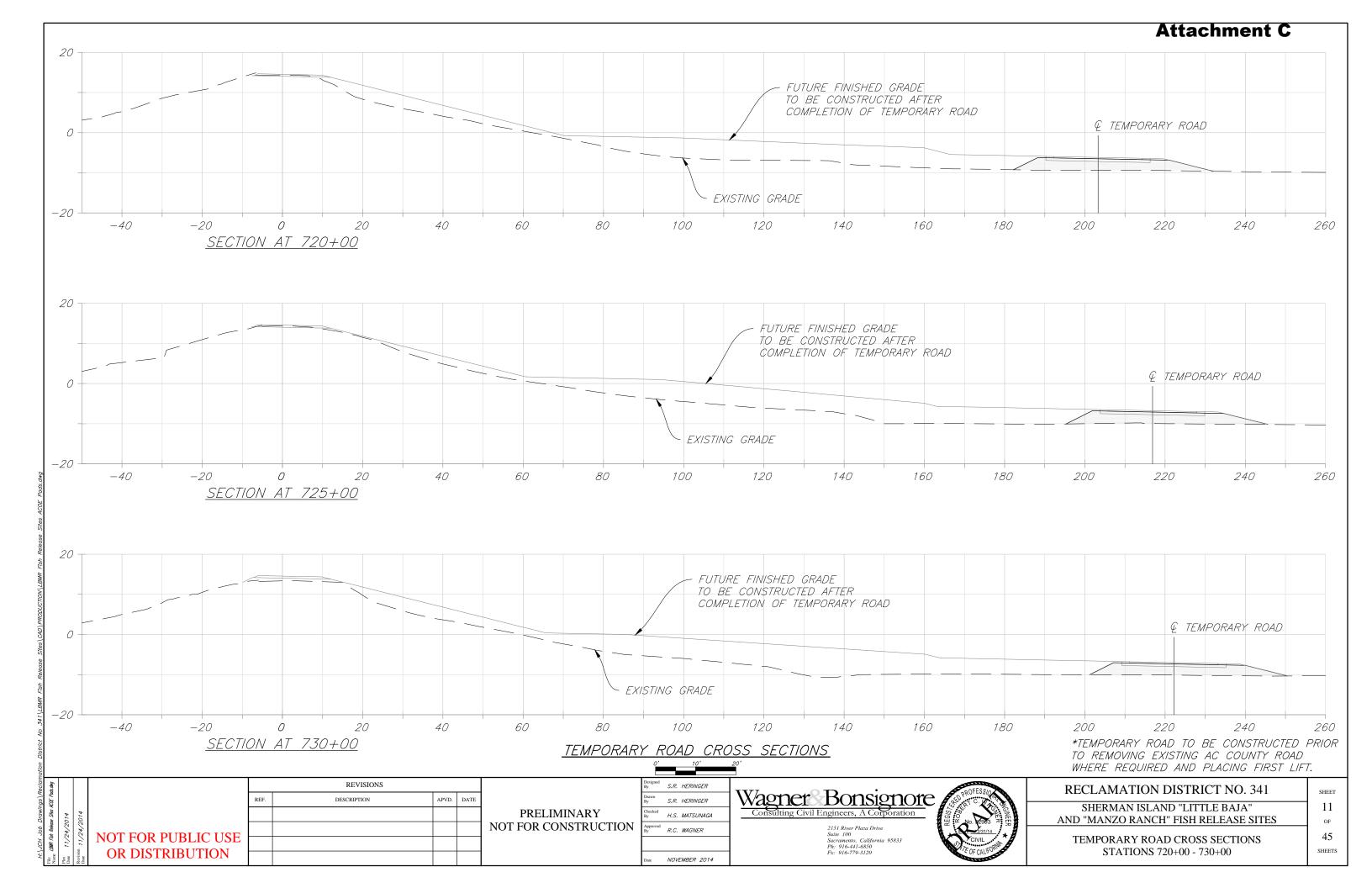


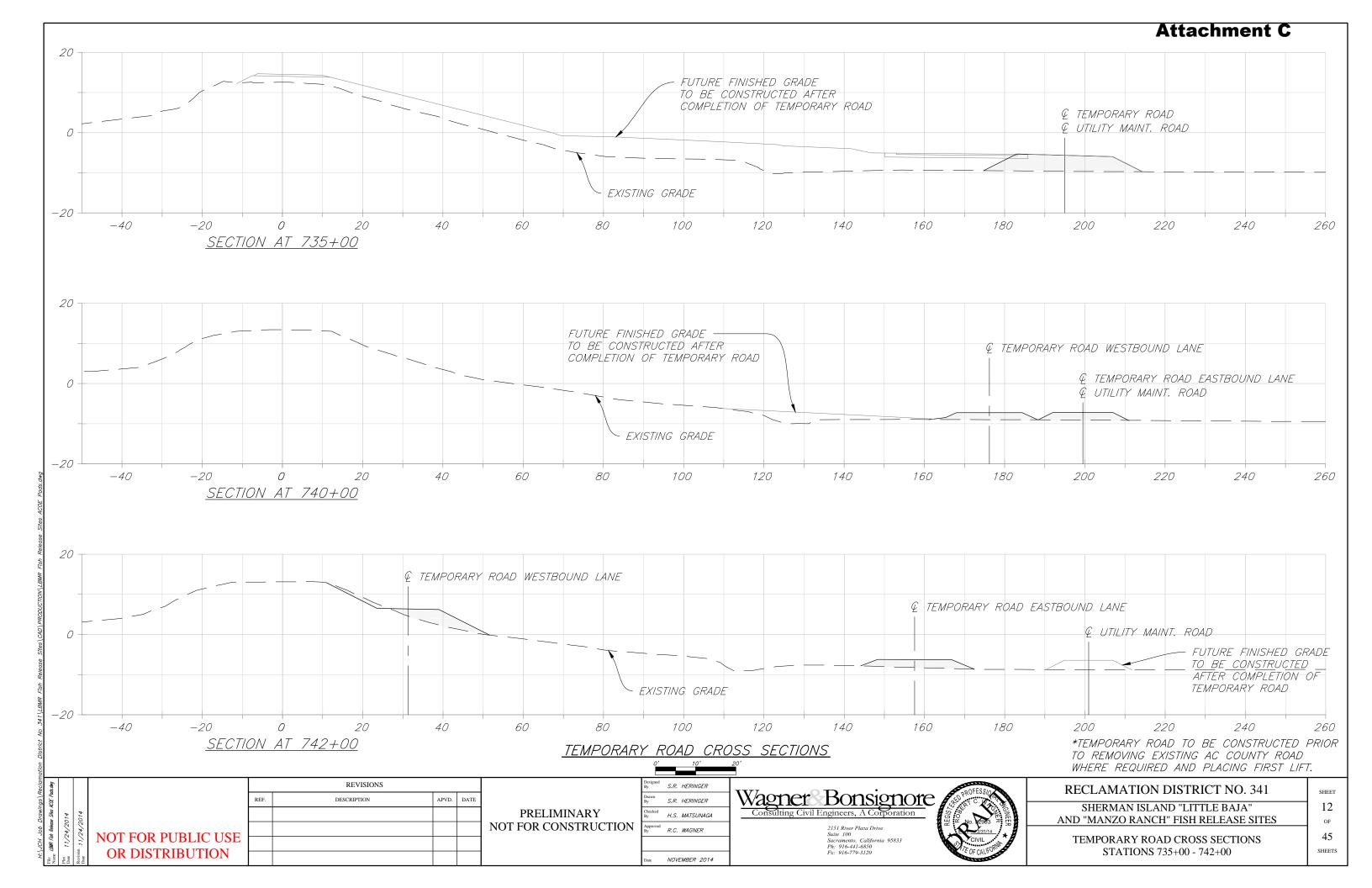


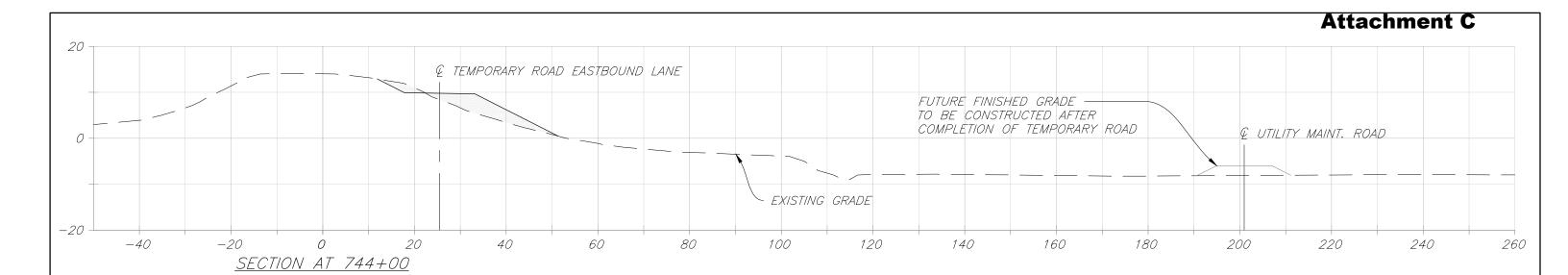












## TEMPORARY ROAD CROSS SECTIONS

\*TEMPORARY ROAD TO BE CONSTRUCTED PRIOR TO REMOVING EXISTING AC COUNTY ROAD WHERE REQUIRED AND PLACING FIRST LIFT.

2								
Reclan	ts.dmg					REVISIONS		
-	ICOE Pos				REF.	DESCRIPTION	APVD.	DATE
rawings	Sites A	14	214					
J 90F	h Release	24/20	24/20	NOT FOR BUILDING LIGH				
$\gamma_{CH}$	LBMR Fis	11/2	111	NOT FOR PUBLIC USE				
Ŧ	2	Plot Date	Revisior Date	OR DISTRIBUTION				

PRELIMINARY NOT FOR CONSTRUCTION

	Designed By	S.R. HERINGER
	Drawn By	S.R. HERINGER
	Checked By	H.S. MATSUNAGA
1	Approved By	R.C. WAGNER
	Date	NOVEMBER 2014

Wagner Bonsignore
Consulting Civil Engineers, A Corporation

2151 River Plaza Drive

gineers, A Corporation

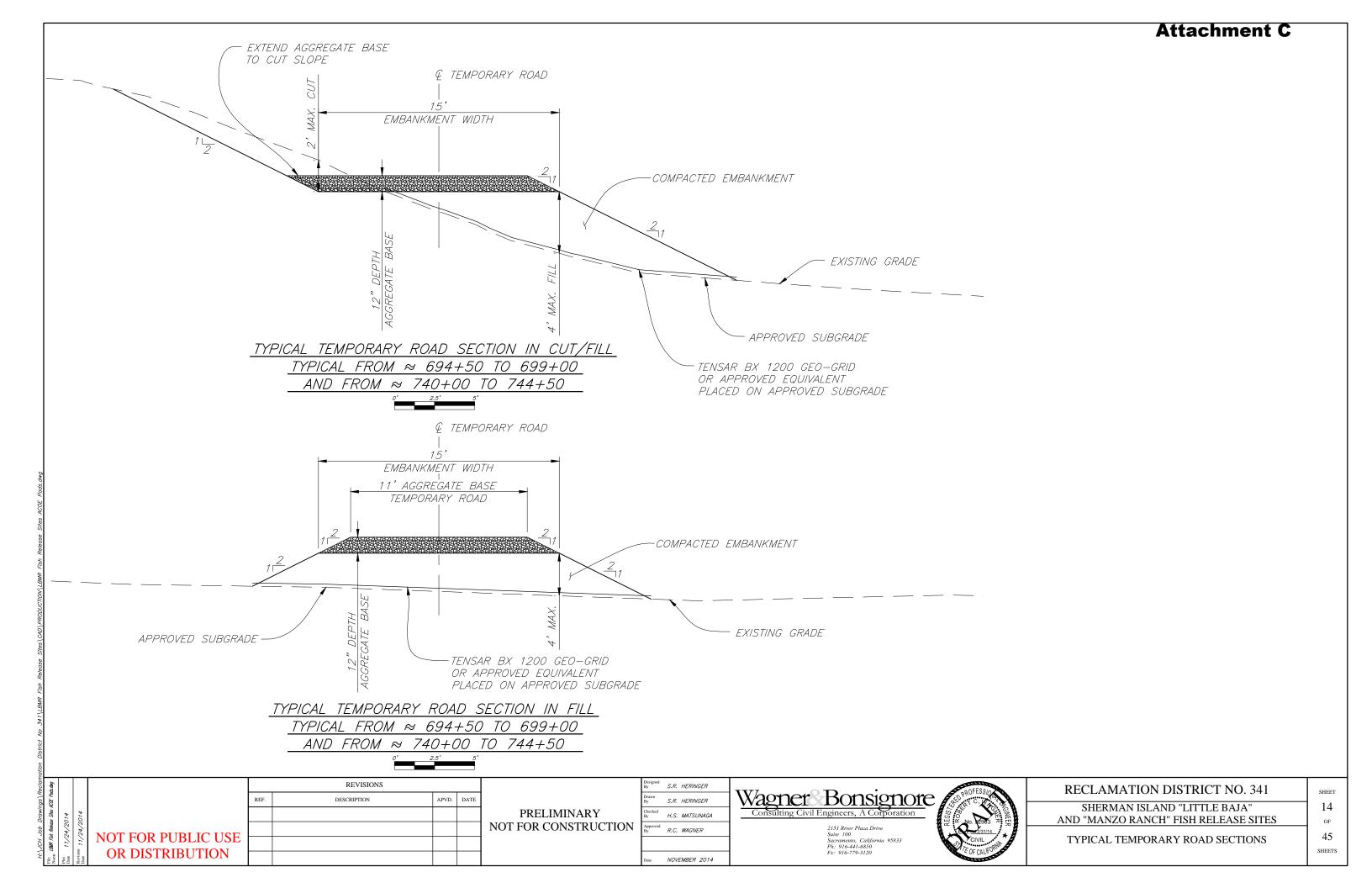
2151 River Plaza Drive
Suite 100
Sacramento, California 95833
Ph: 916-441-6850
Fr. 916-779-3120

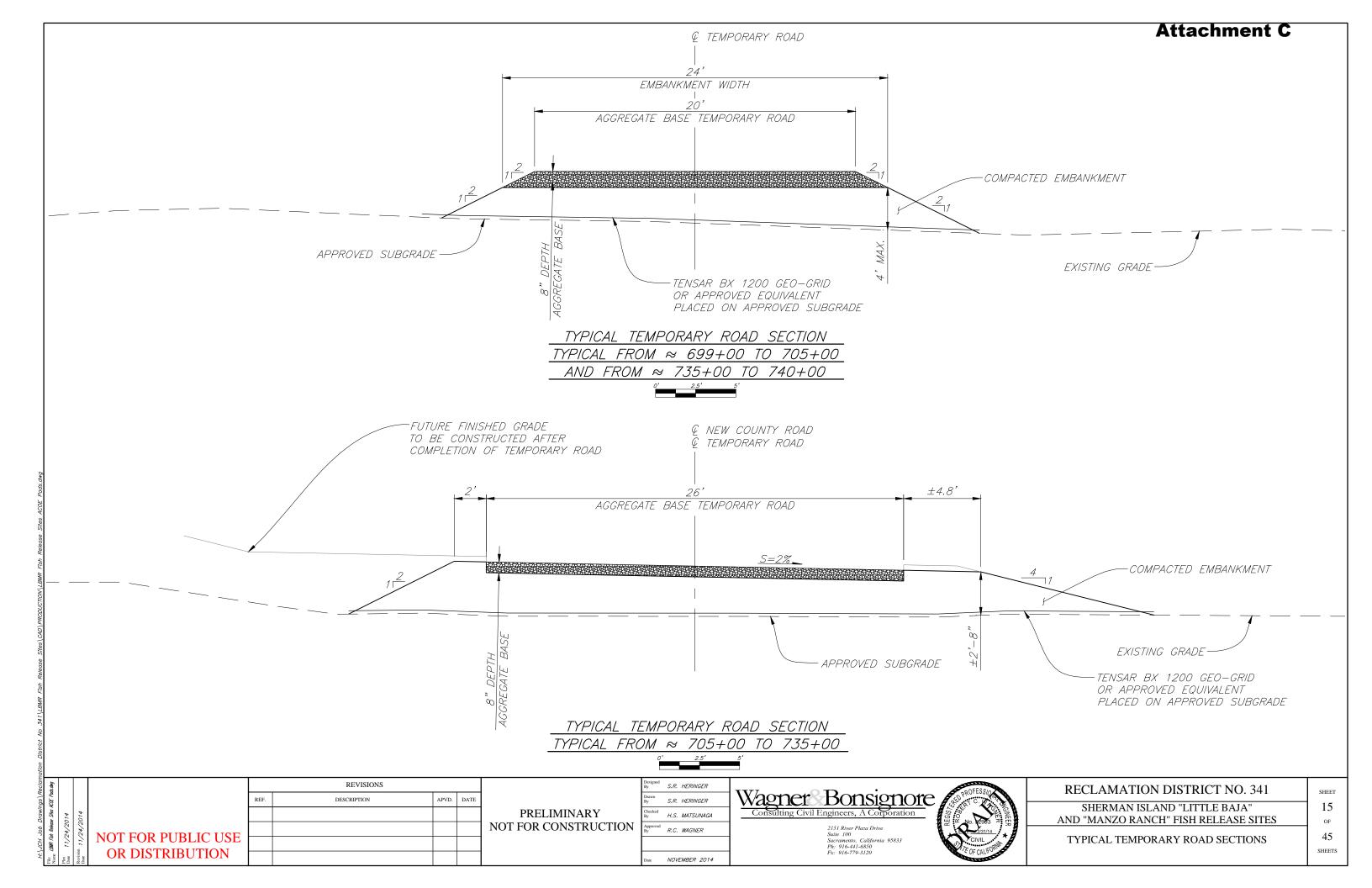
		REQUIRED					•
RECLAMATION DISTRICT NO. 341						SHEET	

