Meeting of the Central Valley Flood Protection Board January 28, 2011

Staff Report – USACE Project Transfer

American River Common Features (Remaining Sites) WRDA 1996 Contract 1 (Site R4), Sacramento County

1.0 – ITEM

Consider approval of Resolution No. 10-45 to accept completed American River Common Features Project from the U.S. Army Corps of Engineers (USACE) and transfer operation and maintenance responsibility to the Sacramento Area Flood Control Agency (SAFCA).

<u>2.0 – LOCATION</u>

The project is located approximately 250 feet south of the Strong Ranch Pump Station and 1,330 feet northwest of the Northrop Avenue west terminus as measured along the American River Bike Trail in Sacramento County.

3.0 – DESCRIPTION

American River Common Features Project, as authorized by the Water Resources Development Act of 1996 (WRDA 1996), consists of strengthening of 8.9 miles of the right (north) bank and 10.6 miles of the left (south) bank of the American River levee, and 12.1 miles of the Sacramento River east levee and some berm raising. Eighteen "windows" were left during the initial construction of the slurry walls along the American River. These windows exist near bridge abutments, deep underground utility lines and under low overhead utility lines along both the right and left bank levees. These windows are designated with either an "L" for left bank and "R" for right bank. The subsequent number designates its ordered position relative to the other similar project locations. This turnover letter is for site R4 only.

The work associated with Site R4 includes the installation of a reinforced concrete drainage channel outlet wall keyed into the existing slurry wall.

Jason Sidley, DWR

4.0 - PROJECT ANALYSIS

4.1 – Project Background

- 1. The USACE, with DWR and SAFCA as local sponsors, completed construction at Site R4 on September 1, 2009. The project work was performed under a California Environmental Quality Act Categorical Exemption, filed April 14, 2008 (State Clearinghouse Number 2008048211).
- 2. The USACE sent a letter, dated July 1, 2010 (Attachment A), alerting the Board that it is transferring the recently completed portion of work (Site R4) performed under the authority of WRDA 1996 and providing a turnover package which included the amendments to the Operation and Maintenance Manual (Attachment B) and the AsBuilt Drawings (Attachment C).
- SAFCA, in a letter dated January 19, 2011(Attachment D), notified the Board of the
 assurance of their acceptance of the completion of construction of Site R4, and its
 desire to accept operation and maintenance responsibility for said completed
 construction.

4.2 - Project Design Review

The project design and construction work was monitored by DWR staff throughout the project. The As-Built Drawings and the revisions to the Operation and Maintenance Manual were also reviewed by DWR staff.

The construction was completed in accordance with American River Common Features WRDA 1996 Remaining Sites Contract I (Site R4), Specification Number 1593, Design File Number 104-0514, Contract Number W91238-09-D-0020. As-Built Drawings and revisions to the Operation and Maintenance Manual are attached. Also, a copy of the "Written Notice of Acceptance of Completed Work" (Attachment E) is included as required by the Project Cooperation Agreement (PCA).

Jason Sidley, DWR 2

<u>5.0 – STAFF RECOMMENDATION</u>

Staff recommends that the Board adopt Resolution No. 10-45, which constitutes the Board's written findings and decision. The Resolution contains the following recommended actions:

- Approves the acceptance of the completed American River Common Features (Remaining Sites) WRDA 1996 Contract 1 (Site R4) from the USACE and authorizes Board Staff to issue a letter to the USACE (Attachment F) informing them of the same.
- 2. Approves the transfer of the operation and maintenance responsibility of the completed American River Common Features (Remaining Sites) WRDA 1996 Contract 1 (Site R4) to SAFCA and authorizes Board Staff to issue a letter to SAFCA (Attachment G) informing them of the same.
- 3. Delegates to the Board Executive Officer the authority to sign the letters to the USACE and SAFCA.

5.0 – LIST OF ATTACHMENTS

- A. Resolution 10-45
- B. Transfer Letter from USACE (Dated July 1, 2010)
- C. Amendment to Operation and Maintenance Manual American River Flood Control Project
- D. American River Common Features (Remaining Sites) WRDA 1996 Contract 1
 (Site R4) As-Built Drawings
- E. Letter from SAFCA accepting O&M responsibility for Site R4 (January 19, 2011)
- F. Written Notice of Acceptance of Completed Work
- G. Letter to be sent to USACE acknowledging transfer of Site R4 (Dated January 28, 2011)
- H. Letter to be sent to SAFCA transferring Operation and Maintenance responsibility to SAFCA (Dated January 28, 2011)

Design Review: Jason Sidley

Environmental Review: Environmental Staff Assigned Document Review: Kent Zenobia, Dan Fua