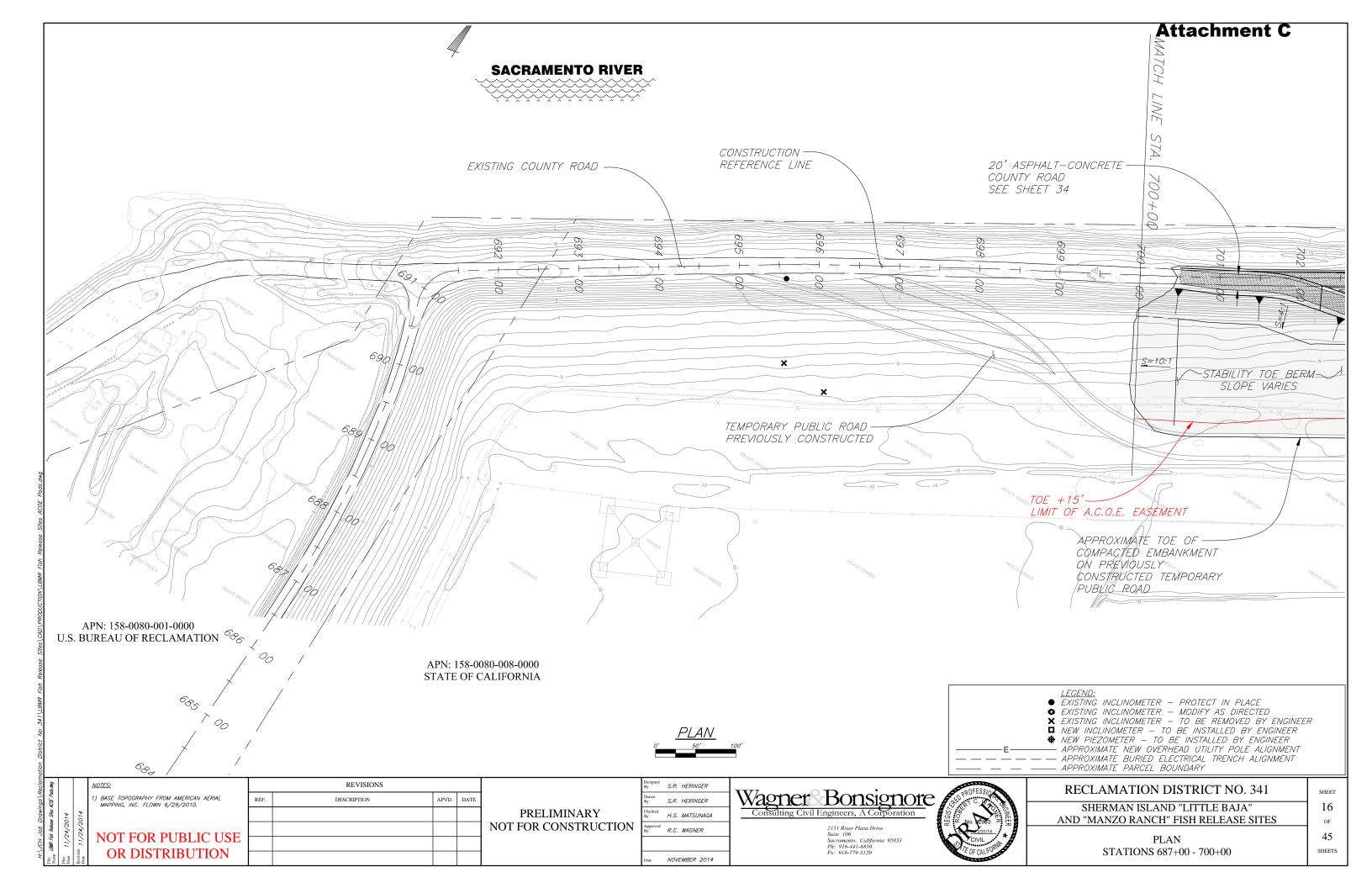
SHERMAN ISLAND "LITTLE BAJA"
AND "MANZO RANCH" FISH RELEASE SITES

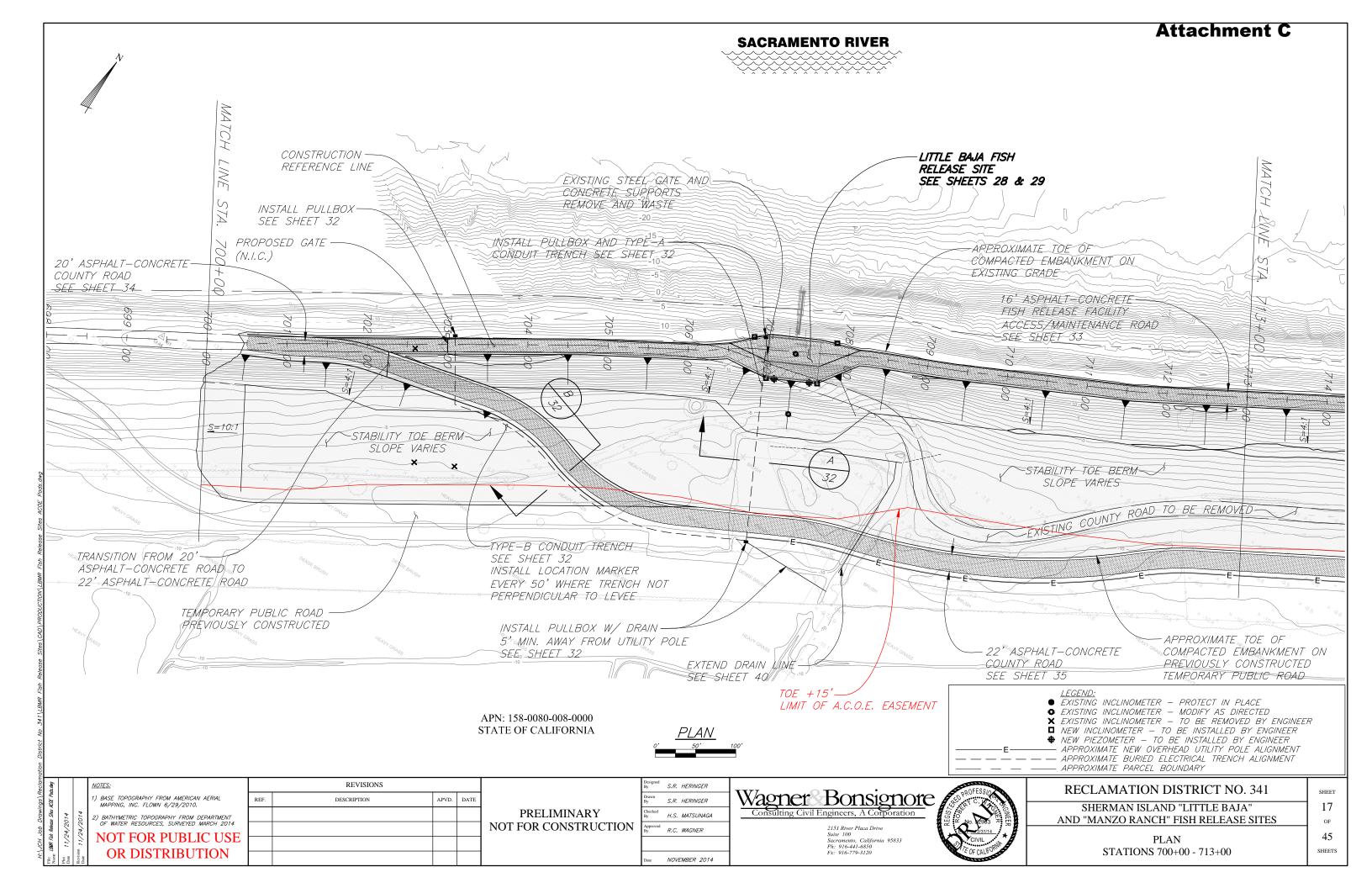
TEMPORARY ROAD CROSS SECTIONS STATION 744+00 45 SHEETS

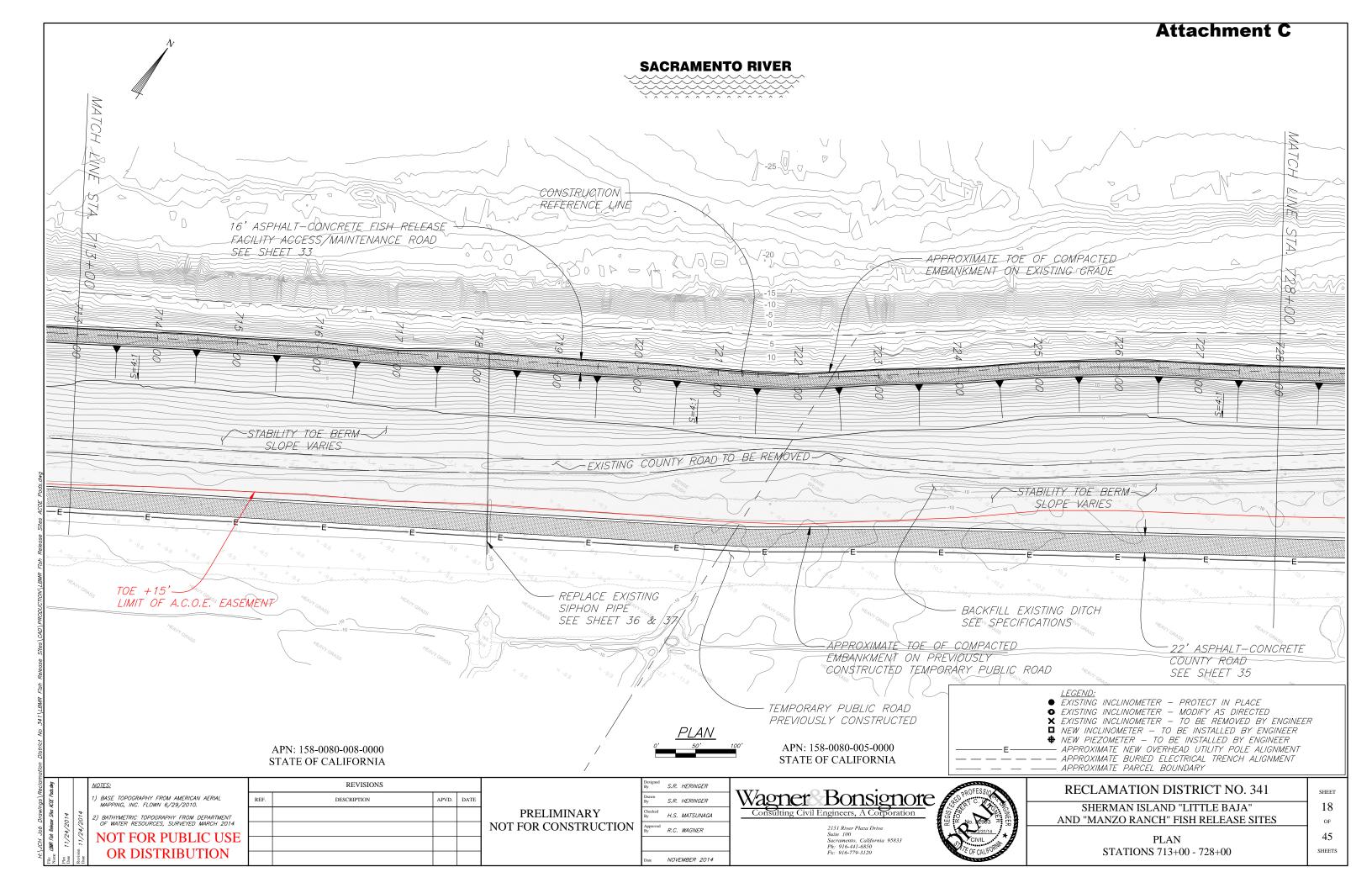
13

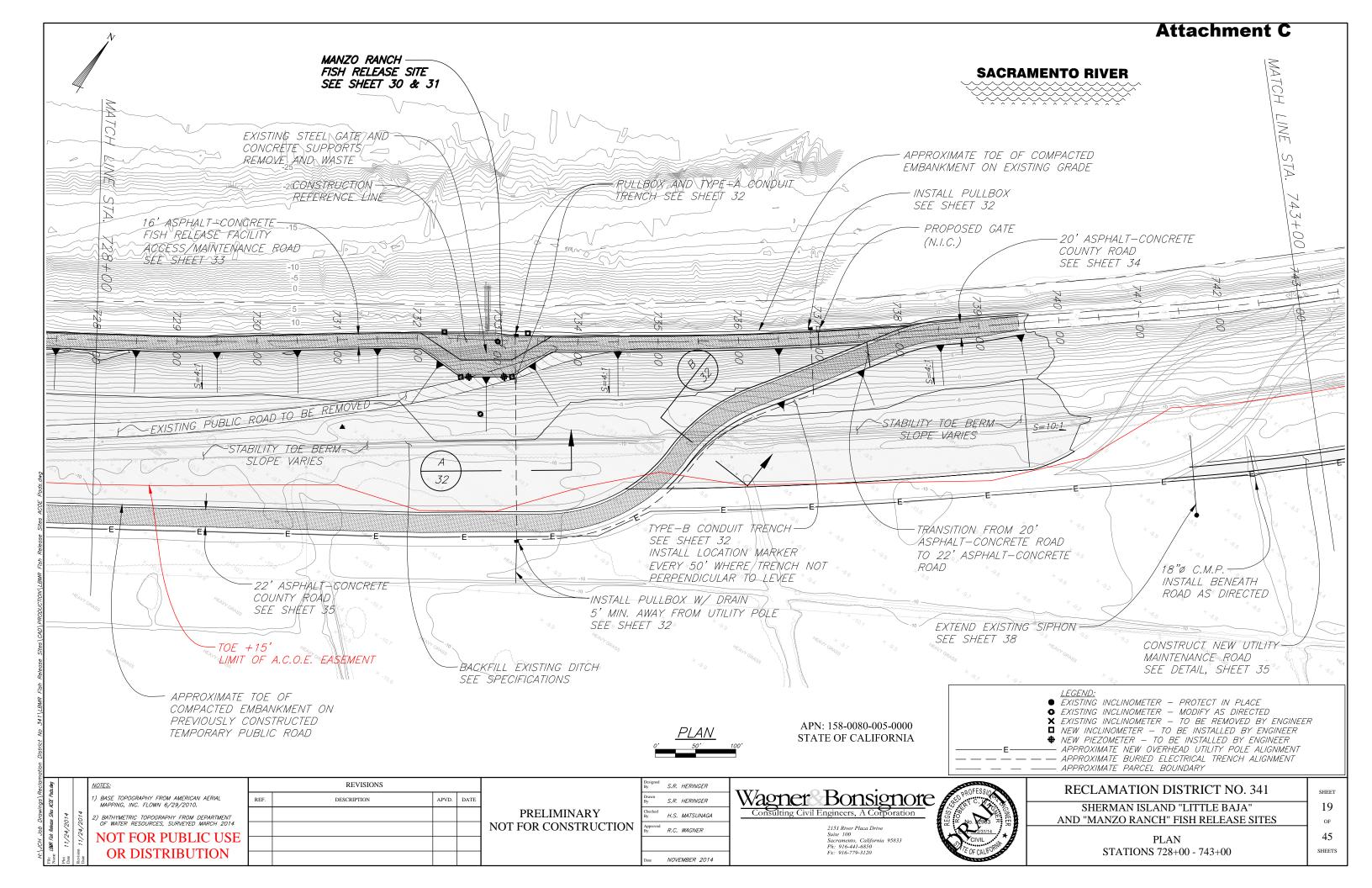


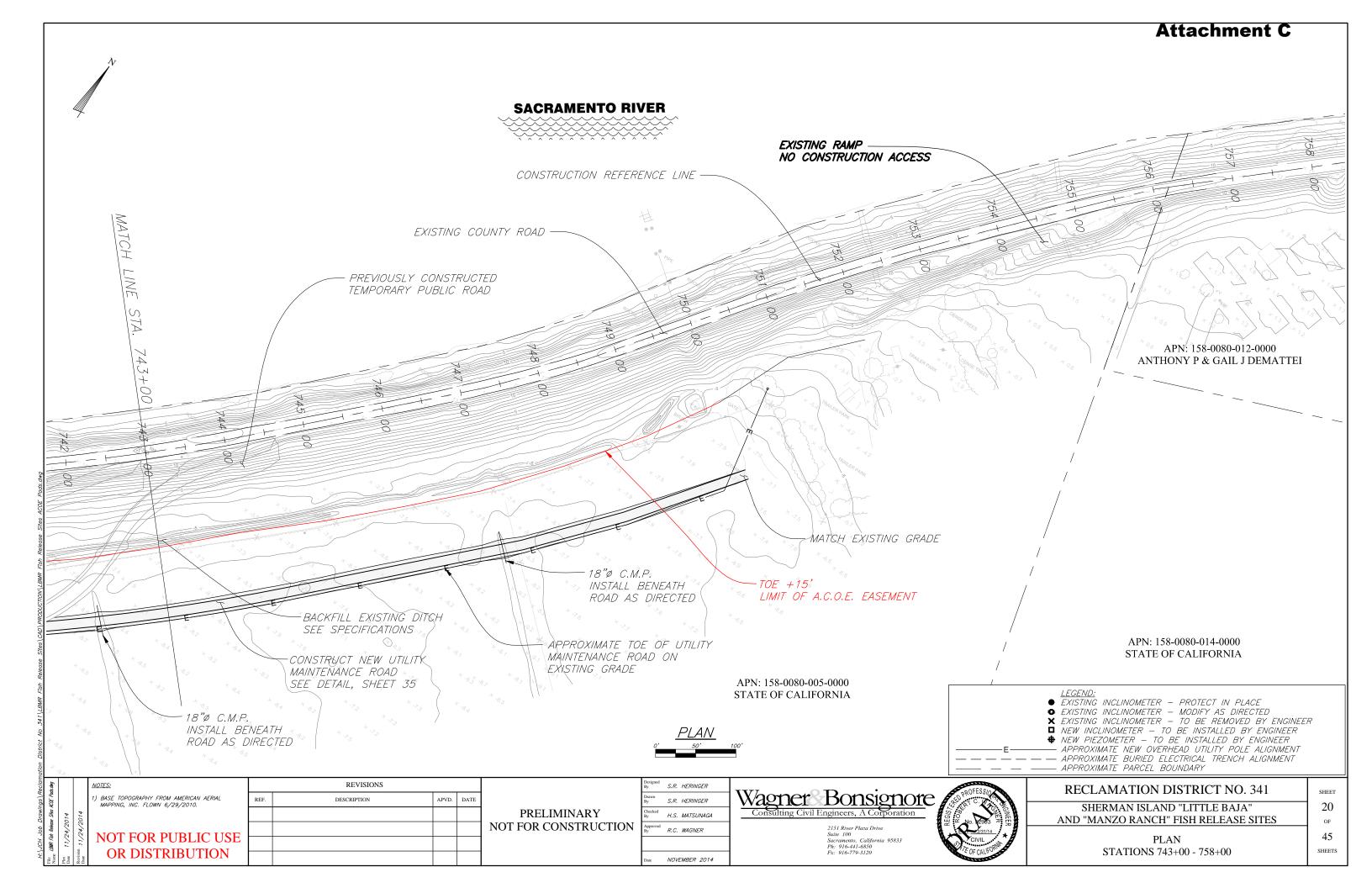


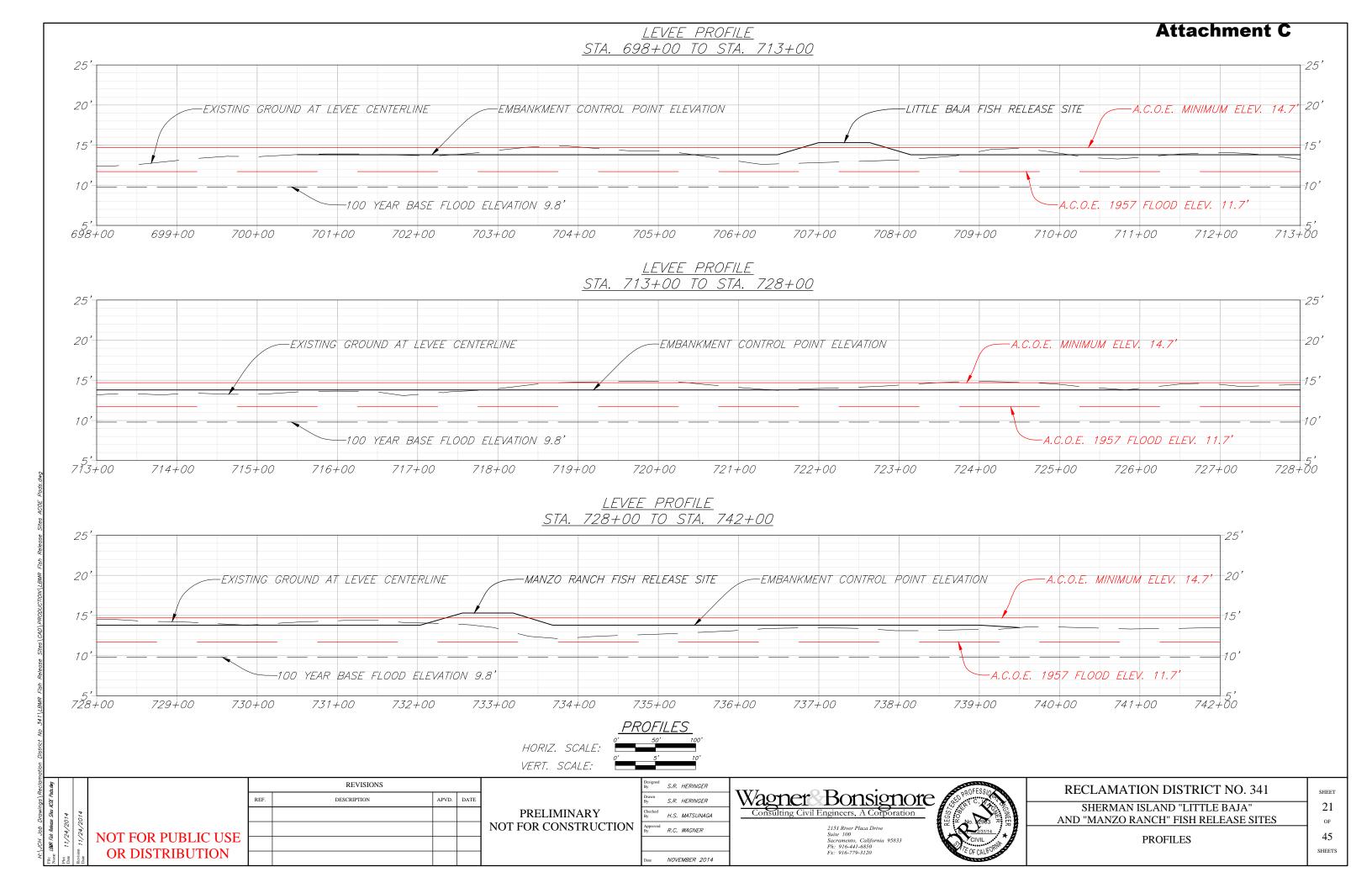


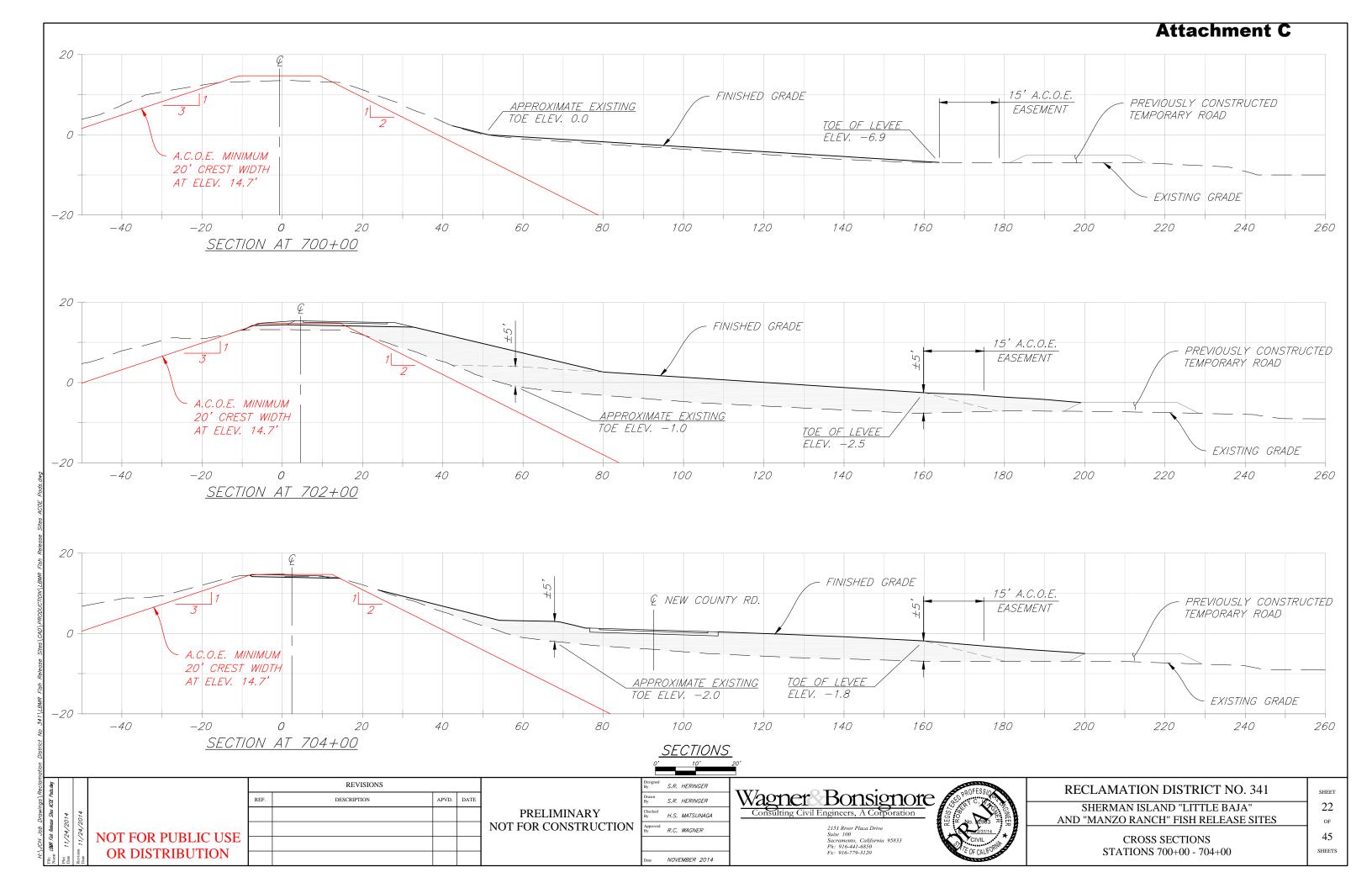


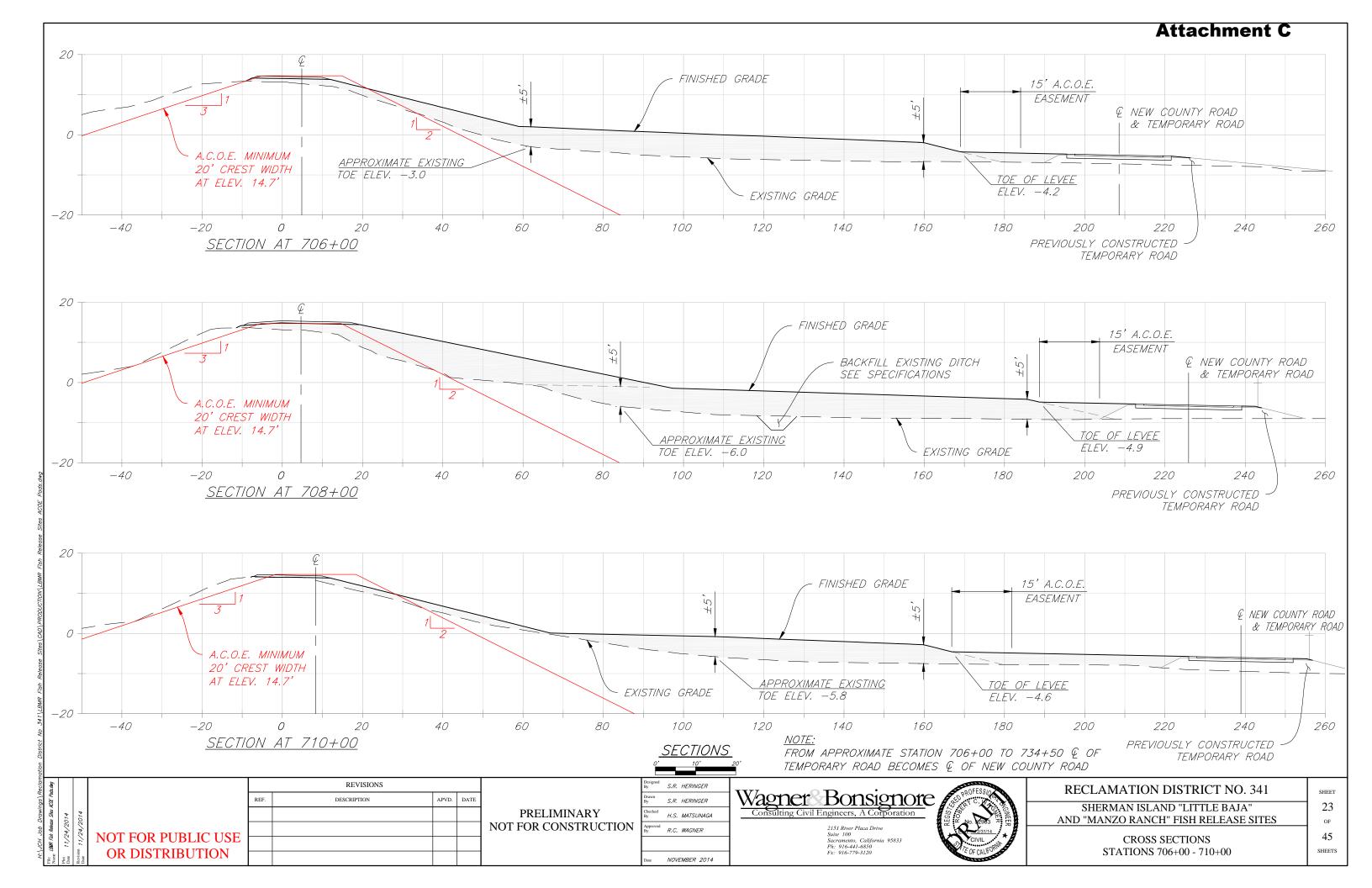


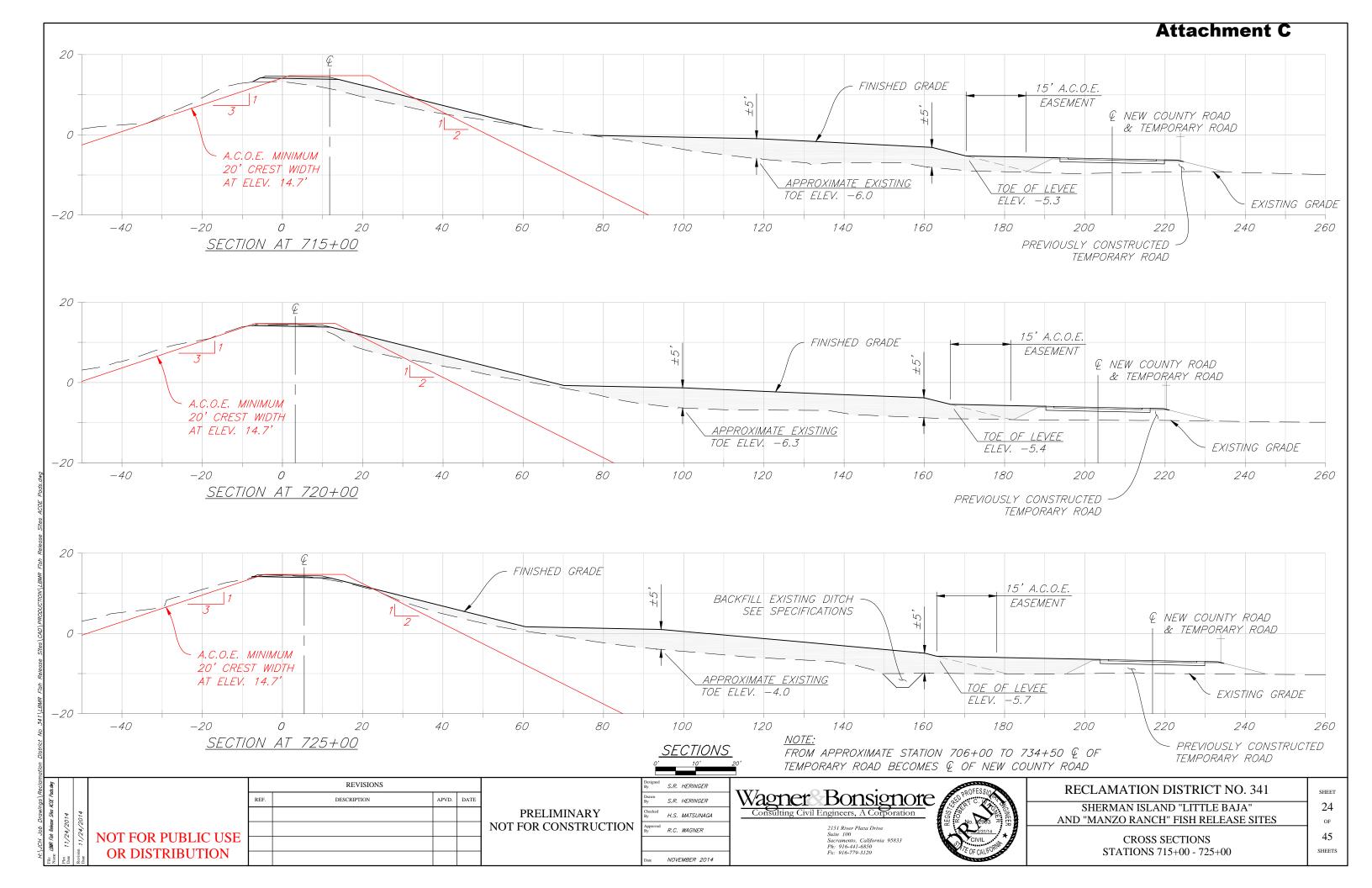


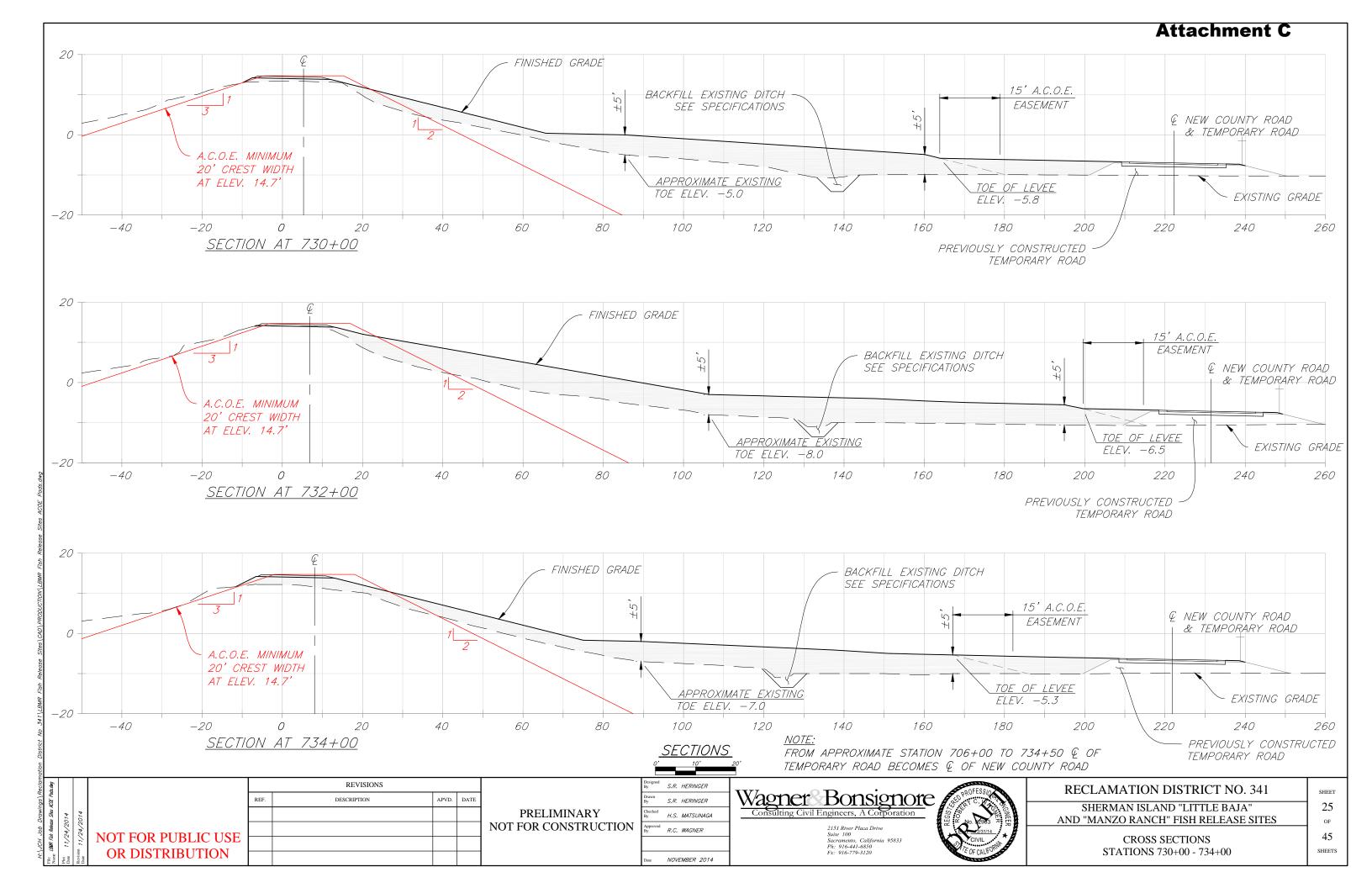


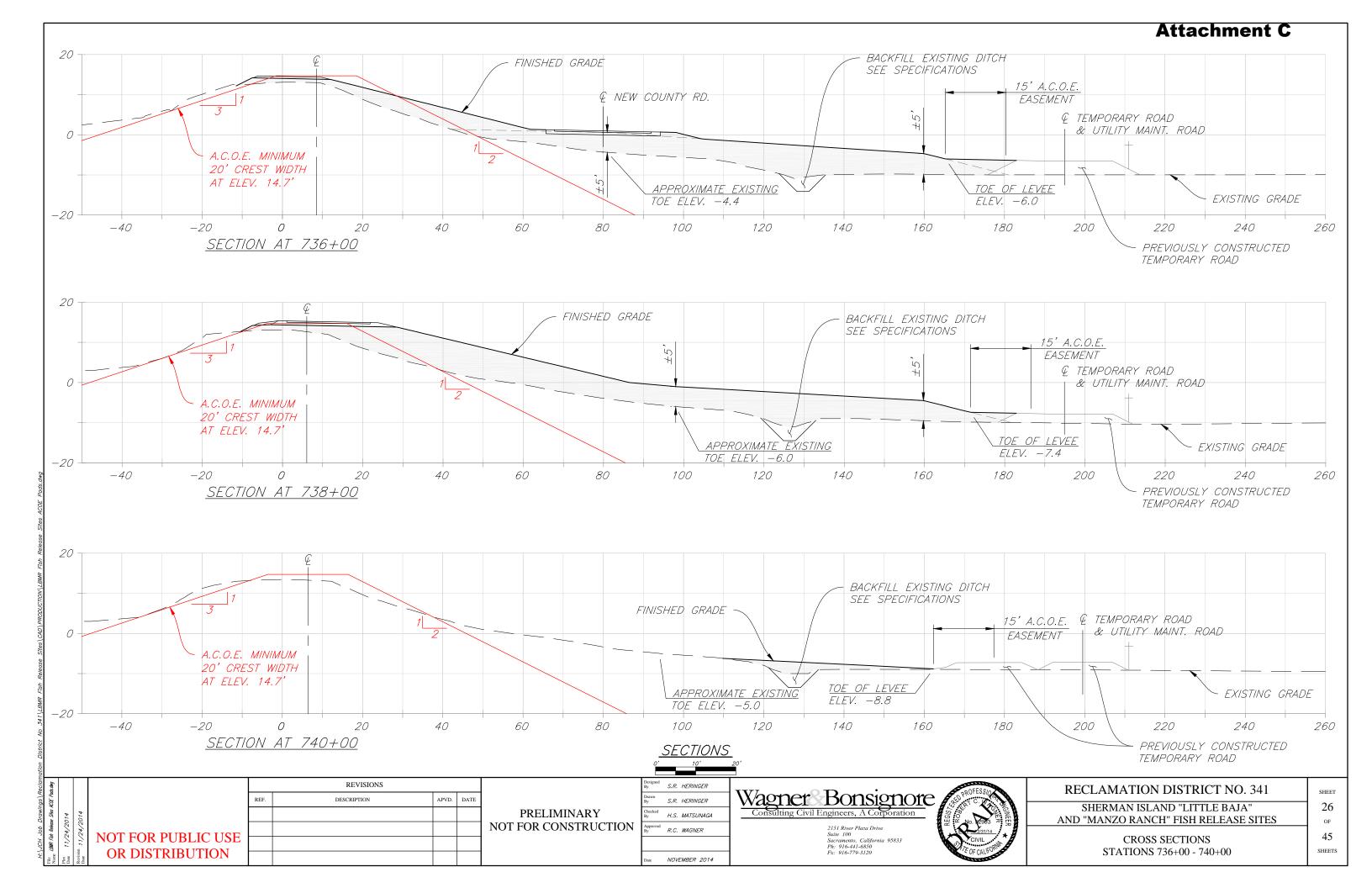


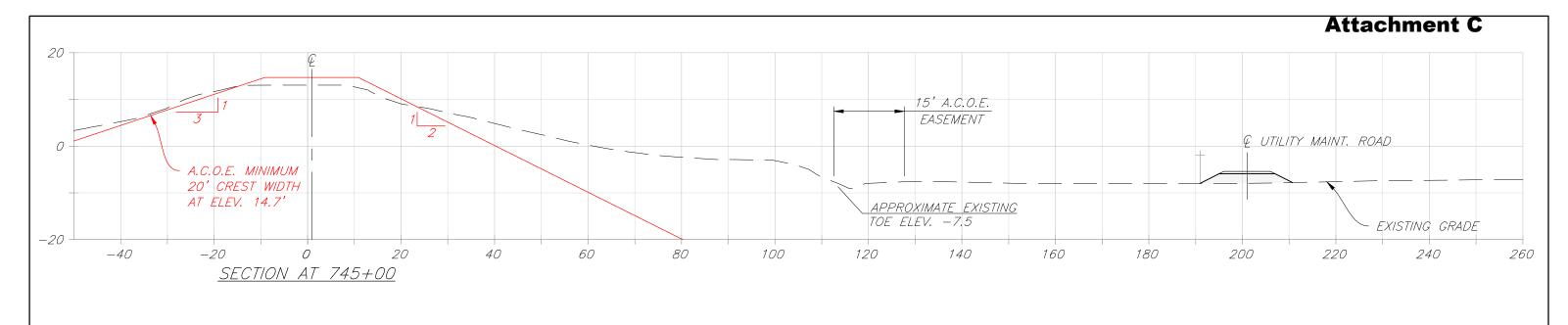


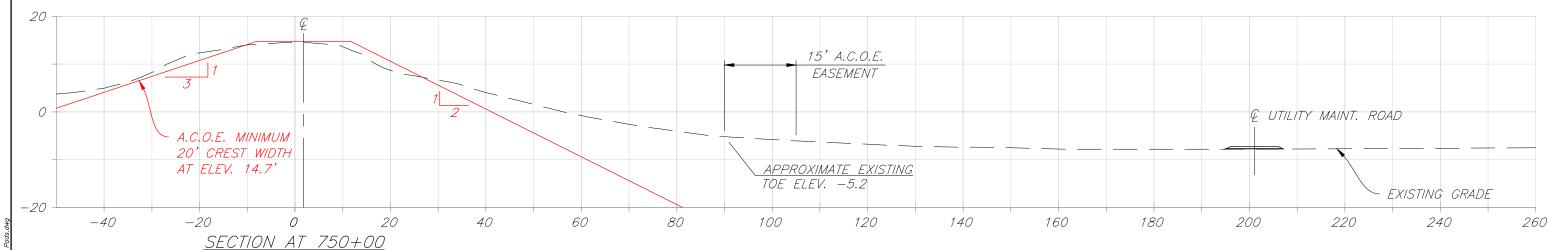












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RECLAMATION DISTRICT NO. 341