Jason Sidley, DWR

STATE OF CALIFORNIA CALIFORNIA NATURAL RESOURCES AGENCY THE CENTRAL VALLEY FLOOD PROTECTION BOARD

RESOLUTION NUMBER: 10-45

ACCEPTANCE OF AMERICAN RIVER COMMON FEATURES REMAINING SITES CONTRACT 1 (SITE R4) FROM THE U.S. ARMY CORPS OF ENGINEERS AND TRANSFER OPERATION AND MAINTENANCE RESPONSIBILITY TO THE SACRAMENTO AREA FLOOD CONTROL AGENCY (SAFCA)

WHEREAS, the Central Valley Flood Protection Board (Board) has entered into a Project Cooperation Agreement with the United States Army Corps of Engineers (USACE), Contract Number B81560, dated 9/9/1998, to cost share funding for the American River Common Features Project; and

WHEREAS, the Board has entered into a Local Project Cooperation Agreement (LPCA) with SAFCA, Contract Number B81559/165946, dated 9/9/1998, to cost share funding for the local share of the American River Common Features Project; and

WHEREAS, the LPCA identified SAFCA as the Local Maintaining Agency (LMA) for the American River Common Features Project; and

WHEREAS, construction of Site R4 was completed by the USACE in accordance with American River Common Features Water Resources Development Act 1996 – Remaining Sites Contract 1, Specification Number 1593, Design File Number 1-04-0514, Contract Number W91238-09-D-0020; and

WHEREAS, the USACE, in a letter to the Board, dated July 1, 2010, has notified the Board of the completion of construction of Site R4, and its desire to turn over to the Board such work, as completed, for operation and maintenance; and

WHEREAS, the LPCA states that the Board shall turn completed projects from USACE over to SAFCA for operation and maintenance; and

WHEREAS, SAFCA, in a letter dated January 19, 2011, has notified the Board of the assurance of their acceptance of the completion of construction of Section R4, and their desire to accept operation and maintenance responsibility for said completed construction.

NOW, THEREFORE, LET IT BE RESOLVED that the Board:

1. Approves the acceptance of the completed American River Common Features (Remaining Sites) WRDA 1996 Contract 1 (Site R4) from the

Page 1 of 2 January 28, 2011

USACE and authorizes Board Staff to issue a letter to the USACE informing them of the same.

- 2. Approves the transfer of the operation and maintenance responsibility of the completed American River Common Features (Remaining Sites) WRDA 1996 Contract 1 (Site R4) to SAFCA and authorizes Board Staff to issue a letter to SAFCA informing them of the same.
- 3. Delegates to the Board Executive Officer the authority to sign the letters to the USACE and SAFCA.

BY:	Date:
Benjamin F. Carter	
President	
BY:	Date:
Francis Hodgkins	
Secretary	
Approved as to Legal Form and Sufficiency	<i>'</i>
 Jeremy Goldberg	
Staff Counsel	

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DEPARTMENT OF THE ARMY U.S. Army Engineer District, Sacramento Corps of Engineers 1325 J Street Sacramento, California 95814-2922

Flood Protection and Navigation Section

JUL D 1 2010

Mr. Jay Punia, Executive Officer Central Valley Flood Protection Board 3310 El Camino Avenue, Room 151 Sacramento, California 95821

Dear Mr. Punia:

This letter is to transfer the U.S. Army Corps of Engineers (Corps) recently completed portion of work (R4) performed under the authority of the Water Resources Development Act of 1996 (WRDA 96). American River (Common Features) as authorized by WRDA 96 consists of lower American River levee strengthening of 8.9 miles of the right (north) bank and 10.6 miles of the left (south) bank levees, and 12.1 miles of Sacramento River east levee and berm raising. Eighteen American River "windows" exist where construction of separate cutoff walls and appurtenant structures around bridge abutments, deep underground utility lines and under low overhead utility lines for both right and left bank levees are required to ensure complete closure of the slurry wall. These "windows" are designated with either an "L" for left bank and "R" for right bank. The subsequent number designates its ordered position relative to the other similar project locations. This turnover letter is for site R4 only.

Site R4 is located approximately 250-feet south the Strong Ranch Pump Station and 1,330-feet northwest of the Northrop Avenue west end terminus as measured along the American River Bike Trail. The R4 project includes a new reinforced concrete drainage channel outlet wall keyed into the slurry wall built under a previous contract.

The construction was completed in accordance with American River Common Features WRDA 1996 – Remaining Sites Contract 1, Specification Number 1593, Design File Number 1-04-0514, Contract Number W91238-09-D-0020. As constructed drawings and revisions to the Operation and Maintenance Manual are enclosed. Also, a copy of the "Written Notice of Acceptance of Completed Work" is included as required by the Project Cooperation Agreement (PCA). Both hard-copy and electronic versions (CD) are provided.