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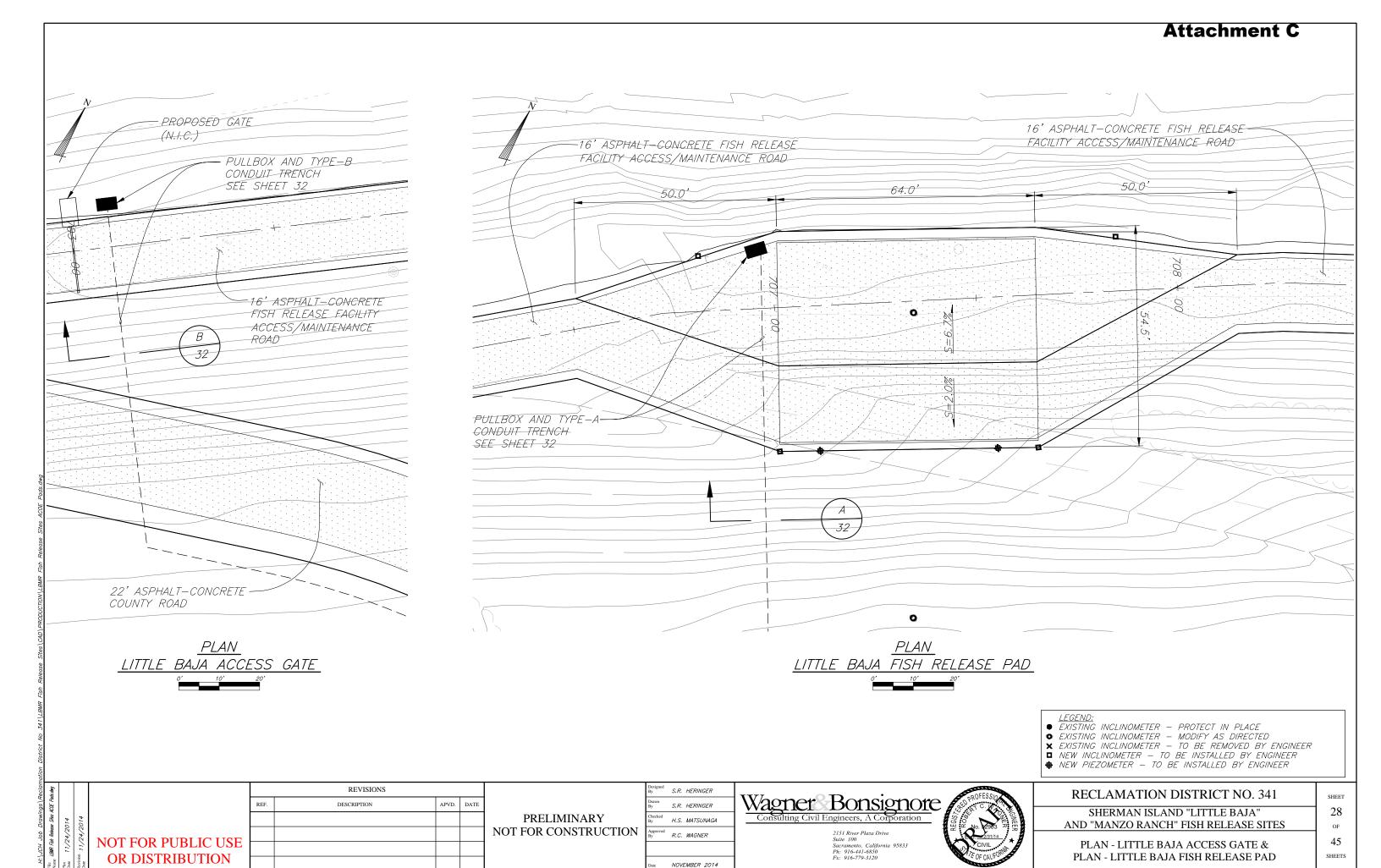
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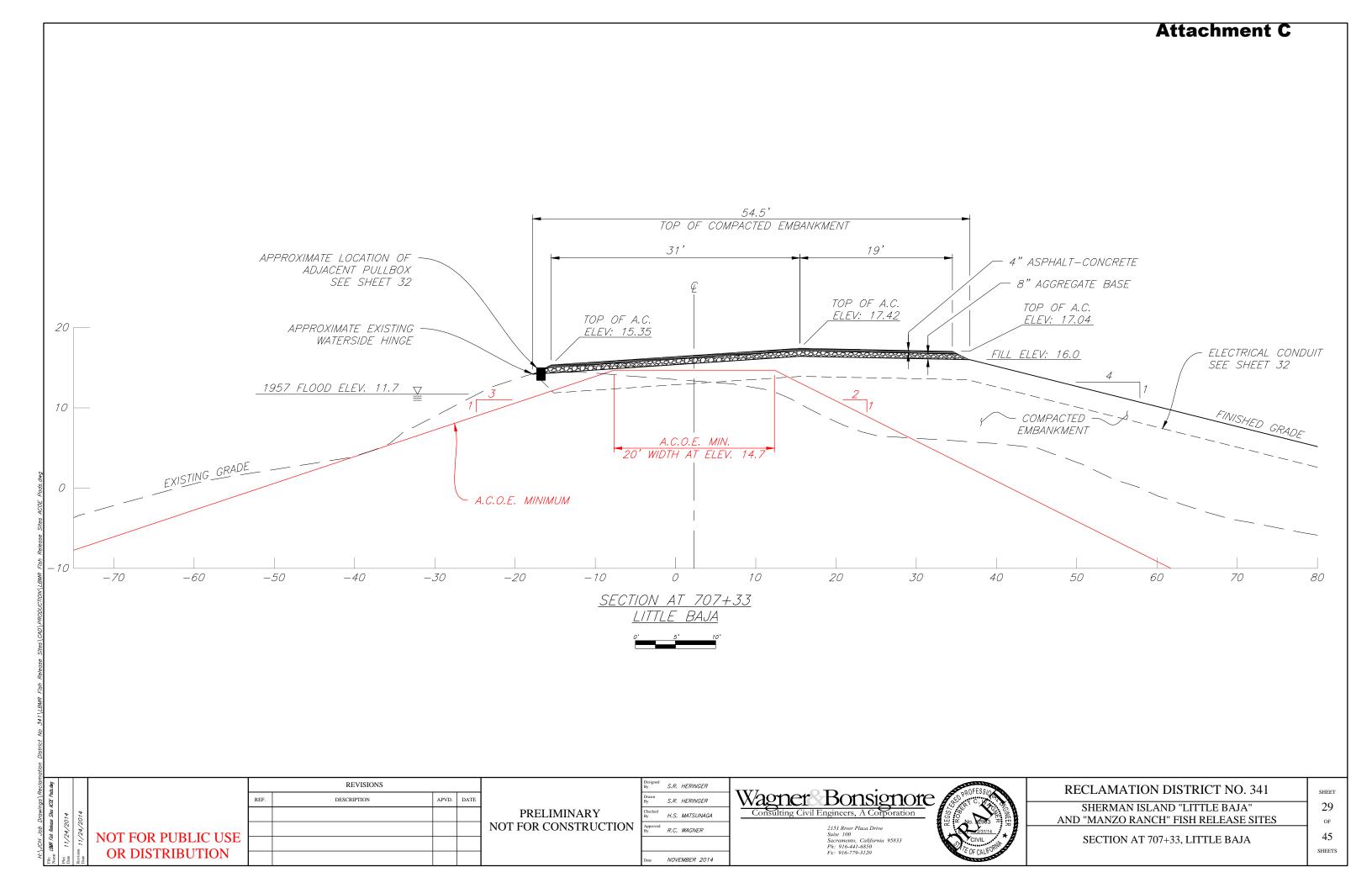
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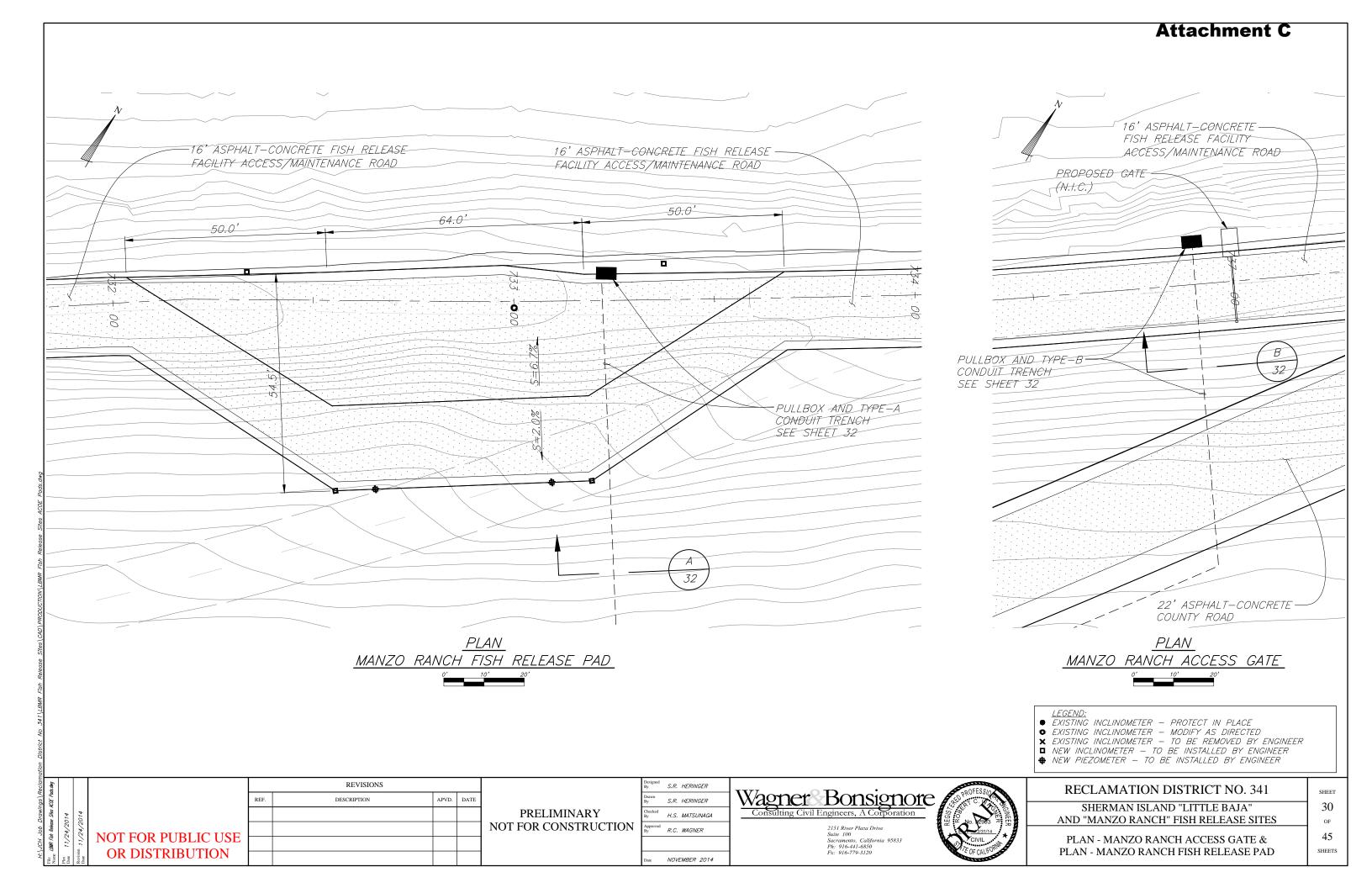
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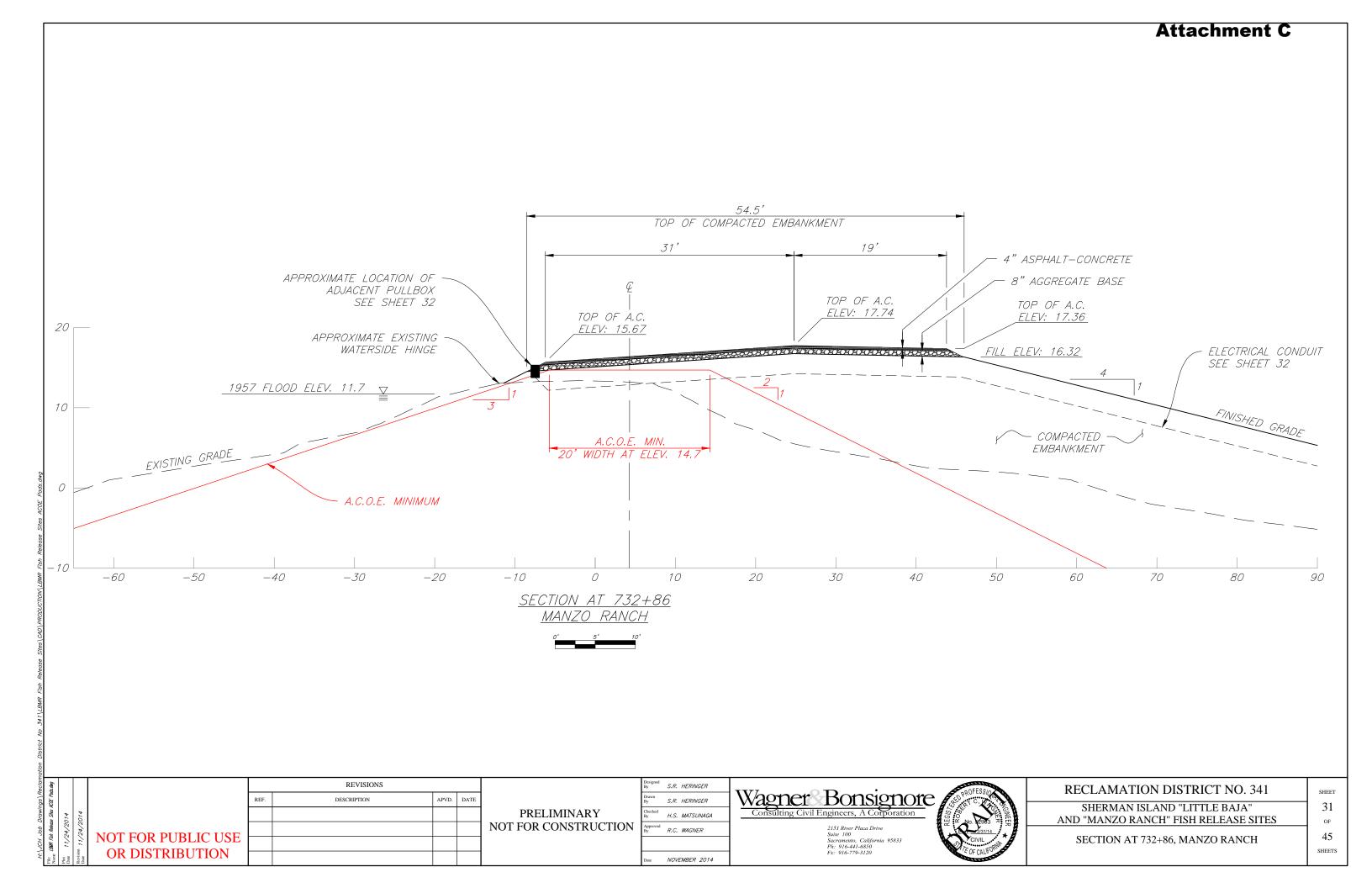
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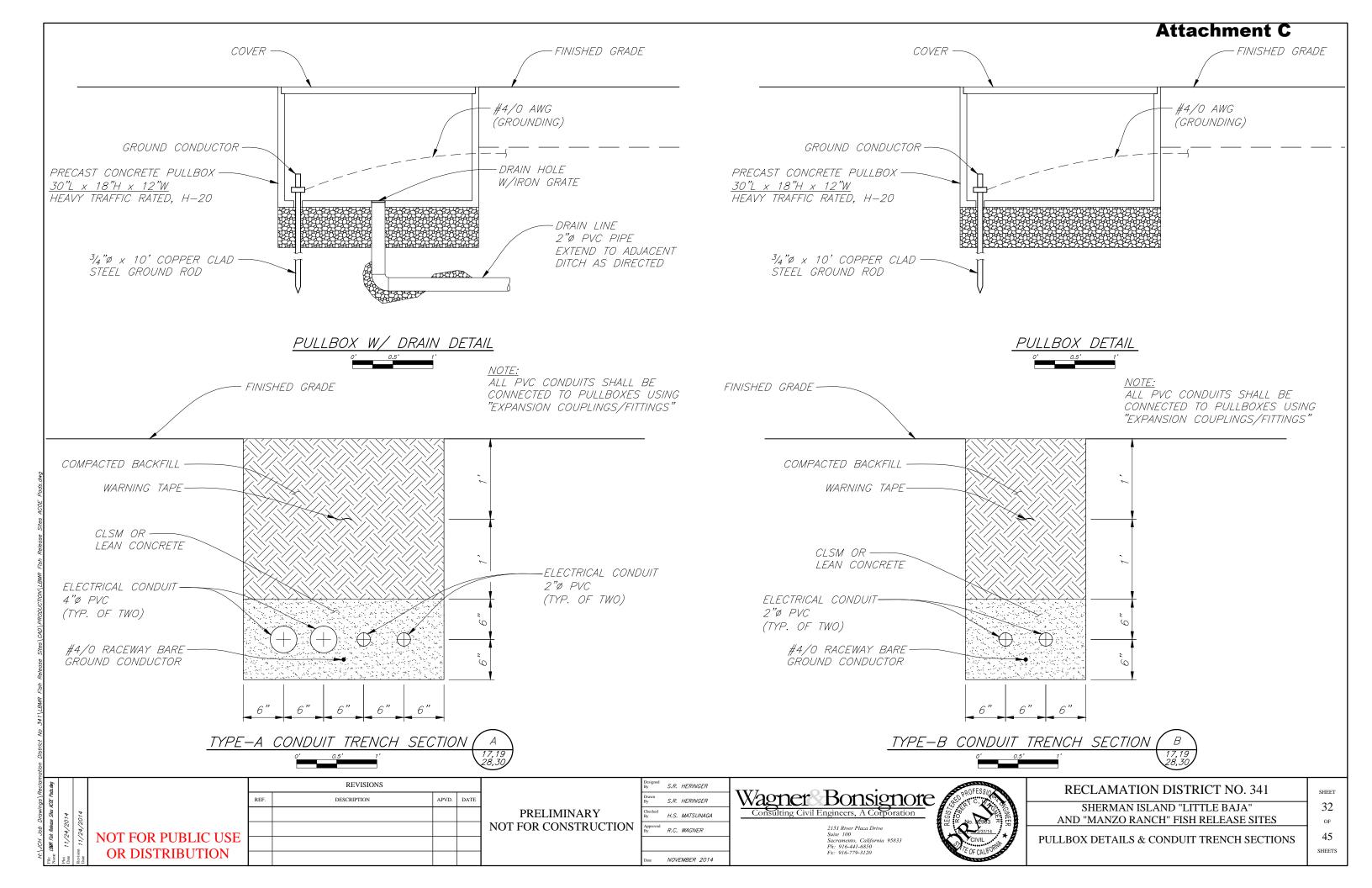
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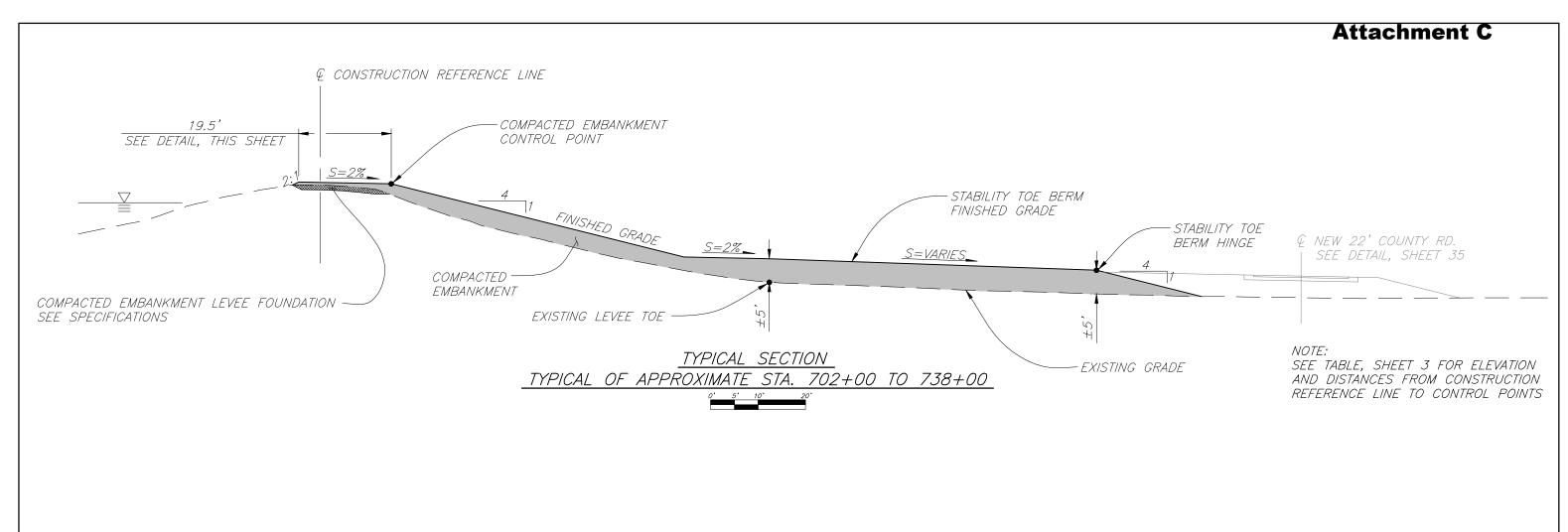


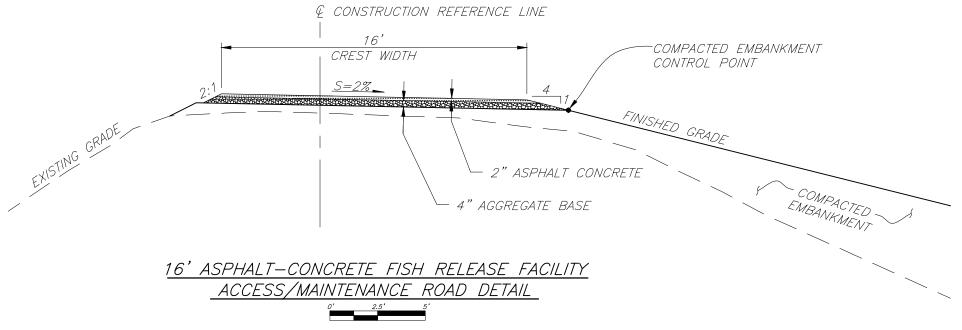












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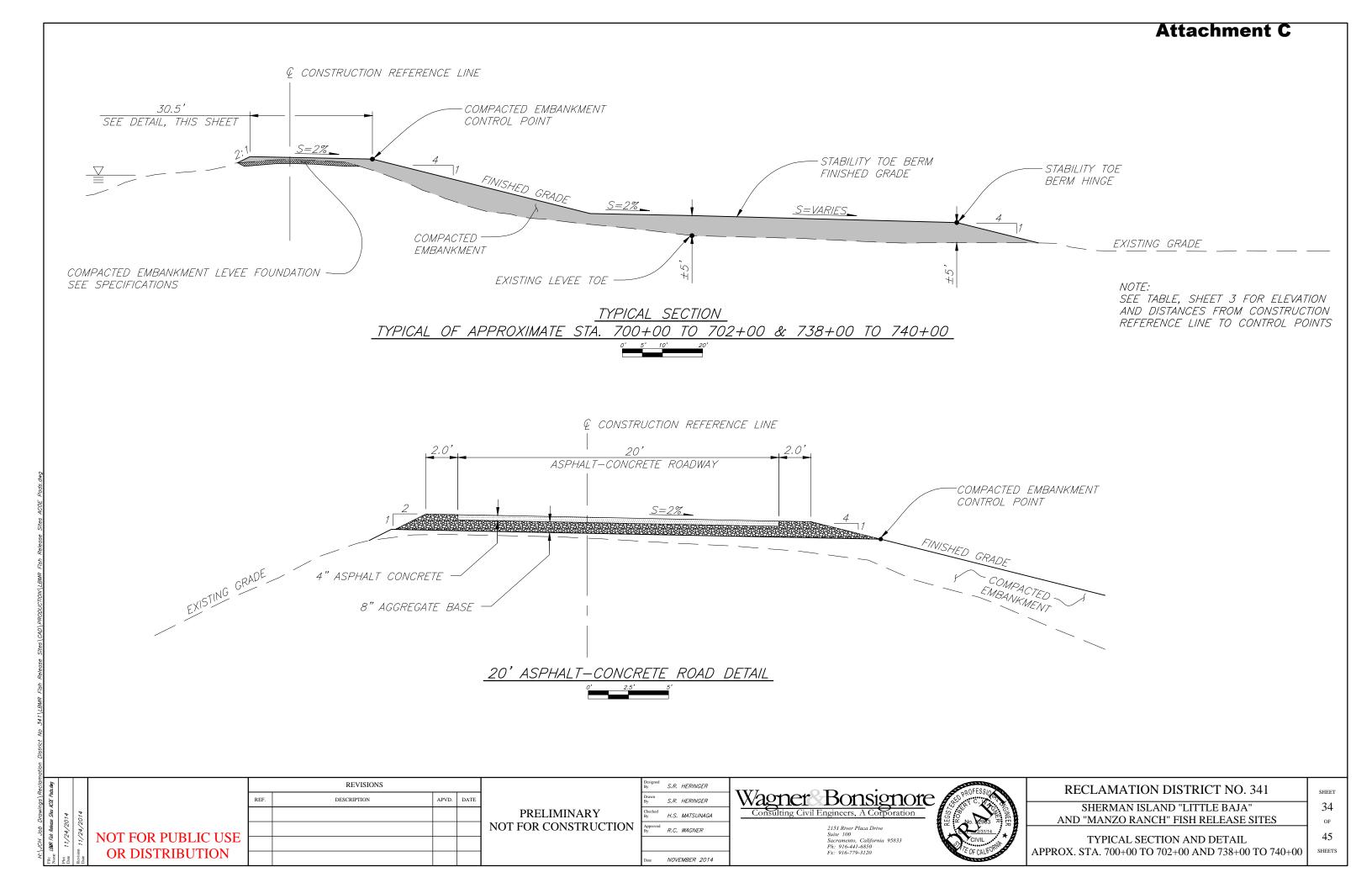
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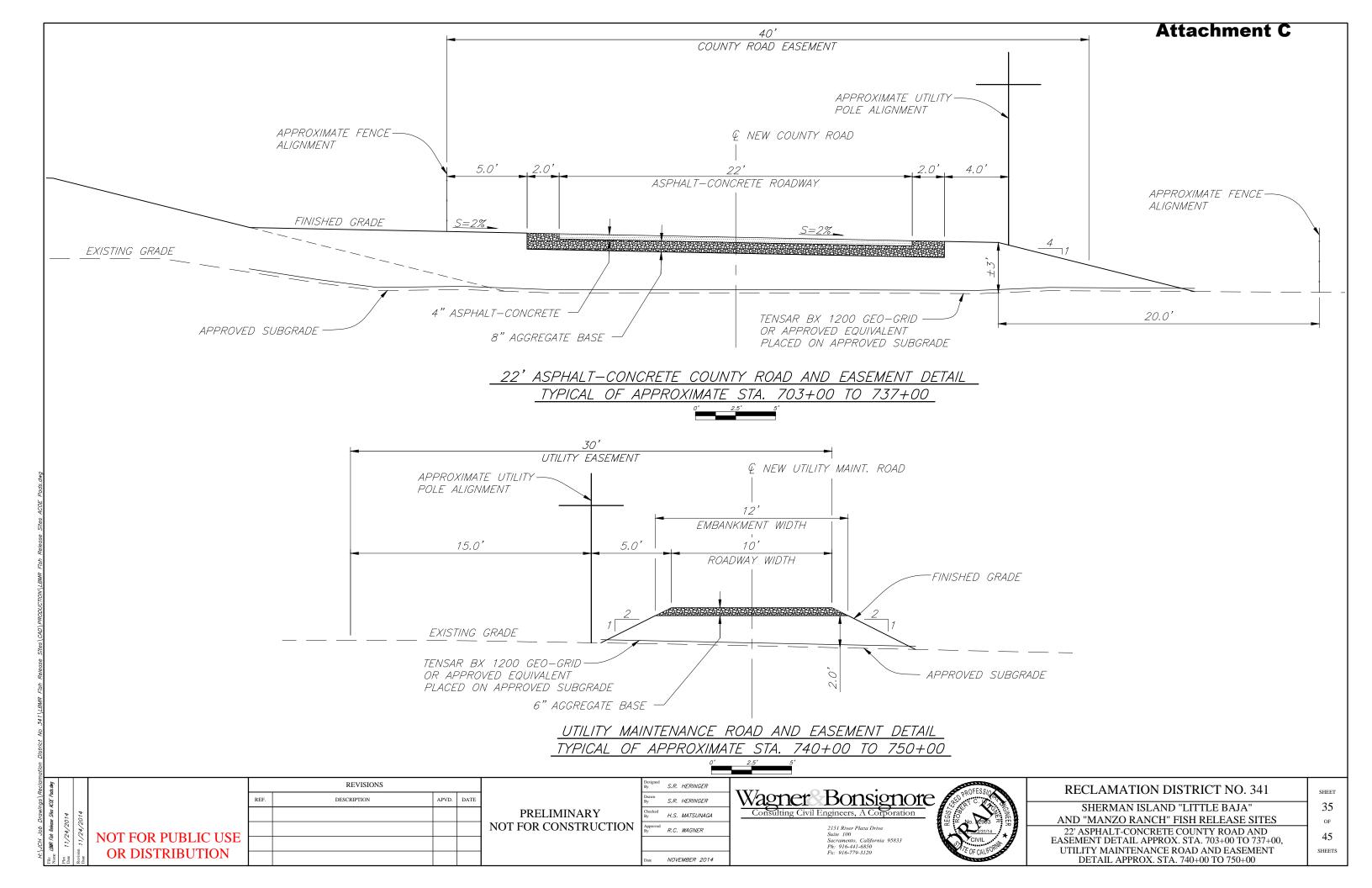
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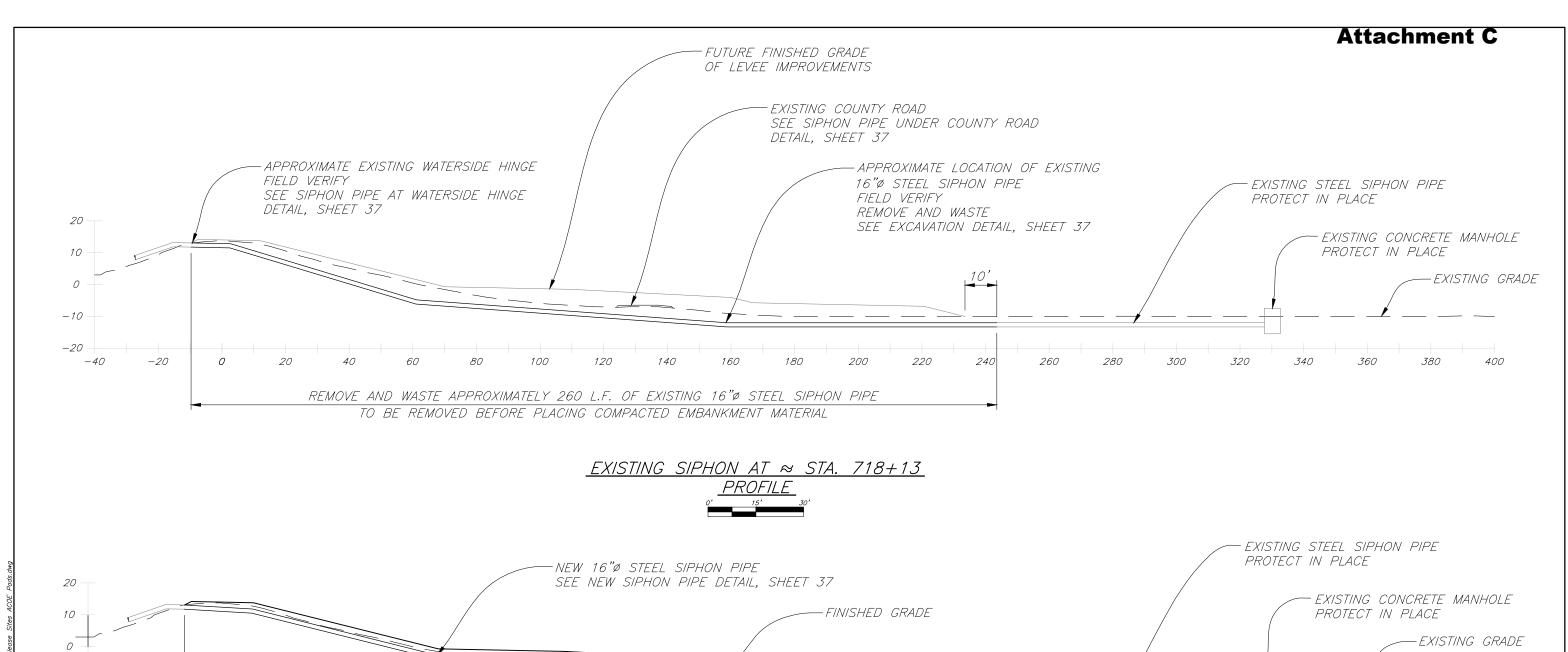
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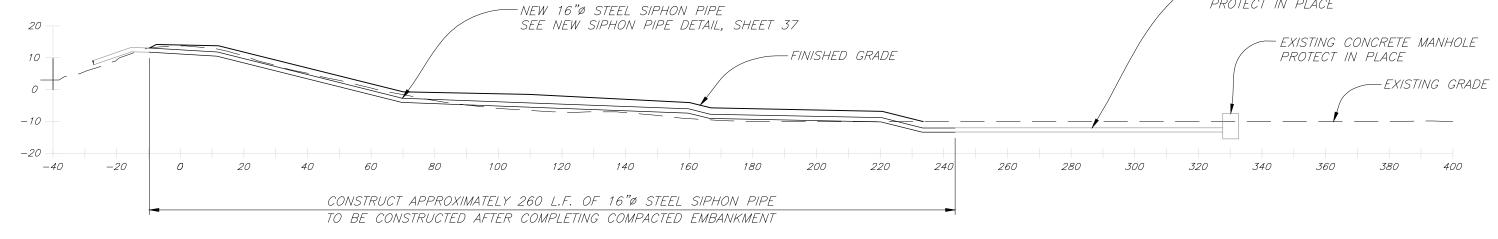
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## NEW SIPHON AT ≈ STA. 718+13 PROFILE 15' 30'