This work meets the requirements of the existing Operation and Maintenance Manual American River - Part No. 1, Levee Construction from Carmichael Bluffs Downstream 8.3 miles; therefore, said flood control work is transferred as of the date of this letter to the State of California for operation, maintenance, repair, replacement, and rehabilitation (OMRR&R). This letter of acceptance into the Federal flood control system should not be construed as an endorsement for inclusion into the National Flood Insurance Program as outlined in Title 44 of the Code of Federal Regulations Section 65.10 of the National Flood Insurance Regulations (44 CFR Section 65.10).

If you have any questions regarding this project, please contact the Project Manager, Mr. John Hoge, at (916) 557-53042. If you have any questions regarding this transfer, please contact Mr. Ryan Larson at (916) 557-7568, Flood Protection and Navigation Section. A copy of this letter is being furnished to Mr. Stein Buer, Sacramento Area Flood Control Agency, 1007 7th Street, Sacramento, CA 95814.

Sincerely,

Thomas C. Chapman, P.E. Colonel, Corps of Engineers

District Engineer

Enclosures

OPERATION AND MAINTENANCE MANUAL AMERICAN RIVER FLOOD CONTROL PROJECT

AMERICAN RIVER – PART NO. 1 LEVEE CONSTRUCTION FROM CARMICHAEL BLUFFS DOWNSTREAM 8.3 MILES

REVISIONS OR ADDITIONS

	ADDITIONS	DATE
	ADDITIONS	DATE
Exhibit F	Add copy of letter of transfer dated 17 Nov 1958	23 Mar 2010
Paragraph 4-05. a.(4)	Slurry Wall Facilities added	01 Jul 2010
Paragraph 4-05. a.(4)(a)	Site R4 added	01 Jul 2010
Paragraph 4-05. b.(5)	Site R4 added	01 Jul 2010
Exhibit F	Copy of R4 Transfer letter added	01 Jul 2010
	REVISIONS	DATE
Paragraph 4-04. a.	Revised to include R4 structure	01 Jul 2010
Exhibit B	Revised to include R4 structure	01 Jul 2010

which become lodged against the bank shall be removed. The improved channel or floodway shall be thoroughly inspected immediately following each major high water period. As soon as practicable thereafter all snags and other debris shall be removed and all damage to walls, drainage outlets or other flood control structures repaired."

4-04. Drainage and Irrigation Structures.

a. Description. Drainage and irrigation structures which extend through the levees are listed as follows. **See drawing set 01-04-0415 for centerline stationing:**

Centerline Station	Bank	Size of Pipe	Other Structure Description	Elevation Invert of Pipe
33+55	Right	1-1/2"		60.6
37+58	"	1-1/2"		63.0
40+10	- 41	2-1/2"		
42+25	"	1-1/2"		62.0
44+46	"	1-1/2"		63.0
48+75	"	1-1/2"		63.0
49+50	"	1-1/2"		63.7
51+50	11	1-1/2"		63.0
201+75	"	8"	Gate valve R.S. crown	55.0
203+45	"	3-24" C.	Pumping plant #2	40.0
203+45	tt.	24" C.	Pumping plant #2	30.0
224+30	"	10"		44.7
227+70	16	6"		43.4
288+65	"	8"	Gate valve R.S. crown	52.0
296+85	tt.	12"	Riser unit R.S. C.E.S.	40.0
346+09	"	12"	Riser unit R.S. C.E.S.	36.0
365+14	11	36"CM.	Riser unit R.S.	31.8
425+30	66	7-48"C.	Pumping plant No. 1	31.0
425+30	"	36"C.	Pumping plant No. 1	18.0
38°35.0514'N		23'±	1-Double Box Culvert and 1-	4.45± DBC
121°25.2672'W	66	Total	Single Box Culvert Endwall and	4.73± SBC
(NAD 83)		Width	Apron [R4]	NGVD 1929

Notes on abbreviations:

R.S. = Riverside

C.M. = Corrugated Metal

C. = Concrete

C.E.S. = Concrete Encased Steel Pipe

DBC = Double Box Culvert SBC = Single Box Culvert

b. Inspection.

(1) Pertinent Requirements of the Code of Federal Regulations.

Flood Control Regulations, paragraph 208.10 (d)(1), are quoted in part as follows:

whether seepage is taking place along the lines of their contact with the embankment. Immediate steps shall be taken to correct any adverse conditions."

(2) The outlets of side drainage structures inundate at relatively low river stages. They should, therefore, be inspected at the first sign of a rise in the river to make certain that the gates are not jammed in an open position and thus allow flood waters to enter behind the levee.

4-05. Miscellaneous Facilities.

a. <u>Description</u>. Miscellaneous structures or facilities which were constructed as a part of, or existed in conjunction with, the protective works, and which might affect their functioning, include the following:

(1) Bridges.

(a) A low water bridge crossing the American River at approximate centerline station 229+00. This bridge to be maintained by the owners at the adjacent rock plant.