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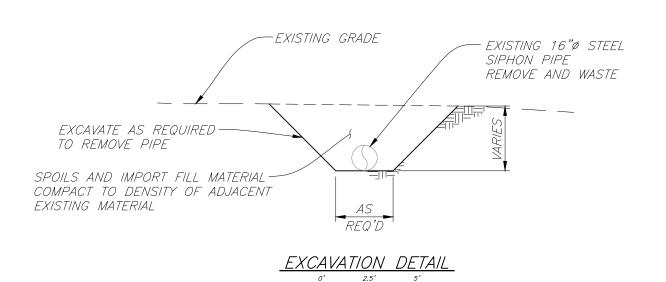
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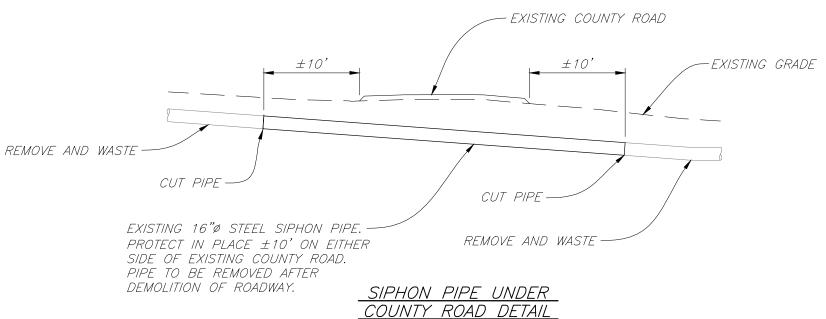
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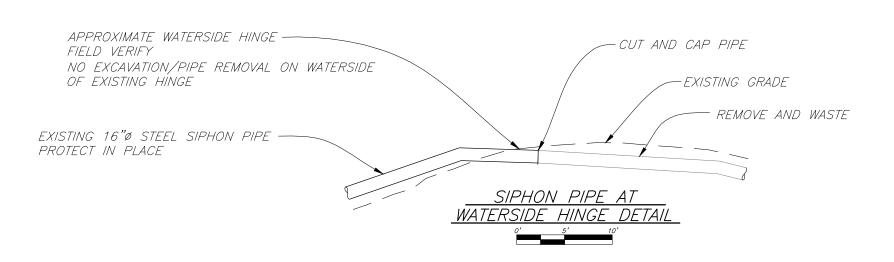
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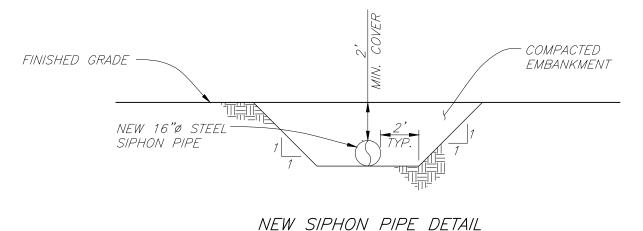
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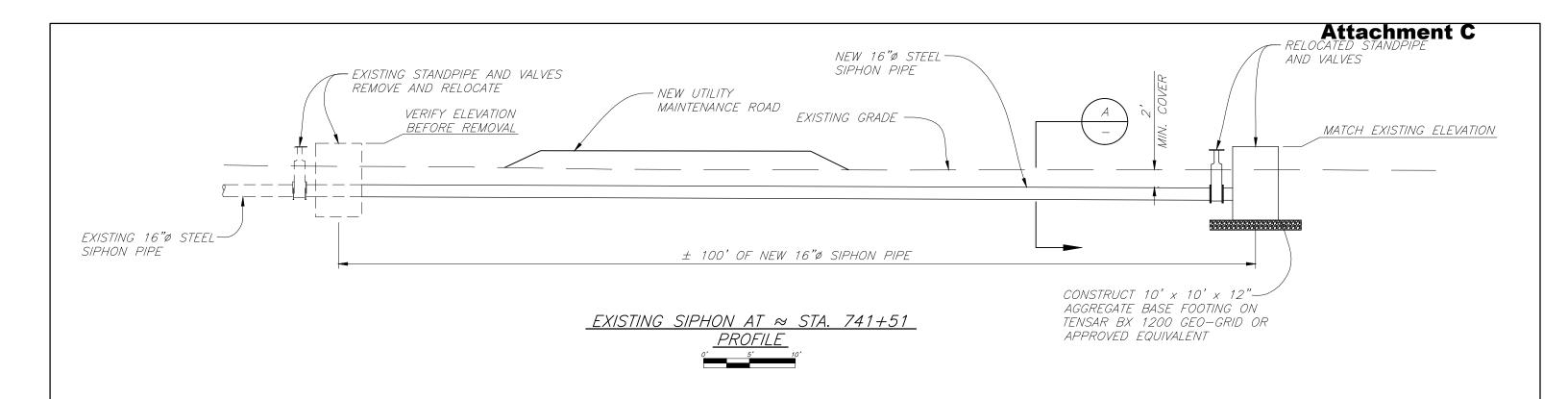
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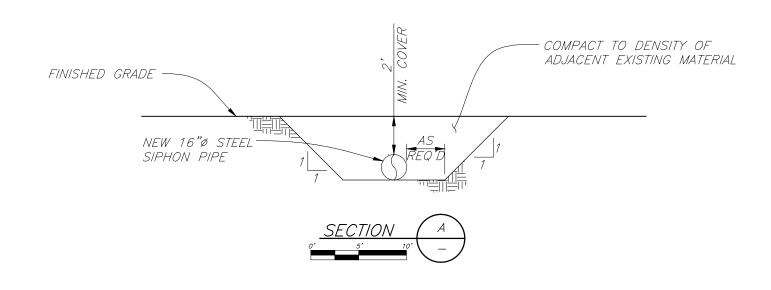
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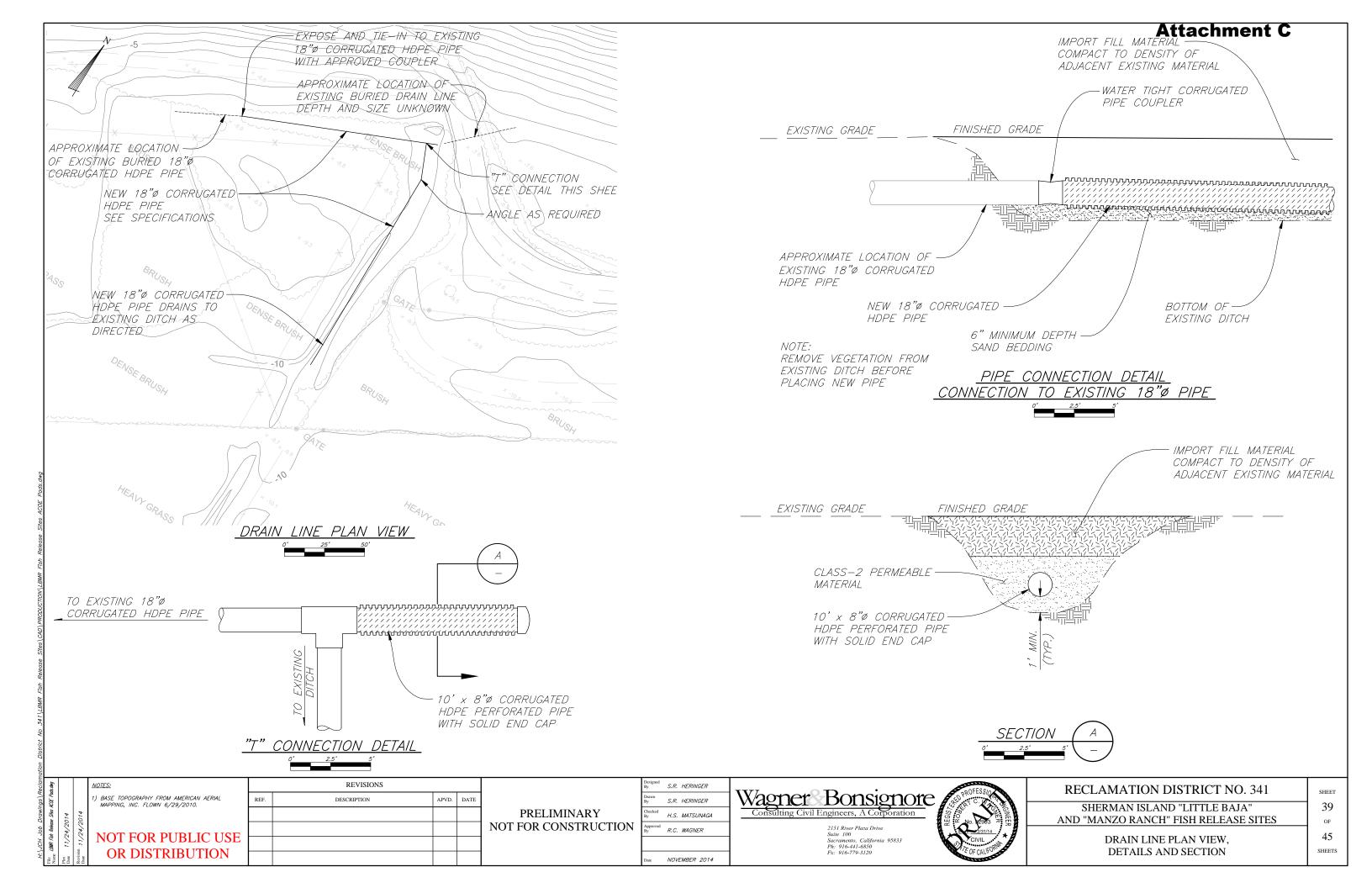
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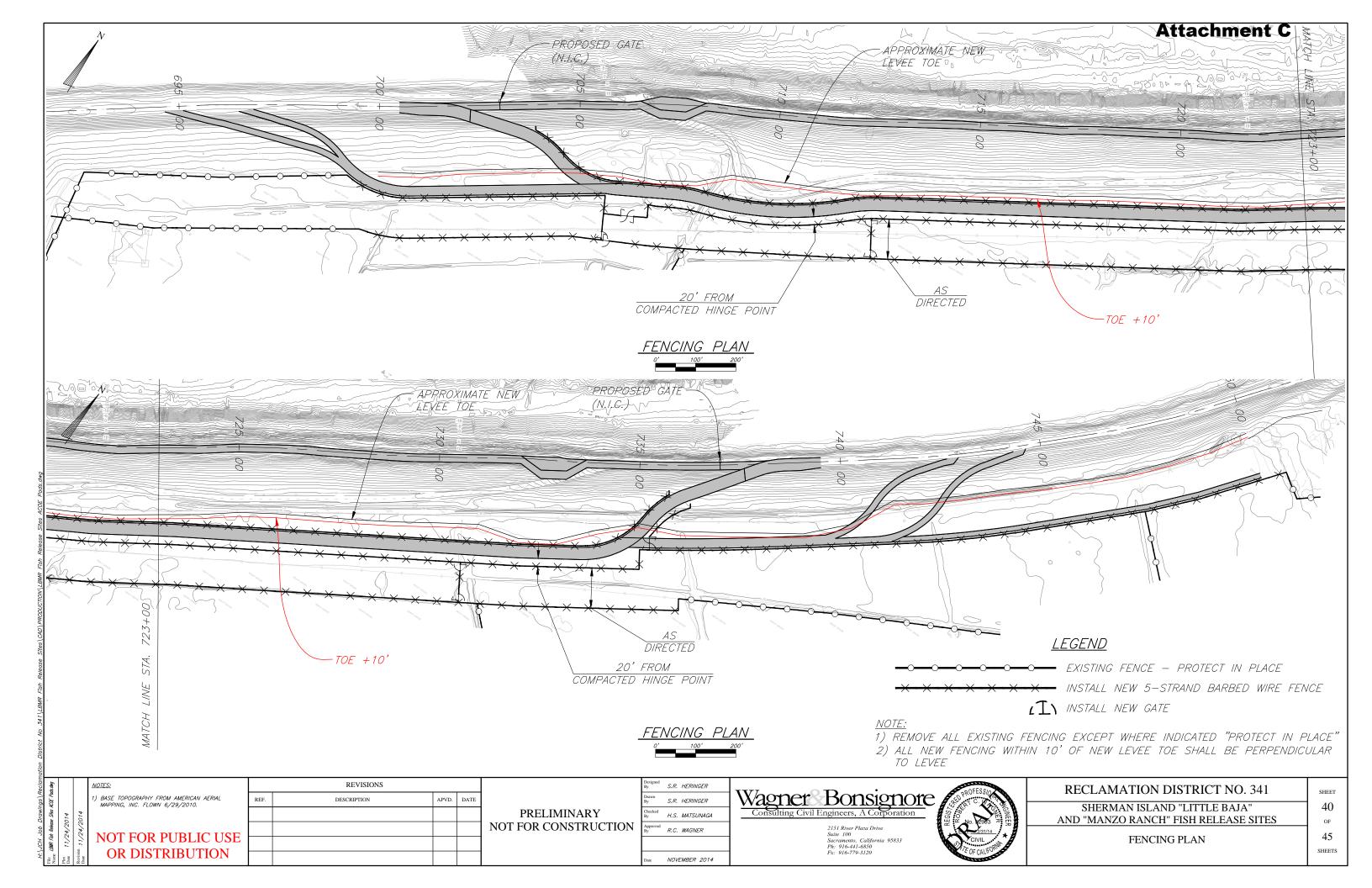
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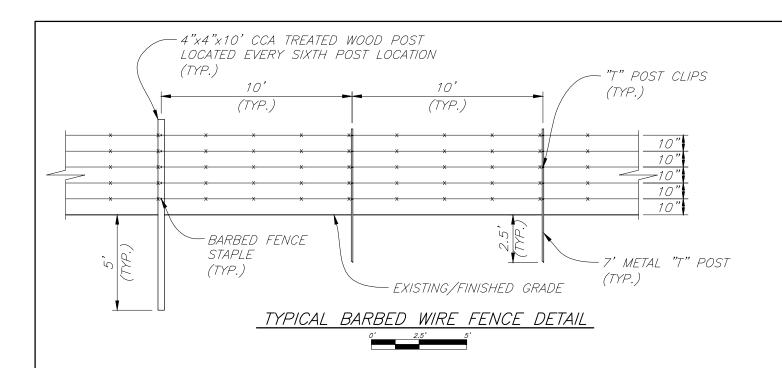
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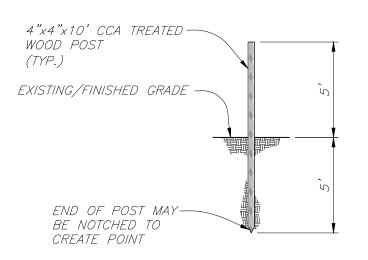
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EXISTING SIPHON AT ≈ 741+51 PROFILE AND SECTIONS





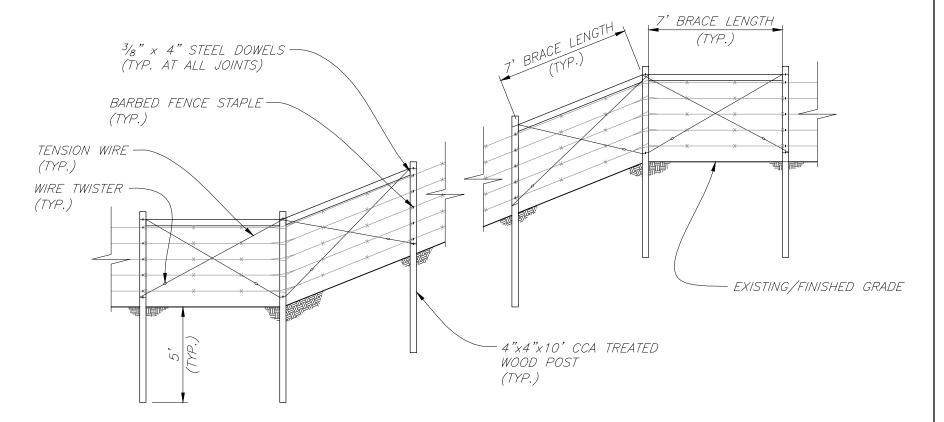




TYPICAL WOOD POST INSTALLATION DETAIL

0' 2.5' 5'

NOTE:
4"ø x 10' GALVANIZED PIPE MAY
BE SUBSTITUTED FOR WOOD
POSTS AT CONTRACTORS OPTION.
ALL CUTS AND WELDS SHALL BE
COLD SPRAYED GALVANIZED.



TYPICAL FENCE ON SHARP GRADE BREAK DETAIL



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RECLAMATION DISTRICT NO. 341

SHERMAN ISLAND "LITTLE BAJA"

**Attachment C** 

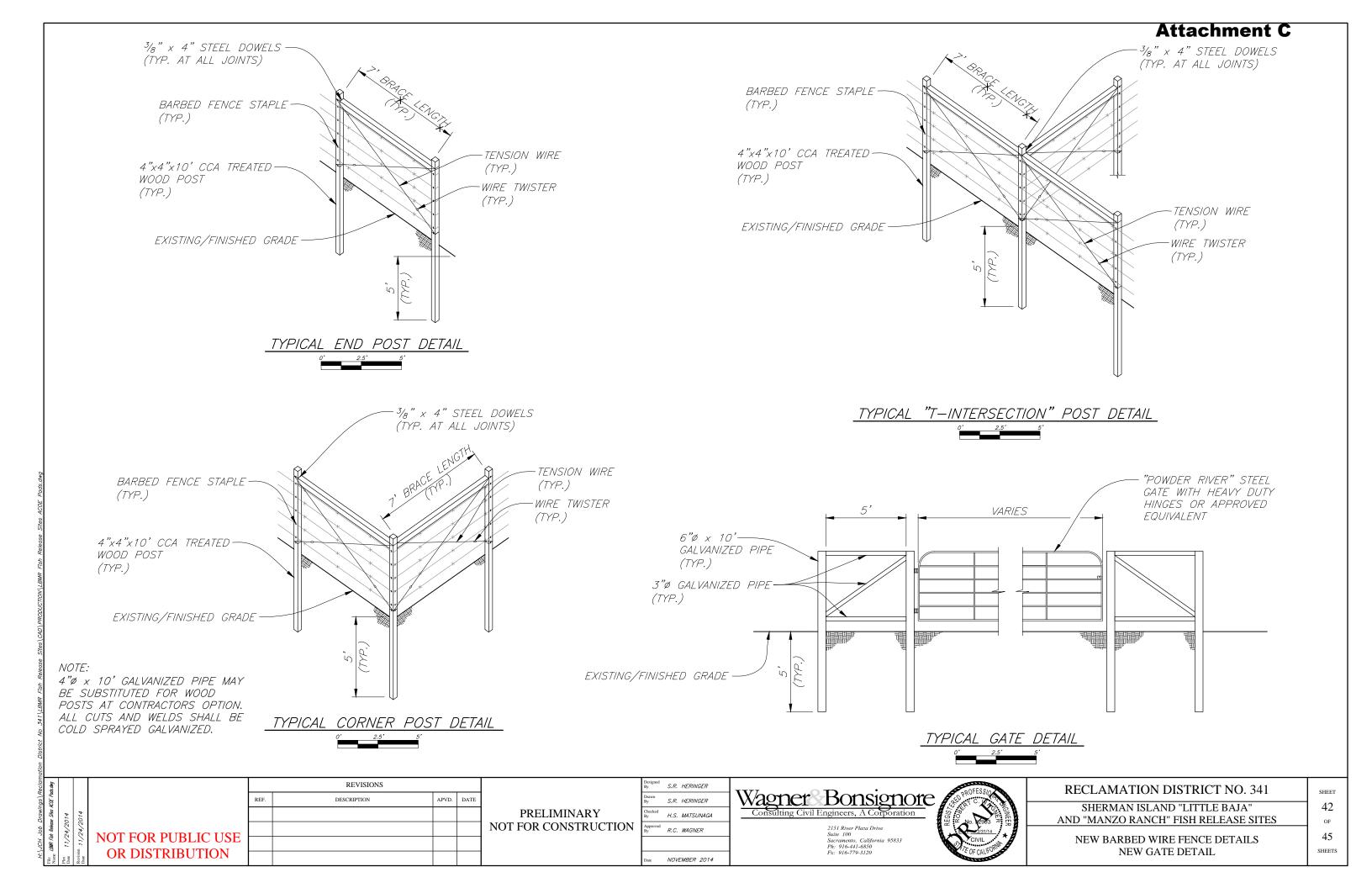
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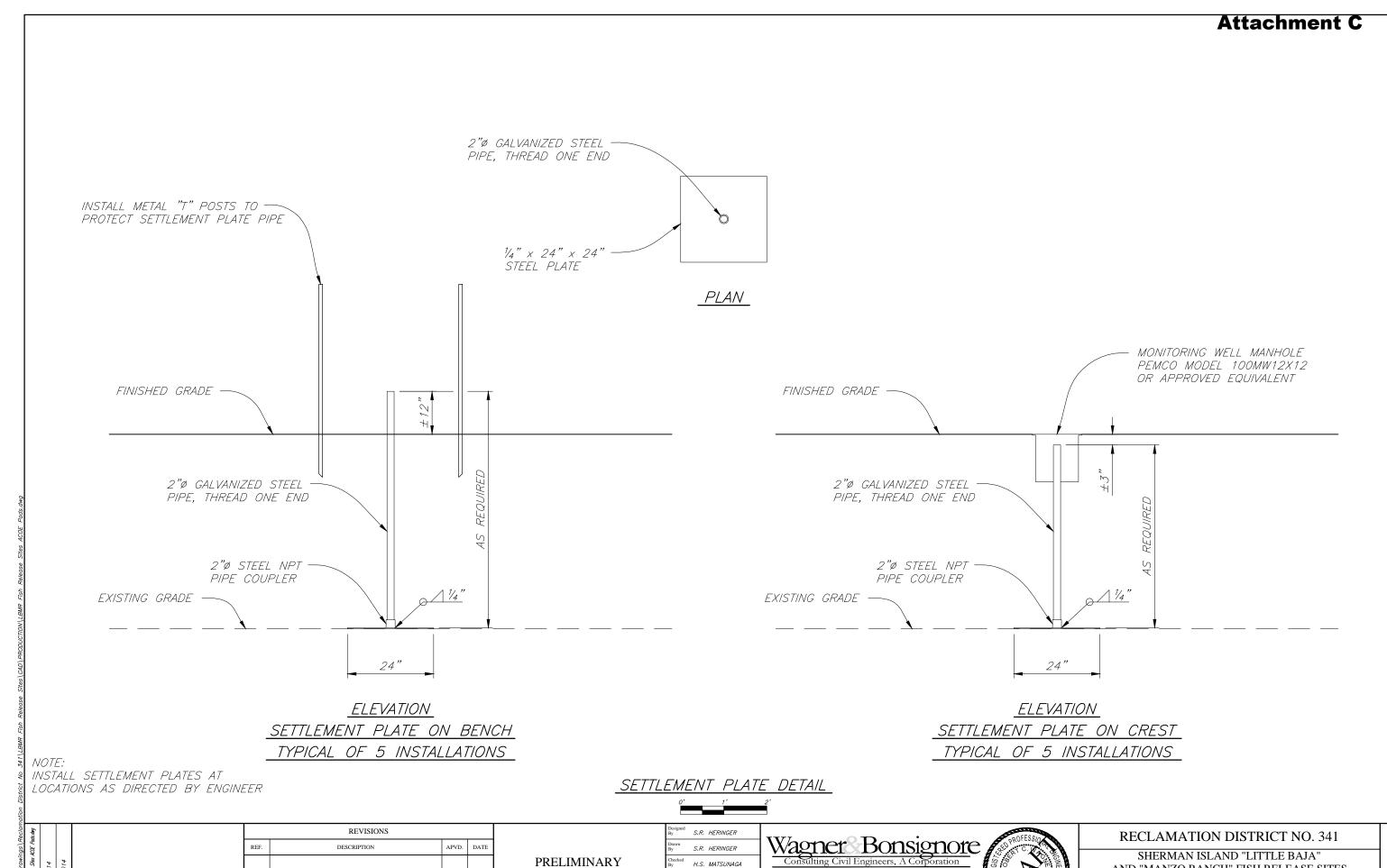
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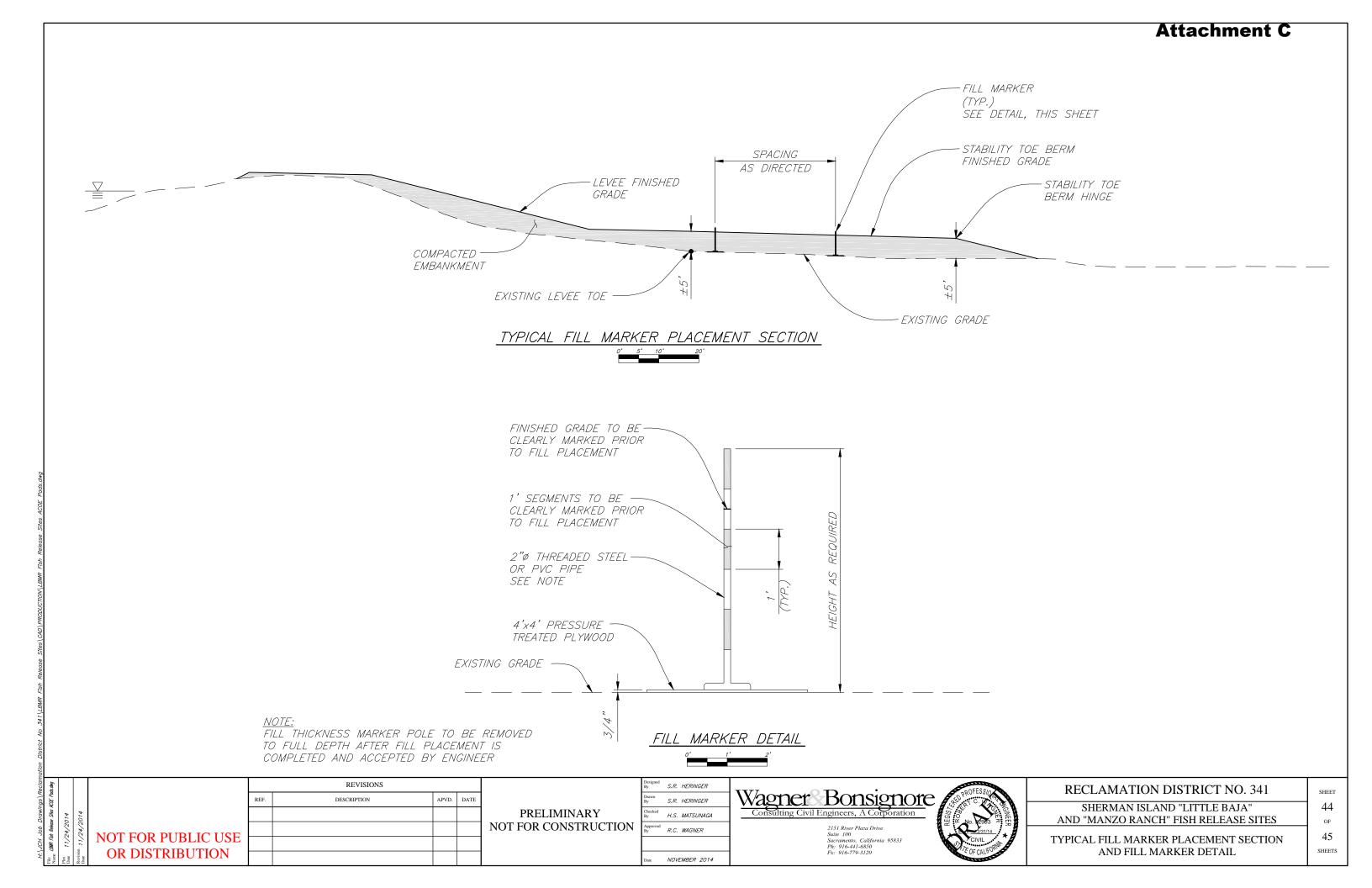
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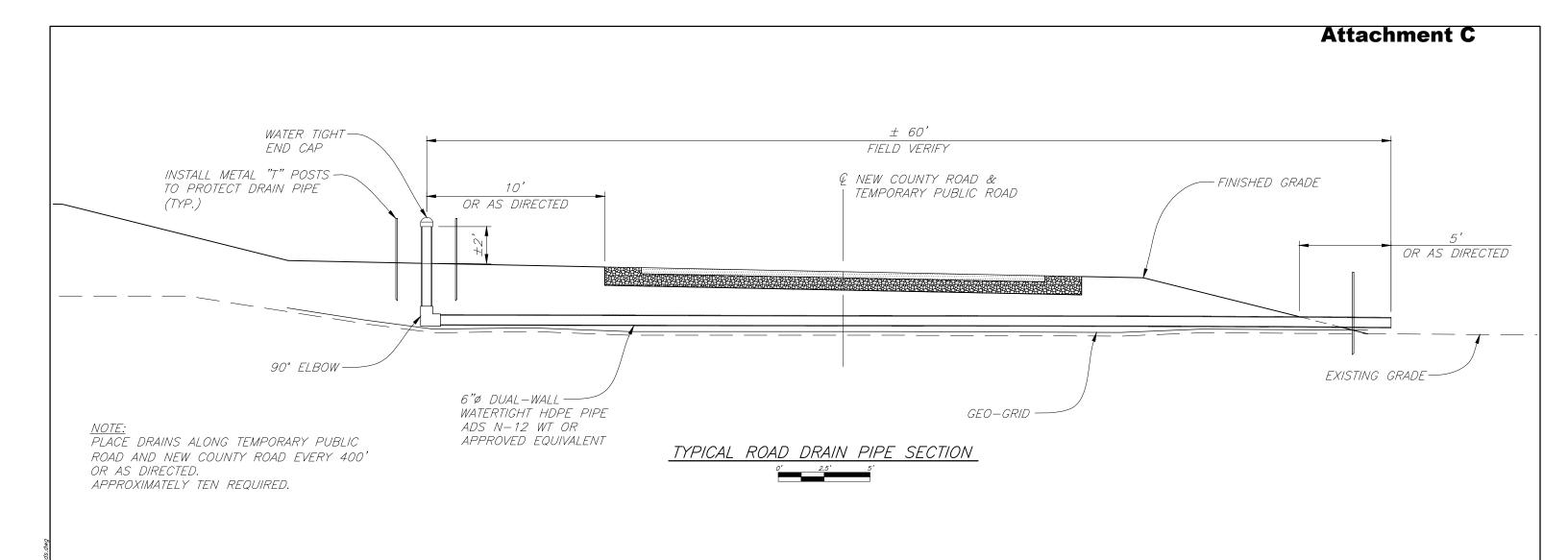
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AND "MANZO RANCH" FISH RELEASE SITES

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#### RECLAMATION DISTRICT NO. 341

SHERMAN ISLAND "LITTLE BAJA" AND "MANZO RANCH" FISH RELEASE SITES

TYPICAL ROAD DRAIN PIPE SECTION

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45

OF

45 SHEETS



## **GEOTECHNICAL EVALUATION**

SHERMAN ISLAND – STATIONS 700 TO 850 RECLAMATION DISTRICT 341 SACRAMENTO COUNTY, CALIFORNIA

Project No. 789.06 November 11, 2014

Prepared by

**Hultgren – Tillis Engineers** 

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#### I. INTRODUCTION

This report presents the results of our geotechnical evaluation for levee rehabilitation for Reclamation District 341 (District) on Sherman Island in Sacramento County, California. The District plans to improve the erosion protection on the waterside slopes, widen the county road on the levee crest and construct other slope modifications to meet the Project Levee requirements between Stations 700 and 850. As part of the modifications the existing county road between District Stations 707 and 733 will be realigned. The limits of levee rehabilitation for this project extend from District Stations 700 to 850, covering approximately 2.8 miles of levee located along the Sacramento River on the northwest side of Sherman Island. This levee reach is a U.S. Army Corps Engineers (USACE) project levee and is part of the State Plan of Flood Control facilities and subject to permitting through the Central Valley Flood Control Board (CVFCB). The existing levee crest serves as a two-lane county road, except the reach between Stations 707 and 733 where the county road is located at the landside slope toe. The California Department of Water Resources (DWR) plans to construct and operate two new salvaged fish release facilities at Stations 707 and 733. The levee between the two fish release sites will be improved as part of the fish release facilities construction. This report addresses levee rehabilitation between Stations 700 to 850. Our geotechnical evaluation for the fish release sites are presented in a separate report.

The criteria for this levee reach is outlined in the Operation and Maintenance (O&M) Manual for Reclamation District No. 341 dated February 1962. The O&M Manual indicates that the design prism for the reach is a 20 foot wide crest with maximum landside and waterside slopes of 2H:1V (horizontal to vertical) and 3H:1V, respectively. The freeboard requirement is 3 feet above the adopted design flood level. The adopted flood level for this reach is Elevation +11.7 feet North American Vertical Datum of 1988 (NAVD88).

Sherman Island and the surrounding areas are shown on the Vicinity Map, Plate 1. The project area and District stationing are shown on the Site Plan, Plate 2. The elevations in this report are referenced to the NAVD88. However, some of the published data (Geologic map and previous boring/drill logs) were recorded using the National Geographic Vertical Datum of 1929 (NGVD29). A correction of +2.4 feet may be used to convert from NGVD29 to NAVD88 elevations for this project by the design team.

Our scope of services was outlined in our proposal dated October 9, 2013. Our scope of services consisted of reviewing existing geotechnical data, performing engineering analysis, and developing conclusions and recommendations regarding geotechnical aspects of the project. The results of our geotechnical investigation are presented in this report.

#### II. FIELD EXPLORATION AND LABORATORY TESTING

#### A. Existing Data

As part of our evaluation of the levee, we reviewed data from two sources: The Salinity Control Barrier Investigation by DWR in the 1950's; and the New Salvaged Fish Release Facilities project. DWR collected subsurface data for the Salinity Control Barrier Investigation project between 1955 and 1958 along the perimeter levee of Sherman Island. Data relevant to the project area includes seventeen (17) borings drilled on the levee crest. DWR recently performed subsurface exploration for the planned two fish release sites and presented the subsurface data in the Project Geology Report 80-10-39 dated January 2014. Subsurface data included six (6) soil borings, fifteen (15) cone penetration test soundings (CPT), two (2) field vane shear tests, and associated laboratory testing results. The laboratory testing performed by DWR included sieve analysis, Atterberg limits, moisture and dry density measurements, organic content, specific gravity, consolidated undrained triaxial compression tests, and a consolidation test.

The DWR 1950's data is presented in Appendix A, including the Site Plan, Plate A-1; the Profile of Borings (graphic logs), Plate A-2; the Legend to Borings, Plate A-3; and the boring/drill logs of borings 100 through 116, Plates A-4 through A-53. The Logs of Borings and CPT's from the Project Geology Report are presented in Appendix B. The approximate boring locations are presented on the Site Plans, Plates 1 and 2 in Appendix B.

The USACE performed a levee rehabilitation as shown on plans from 1954. The rehabilitation included raising the levee to Elevation +16.7 (NAVD88) with a 20 foot wide crest.

#### B. Hand Auger Borings

To gain information on the existing fill between Stations 707 and 733, we explored shallow subsurface conditions with a hand auger. We explored subsurface conditions on March 28 and March 31, 2014 by hand augering 12 borings to depths of 4 to 8 feet below existing grade. The approximate boring locations are shown on the Site Plan in Appendix E, Plate E-1. We collected samples from the soil cuttings.

Our engineer logged the borings and recovered samples from the borings for further visual classification and for selection of materials for laboratory testing. The laboratory

testing program consisted of moisture content and dry density measurements, Atterberg limits and sieve analysis.

Soil descriptions and the laboratory test data are shown on the Logs of Boring, Plates E-2 through E-13. The soil descriptions are presented in general accordance with the Soil Classification System presented on Plate E-14 with laboratory test results presented in the manner described by the Key to Test Data. Atterberg limits test results are shown on Plate E-15. Sieve analysis test results are shown on Plates E-16 and E-17.

#### III. SITE CONDITIONS

## A. Geology

The United States Geological Survey (USGS) has published geologic maps for the Sacramento-San Joaquin Delta (Atwater 1982). The Atwater geologic map that includes Sherman Island and the geologic descriptions of the map units is presented on Plate 3. The geologic map shows that the island is mostly covered by peat and mud (Qpm) except one relatively small area (between approximately Stations 755 and 805) along the Sacramento River where hydraulic fill (Qds) blankets the surface.

The present configuration of the Sacramento-San Joaquin River Delta began to form after the last ice age, about 10,000 years ago. During the ice age, sea levels were 200 to 300 feet below present levels. Sea levels rose rapidly for several thousand years then the rate of sea level rise slowed. As sea levels rose, the Delta was inundated. The sea level rise was slow enough to allow for the accretion of marsh vegetation and formation of a widespread peat marsh (mapped as Qpm).

The peat continued to accumulate as sea levels rose. The marsh formation was halted upon reclamation of land in the late 1800's and early 1900's within the Delta.

The reclamation of the Delta allowed the peat materials to dry. The process of drying an organic material such as the peat causes it to oxidize and deplete. The ground surface within Sherman Island has subsided since the island was reclaimed predominately due to soil oxidation and disappearance of the peat. Oxidation continues to occur throughout the Delta.

#### B. History

Based on notes shown on the 1950's DWR drill logs (Appendix A), landside slope instability and seepage were observed at several locations from approximately 200 feet west of Boring 100 (about Station 697) to Boring 104 (about Station 728). The drill logs indicated observations of slope failures and active seepage areas at the levee toes and in the fields (up to 300 yards measured from the levee toe). The DWR borings were drilled in 1956, about 2 years after the USACE rehabilitation.

The existing county road is located at the landside toe between Stations 707 and 733. While we do not have records indicating the reason for moving the road off the levee crest, the location is coincident with the reported instability and seepage. The existing levee has been widened several times since the 1950's and a 4H:1V slope has been used by the District. No visual evidence of seepage or instability was noted during our field visit and no seepage or slope failure has been reported recently.

#### C. Surface Conditions

Sherman Island comprises approximately 10,000 acres of agricultural land and is protected by 19.4 miles of levee. The survey data indicates that the centerline of the levee crest varies from a low of Elevation +12.1 feet at Station 734 to a high of Elevation +17.6 feet at Station 846. The levee crest width varies and generally ranges from 16 feet to 30 feet. A two-lane county road consisting of asphalt concrete pavement covers the levee crest except the reach from Stations 707 to 733. Beside the county road, numerous structures are located along the levee including houses between Stations 765 and 800, an RV park between Stations 755 and 765, a trailer park between Stations 752 and 755, a pump station and associated pipelines at Station 811, trees, brush, and overhead utility lines.

The island interior near the levee reach varies from Elevation -10 feet to +11 feet. The island interior is lower between Stations 700 and 740 and ranges from Elevation -7 feet to -10 feet. The landside slopes are steepest between Stations 700 and 740 and incline at about 2.5H:1V to 3H:1V. The rest of the landside slopes are flatter with typical inclinations near 4H:1V or flatter. In the house and RV park areas, the island interior is relatively high ranging from Elevation -2 feet to +11 feet and the slope inclination is 10H:1V or flatter. In the area to the north of the house area (from Stations 800 to 850), the island interior ranges from Elevation -3 feet to +1 feet. The landside levee slope and toe are covered by vegetation that consists of grass and brush. Trees and dense brush are located on the landside and waterside levee slopes at several locations. Drainage ditches are present beyond the existing levee toe.

The waterside slope generally varies in inclination from 2H:1V to 5H:1V. Between Stations 761 and 765, the lower portion (below Elevation +0 feet) of the waterside slope is steeper with an inclination of approximately 1H:1V to 1.5H:1V. The mudline near the waterside slope is generally between Elevation -14 feet to -30 feet. Two depressions exist near Stations 763 and 847, respectively, with the base of the depression at approximately Elevation

-50 feet. The upper portion of most of the waterside slope is not covered by riprap. The riprap has eroded and slumped over portions of the reach. Erosion within the waterside slope has occurred.

The typical daily tidal range at Sherman Island varies from about Elevation +2 feet to +6 feet. Extreme low tides are near Elevation +1 foot and extreme high tides are near Elevation +7 feet. The 100-year base flood level used by the District and the design team is Elevation +9.8 feet.

#### D. Subsurface Conditions

The levee consists of fill over marsh deposits. The upper portion of fill below the levee crest consists of asphalt concrete pavement over road base (gravel). Beneath the road base is a heterogeneous mixture of fill consisting predominately of silty sand, lean clay, and clayey sand. The consistency of the silty sand fill ranges from very loose to very dense. The fill thickness generally ranges from 15 to 25 feet at the boring locations.

The fill is underlain by marsh deposits, except between Stations 760 and 845, where zones of sand were encountered beneath the levee fill. No standard penetration tests (SPTs) were performed during the DWR 1950's investigation. The consistency of the sand beneath the levee fill is unknown. The marsh deposits consist predominately of weak and compressible peat and organic silt. The base of the marsh deposits range from Elevation -40 feet to -62 feet. The marsh soil beneath the levee is much thicker than below typical levees in the Delta. Our interpretation of the elevation of the base of marsh deposits beneath the levee is presented on Plate 4.

The material beneath the marsh deposits varies with several locations underlain by sand to the depths explored and with other locations underlain by variable zones of silt and sand. The consistency of the sand is variable and ranges from loose to dense. The loosest sand is typically located directly below the marsh soils.

The island is below sea level and groundwater levels within the island are artificially controlled by evapo-transpiration and pumping. Conditions are expected to vary across the site with time, and depend on several factors including changes in moisture content resulting from seasonal precipitation, irrigation practices, and tides.

#### IV. DISCUSSION AND CONCLUSIONS

#### A. General

The significant geotechnical considerations for the levee reach are lack of erosion protection for waterside slopes, the presence of marsh soils below the levee, and ongoing settlement of the levee crest.

The District's focus is to improve the erosion protection for the waterside slopes along with evaluating the levee for general conformance with the PL 84-99 design standard. The levee is well below the adopted standard in the O&M Manual. We focused on evaluating an interim level of protection for the levee that is consistent with the PL 84-99 standard that includes 3 feet of freeboard above the 100-year flood. The 100-year flood level used by the District is at Elevation +9.8 feet. The O&M flood level is 11.7 feet. It is not clear how the O&M level was developed by the USACE but it is well above historic levels within the Delta. The intent of our analysis is to provide for an interim level of improvements (first phase) for the levee. The ground has subsided since the 1954 construction and the levee is much taller now. The funding is not available to raise the levee to Elevation +16.7 feet and meet the O&M requirements. The District plans to raise the levee to at least 12.8 feet and provide 3 feet of freeboard above the 100-year flood level.

DWR plans to construct two fish release facilities within this levee reach (Stations 707 and 733). This report includes recommendations for levee rehabilitation for Stations 700 to 850 and includes the levee rehabilitation at the two fish release sites. Specific recommendations for the fish release facilities are presented in a separate report.

The existing county road along the levee toe will need to be relocated to accommodate the levee rehabilitation. We understand that the county road is located along the levee toe from about Stations 707 to 733 due to past concerns with the performance of the levee. The DWR borings from the 1950's described the levee as having landside slope failures and described the presence of numerous active seeps and slope failures.

The levee has been widened and is not showing evidence of seepage or instability. The exact cause of the previous slope movement is not known. Extensive filling was required for the USACE improvements shown in the 1954 plans. The fill may have overstressed

the peat and caused failure of the levee slopes.

The rehabilitation for the levee reach should include provisions that recognize past concerns. The seepage analysis indicates that the exit gradients at the levee meet the USACE criteria for levees. The use of flatter slopes and a toe berm will provide protection from high seepage through the levee and from slope movements.

The levee on Sherman Island is founded on weak marsh deposits consisting predominately of peat and organic soil. The presence of the marsh deposits has a significant effect on the safety of the levee relative to levees constructed on firm soil. The presence of the marsh soil requires consideration of principles for design and construction on soft ground. Routine practices include broad landside berms to buttress slopes, construction in stages, and allowing for long-term settlement and deformation. Peat exhibits creep (long-term deformation under sustained loading) that must be considered in design. Levees on marsh deposits continually settle requiring successive episodes of filling to raise and maintain the minimum crest elevation and crest settlement will be an on-going concern for this levee reach.

To buttress the levee between Stations 700 and 745, where the landside slope is steeper and the island interior is lower than the remainder of the levee reach, we conclude that a 150 feet wide toe berm measured from the new landside hinge is needed. The landside slope should be inclined at 4H:1V. Typical design configurations are presented on Plates 5 and 6. To fit the existing geometry, some adjustment to the toe berm dimensions may be needed.

#### B. Erosion

The waterside slope lacks adequate protection from erosion from wind-generated waves or boat wakes. The levee will be vulnerable during high wind events particularly during high tides and flood events. We conclude that the levee reach does not meet USACE criteria for protection of the waterside slope from erosion.

#### C. Seepage

We performed seepage analysis to evaluate the potential for underseepage and to compare with the USACE guidance criteria for underseepage. The gradients for the existing levee and rehabilitation configuration are at 0.19 or below. The calculated values are below the USACE criteria of 0.5 or lower.

A discussion of the seepage analysis and the results of our seepage analysis are presented in Appendix C. The seepage analysis results from the analysis are also presented in Table 1.

**Table 1: Seepage Analysis Summary Results** 

Station	Design Water Surface Elevation (ft)	Existing Levee		Rehabilitated Levee	
		Exit Y-Gradient (Avg.)	Seepage Flow	Exit Y-Gradient Seepage Flow	
		Slope Toe or / Flux (gpm)		Slope Toe or Berm Toe	Rate / Flux (gpm)
700	9.8	0.18	0.02	0.18	0.02
715	9.8	0.15	0.01	0.16	0.01
724	9.8	0.19	0.01	0.19	0.01
740	9.8	0.19	0.02	0.18	0.01
761	9.8	0.06	0.03	0.06	0.02
839	9.8	0.10	0.02	0.10	0.02

#### D. Slope Stability

We performed stability analysis to evaluate the safety of the levee for stability and to develop a rehabilitation scheme for the levee. The site is underlain by a thick deposit of marsh soil. The factor of safety for the landside slope is below the minimum USACE criteria of 1.4 for Stations 715 and 724 for the existing levee (effective stress). To increase the factor of safety, a toe berm is needed to provide a buttress for the levee. The toe berm was used in the analysis for Stations 715, 724 and 740. The factor of safety is above 1.4 for the rehabilitated levee. The analysis indicated that a toe berm is needed from Stations 700 to 745 and a uniform slope inclined at 4H:1V meets the criteria for the remainder of the reach.

A discussion of the stability analysis and the results are presented in Appendix D. The factors of safety from the analysis are also presented in Tables 2 through 4.

**Table 2: Factors of Safety for Landside Slopes** 

Station	Existing Levee		Rehabilitated Levee			
	Effective	Undrained	End of Construction	Long Term	Consolidated	
	Strength	Strength	Undrained Strength	Effective Strength	Undrained Strength	
700	1.85	2.10	1.95	1.99	2.06	
715	1.39	1.66	1.63	1.62	1.83	
724	1.21	1.48	1.55	1.44	1.68	
740	1.60	1.75	1.78	1.83	2.12	
761	2.01	2.04	2.52	2.46	2.55	
839	2.27	2.57	2.46	2.62	2.52	

**Table 3: Factors of Safety for Waterside Slopes** 

Station	Existing Levee		Rehabilitated Levee			
	Effective	Undrained Strength	End of Construction	Long Term	Consolidated	
	Strength		Undrained Strength	Effective Strength	Undrained Strength	
700	1.84	1.58	1.57	1.83	1.57	
715	1.59	1.42	1.39	1.58	1.42	
724	1.57	1.40	1.40	1.57	1.40	
740	2.20	1.89	1.85	2.18	1.87	
761	1.68	1.58	1.58	1.68	1.58	
839	3.12	2.50	2.50	3.12	2.50	

Table 4: Yield Coefficients (K<sub>v</sub>) from Pseudo-Static Loading

Station	Land	Iside	Waterside		
	Existing Levee	Rehabilitated Levee	Existing Levee	Rehabilitated Levee	
700	0.16	0.15	0.09	0.09	
715	0.12	0.12	0.09	0.09	
724	0.11	0.11	0.08	0.08	
740	0.15	0.16	0.14	0.14	
761	0.28	0.28	0.12	0.12	
839	0.15	0.15	0.12	0.12	

#### E. Settlement and Lateral Deformation

The levee is underlain by deep marsh deposits. The marsh deposits are highly compressible. We performed analysis to estimate settlement of the levee based on the theory of one-dimensional consolidation. The ground settles from a combination of primary consolidation and secondary compression (creep). Primary consolidation occurs from compression of the marsh soils beginning when weight is placed on the soil. The initial weight is transferred to the water within the soil. The water builds up pressure causing flow to occur. As the water flows out of the soil, the soil structure compresses and continues to compress until the water flow is complete and the water pressure returns to hydro-static levels.

Secondary compression is deformation without flow of water. With most soils, the amount of secondary compression is small relative to the primary consolidation and is not a concern. With peat, secondary compression is a significant phenomenon and is a primary cause of long-term settlement of Delta levees and their loss of freeboard. The secondary compression will continue for many years at a diminishing rate with time.

We performed analysis at five stations and our estimate of settlement at each station is presented in Table 5. The estimates are provided for settlement expected within 10 years after fill placement. The settlement estimates are based on raising the crest 12-inches above the minimum crest level.

**Table 5: Settlement Estimate - Crest Centerline** 

Station	Settlement within 10 years (feet)
700	0.6 to 0.9
715	1.4 to 1.7
724	0.5 to 0.7
740	0.8 to 1.1
761	0.5 to 0.7
839	0.3 to 0.5

The toe berm and slopes will also settle under the weight of new fill. We estimate the toe berm will settle 2 to 3 feet under the weight of 5 feet of fill. We estimate that the landside edge of the toe berm will settle about 1.0 to 1.3 feet under the weight of 2 feet of fill.

Deformation of the levee can lead to cracking in the levee crest and slopes. Cracks are prevalent throughout the Delta levee system. The cracking is undesirable and, coupled with an inadequate seepage barrier (core), will be a continuing concern for the levee. Deformation cannot be avoided and cracking should be expected.

#### F. County Road

The new county road alignment is within the interior of the island and will be founded on the marsh deposits. We conclude that a layer of geogrid should be placed and covered with at least 2 feet of fill prior to placement of the aggregate base. The road will settle under the weight of fill and paving materials. We estimate settlement in the range of 1.5 to 2 feet.

#### G. Existing Vegetation and Encroachments

As a general practice, trees, brush, heavy vegetation, and encroachments located within the footprint of the levee is undesirable. After trees die, the root system decays and may leave a void. The active or decayed root system of a tree could provide a convenient path for seepage to flow through a levee. Trees and other dense vegetation make it difficult to inspect levees and can obscure problems with the levee. The vegetation also makes it difficult to repair or rehabilitate the levee because the vegetation must be removed first. We conclude that trees and dense vegetation should be removed from the footprint of the levee, including the toe berm, and not be allowed in the future.

#### V. RECOMMENDATIONS

### A. Levee Configuration

We recommend that the design generally conform to the configurations and details shown on Plates 5 and 6. The design of the levee should include provisions for raising and widening as needed in the future to meet the criteria for the levee established in the O&M Manual. The design shown on Plates 5 and 6 is intended for phase 1 of these improvements. We recommend that the levee rehabilitated configuration consist of landside slopes inclined at 4H:1V or flatter. Between Stations 700 to 745, the levee rehabilitated configuration should consist of landside slopes inclined at 4H:1V with a toe berm extending at least 150 feet from the final landside hinge point. The toe berm thickness and inclination may need to vary to account for variation in existing slopes, levee toe elevations, and depth of marsh deposits.

The design of the riprap is not in our scope of work. The slopes need to be covered with riprap to provide protection from wind-generated waves and boat wakes and meet USACE criteria. The toe berms should be constructed before the levee crest is raised or slopes graded.

#### B. Earthwork

#### 1. Site Preparation

The footprint of the levee and toe berm should be cleared and grubbed of surface and subsurface deleterious matter including trees, grasses, other vegetation, and debris designated for removal. The site should be stripped to sufficient depth to remove vegetation and soil containing roots. Tree roots greater than 1-inch in diameter should be removed. Stripped and grubbed materials should be removed from the site and should not be used as fill. The existing asphalt concrete and aggregate base should be removed from the crest where new fill is planned for the crest. Where possible, the existing asphalt concrete and aggregate base may be reused on the levee crest to reduce the thickness of new imported aggregate base if allowed by the District and the County. In areas where existing ditches are within the planned toe berm, the ditches should be filled and new ditches should be located with a minimum setback of 20 feet landside of the new toe berm. Soft sediment in the existing ditches should be removed prior to placement of fill.

#### 2. Compaction

Surfaces exposed by stripping or excavation should be scarified to a depth of at least 8-inches except where peat is exposed. Scarification is not required where peat exists at the ground surface. The scarified soil should be moisture conditioned to at least optimum moisture content and compacted to at least 95 percent relative compaction.

ASTM D698 should be used to establish the reference values for computing optimum moisture content and relative compaction.

Fill should be placed in lifts 8-inches or less in loose thickness and moisture conditioned to between one percent below optimum moisture content and 3 percent over optimum moisture content. Moisture conditioning should be performed prior to compaction. Each lift should be methodically compacted to at least 95 percent relative compaction. Material that fails to meet the moisture or compaction criteria should be loosened by ripping or scarifying, moisture conditioned, and then recompacted. Fill should be placed on horizontal surfaces. The fill should be benched into the existing landside levee slope to allow recompaction of some of the existing soil. The horizontal bench width into the existing slopes should not exceed 5 feet.

In pavement areas subject to vehicle traffic, the upper 6-inches of subgrade should be compacted to at least 95 percent relative compaction and rolled to provide a smooth, non-yielding surface. Subgrade soils should be proof-rolled before placing aggregate base. Proof-rolling should be performed with the heaviest available rubber-tired construction equipment and should be observed by the geotechnical engineer. Soft or pumping areas should be aerated or excavated and recompacted.

On the levee crest and ramps, the upper 6-inches of subgrade should be compacted to at least 95 percent relative compaction and rolled to provide a smooth, non-yielding surface. Subgrade soils should be proof-rolled prior to placing aggregate base. Soft or pumping areas should be aerated or excavated and recompacted.

Aggregate base should be placed in thin lifts no greater than 8-inches in loose thickness and in a manner that avoids segregation, moisture conditioned as necessary, and compacted to at least 95 percent relative compaction.

#### 3. Slopes

Fill slopes should be inclined at 4H:1V or flatter except as noted. Fill slopes should be constructed fat and trimmed back to expose well-compacted fill. Finished slopes and toe berms should be trackwalked perpendicular to the slope face with a bulldozer after completion. The slopes should be hydroseeded to promote vegetation. Vegetation should be limited to grasses or other vegetation that can be mowed or disced to allow inspection of the landside levee slope. Trees, bushes, and brush should not be allowed within the footprint of the levee slopes and toe berms.

## 4. County Road and Levee Toe

Prior to construction of the county road, new fill 2 feet thick should be placed 5 feet beyond the footprint of the roadway. The fill should be compacted as described in the Earthwork section of this report. The county road should be supported on a layer of geogrid placed below the fill. The geogrid should be Tensar BX 1200 or equivalent.

#### 5. Fill

The existing fill is a mixture of materials. We recommend that new fill conform to the typical fill requirements from the CVFCB. Fill placed at the site should be a soil or soil/rock mixture free of deleterious matter and contain no rocks or hard fragments larger than 4-inches in maximum dimension with less than 15 percent larger than 1-inch in maximum dimension. The fill should have at least 20 percent passing the No. 200 sieve. The fill should have a liquid limit below 50 and a minimum plasticity index of 8.

## C. Geotechnical Services During Construction

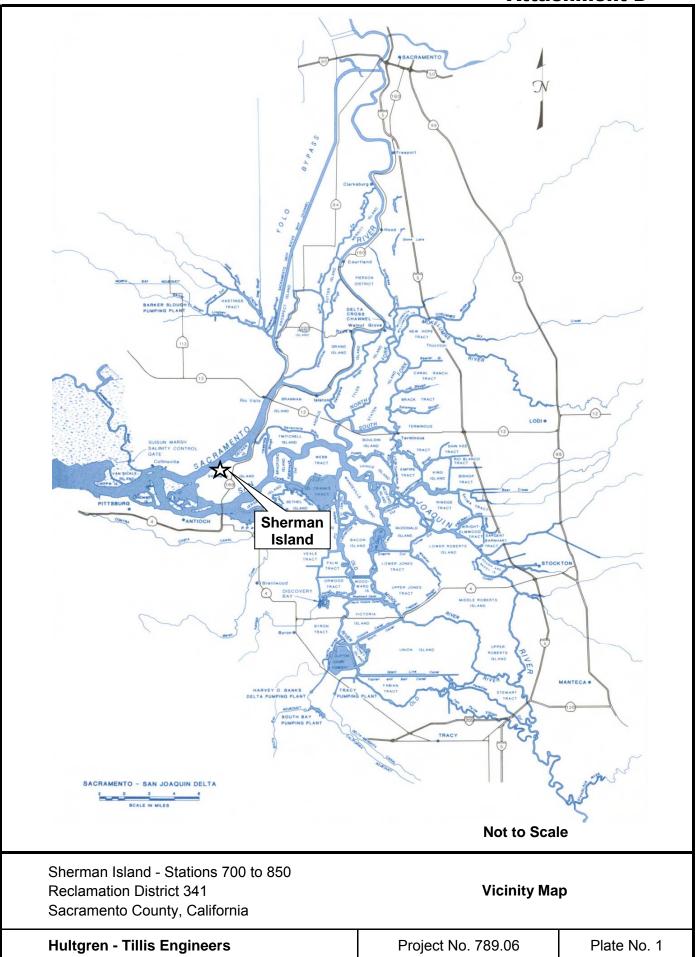
Before construction, we should review project grading plans and specifications for conformance with the intent of our recommendations. During construction, we should observe and/or test the geotechnical aspects of grading including but not limited to subgrade preparation, placement and compaction of fill, and finish grading. If conditions are encountered during construction that are not consistent with those described herein, we should be contacted to review our recommendations and provide alternatives, if appropriate.

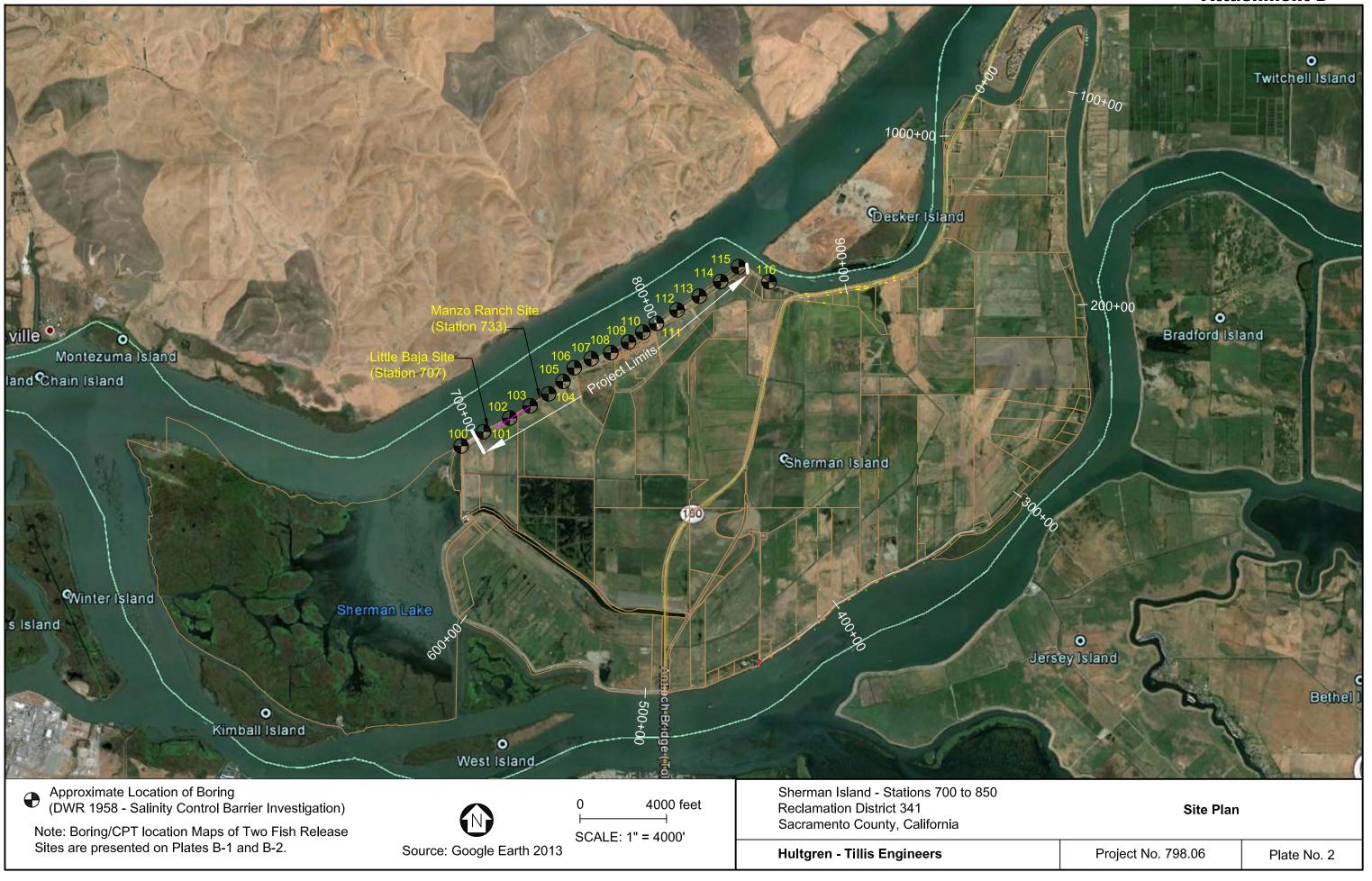
**REFERENCES** 

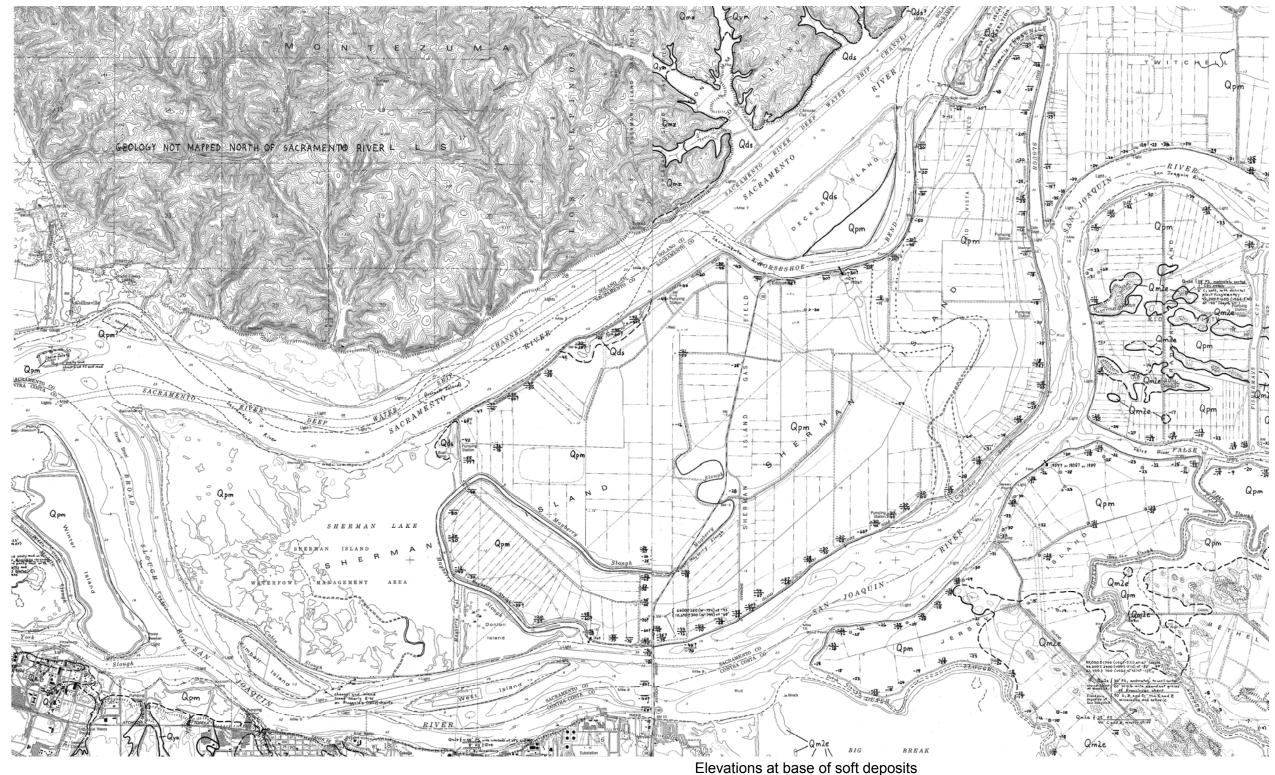
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**PLATES** 







**Not to Scale** 

Qds: Hydraulic-dredge soils (Holocene; post-1900)

Peat and mud of tidal wetlands and waterways (Holocene)
Eolian deposits of upper member of Modesto Formation (Upper Pleistocene) Qpm:

Qm2e:

Younger alluvium of Montezuma Hills and vicinity (Holocene)
Youngest alluvium of Antioch and vicinity (Holocene)
Montezuma Formation (Pleistocene) Qym: Qya:

Qmz:

Source: Map by Brian F. Atwater 1982.

Basal deposits peat or peaty mud

<u>-11</u>: -16: Basal deposits mud

Soft deposits missing; firm or stiff deposits at ground surface at indicated elevation (-1):

Sherman Island - Stations 700 to 850 Reclamation District 341 Sacramento County, California	Geologic N	Лар
Hultgren - Tillis Engineers	Project No. 789.06	Plate No. 3