(b) The H Street bridge crossing the American River at approximate centerline station 364+00. This bridge to be maintained by the County of Sacramento

- (2) <u>Utility Relocations</u>. Because of the nature of the construction of the levee by local interests, no records of any utility relocations are available.
- (3) <u>Hydrographic Facilities</u>. A continuous water stage recorder and staff gage located on the H Street Bridge. This station to be maintained by the U.S. Geological Society and the State Department of Water Resources.

(4) Slurry Wall Facilities.

(a) Site R4: A reinforced concrete drainage channel outlet wall keyed into the slurry wall built under a previous contract, at approximate centerline station 422+70. The integrity of the structure/SCB (soil cement bentonite) cutoff wall interface is to be maintained by the Superintendent.

b. Inspection and Maintenance.

- (1) <u>Pertinent Requirements of the Code of Federal Regulations</u>. Flood Control Regulations, paragraph 208.10 (h)(1) are quoted in part as follows:
 - "(h) Miscellaneous Facilities. (1) Maintenance.

 Miscellaneous structures and facilities constructed as a part of

the protective works and other structures and facilities which function as a part of, or affect the efficient functioning of the protective works, shall be periodically inspected by the Superintendant and appropriate maintenance measures taken. Damaged or unserviceable parts shall be replaced without delay...."

- (2) Inspection of the miscellaneous facilities shall be made at the same time that the inspection of the other features of the project are made, and shall be reported on check list No. 3, sheet No. 4 of Exhibit E.
- (3) The interest of the Corps of Engineers and the responsibility of the local interests in the existing highway and railroad bridges is confined to their effect on the safety and functioning of the flood control channel, but any conditions noted in the inspections that may affect them in any way should, as a matter of courtesy, be brought to the attention of the agencies maintaining and operating them. If the inspection of any miscellaneous structure, either existent or constructed in the future under permit, discloses any condition that indicates the probability of failure during periods of high water, the Superintendent shall address a letter to the owner of the structure, quoting this manual as authority and inviting attention to the conditions observed and requesting that immediate steps be taken to correct them. A copy of such letter shall be forwarded to the District Engineer for his information. A report of the action taken by the owner shall be submitted to the District Engineer to accompany the next semi-annual report. A suggested report form is included as EXHIBIT D of this manual.
- (4) The purpose of maintenance work is to insure continuous satisfactory operation of equipment. It is, therefore, important in such work that all possible causes of future trouble be found and corrected. Particular attention should be given to minor weaknesses which may be given to minor weaknesses which may be an indication of future trouble.
- (5) Site R4: Modification or repair work in the vicinity of the concrete liner where the SCB cutoff wall is located should include provisions to avoid damaging the SCB cutoff. Work that requires penetrating the cutoff wall would require special consideration to assure that the cutoff wall forms a continuous seepage barrier as originally designed.

Acceptable seepage barrier earth fill would have at least 30 percent fines (percent passing the No. 200 sieve) with liquid limit less than or equal to 45 and plasticity index of greater than 7 and less than 25. The controlled low strength material replacing the cutoff wall material should have a permeability similar to the

cutoff wall with at least 1x10-6 cm/sec permeability at 28 days and a strength of at least 40 psi at 28 days. Any potential impact to the cutoff shall be coordinated with the Corps of Engineers, Sacramento District to develop mitigation measures or repair methods for the cutoff wall.

Any channel modifications or repairs impacting the impervious blanket that starts at the SCB cutoff and extends to the levee slope shall be restored to the original layer thickness, using the same soil material type. Any repair to the concrete lined channel that affects the geomembrane and that connects the concrete liner edge to the impervious blanket shall be restored.

c. Operation.

- (1) <u>Requirements of the Code of Federal Regulations</u>. Flood Control Regulations, paragraph 208.10(h)(2) is quoted as follows:
 - "(2) Operation. Miscellaneous facilities shall be operated to prevent or reduce flooding during periods of high water. Those facilities constructed as a part of the protective works shall not be used for purposes other than flood protection without approval of the District Engineer unless designed therefore."

Exhibit B

"AS CONSTRUCTED" DRAWINGS

(See Separate Folder for the Following Drawings)

FILE NO.	TITLE
1-4-375	Emergency Levee Repairs and Bank Protection North Levee American River 1.3 Miles above H Street Bridge, Sheets 1 to 3, inclusive.
1-4-415	Levee Construction and Bank Protection, Fair Grounds to Carmichael Bluffs, Sheets 1 to 20 inclusive.
1-04-0514	American River Common Features (Remaining Sites) WRDA 1996 Contract 1, Sheets 1 to 21 inclusive. [Site R4]



AMERICAN RIVER COMMON FEATURES (REMAINING SITES) WRDA 1996 CONTRACT 1

SACRAMENTO COUNTY CALIFORNIA

FY 2009

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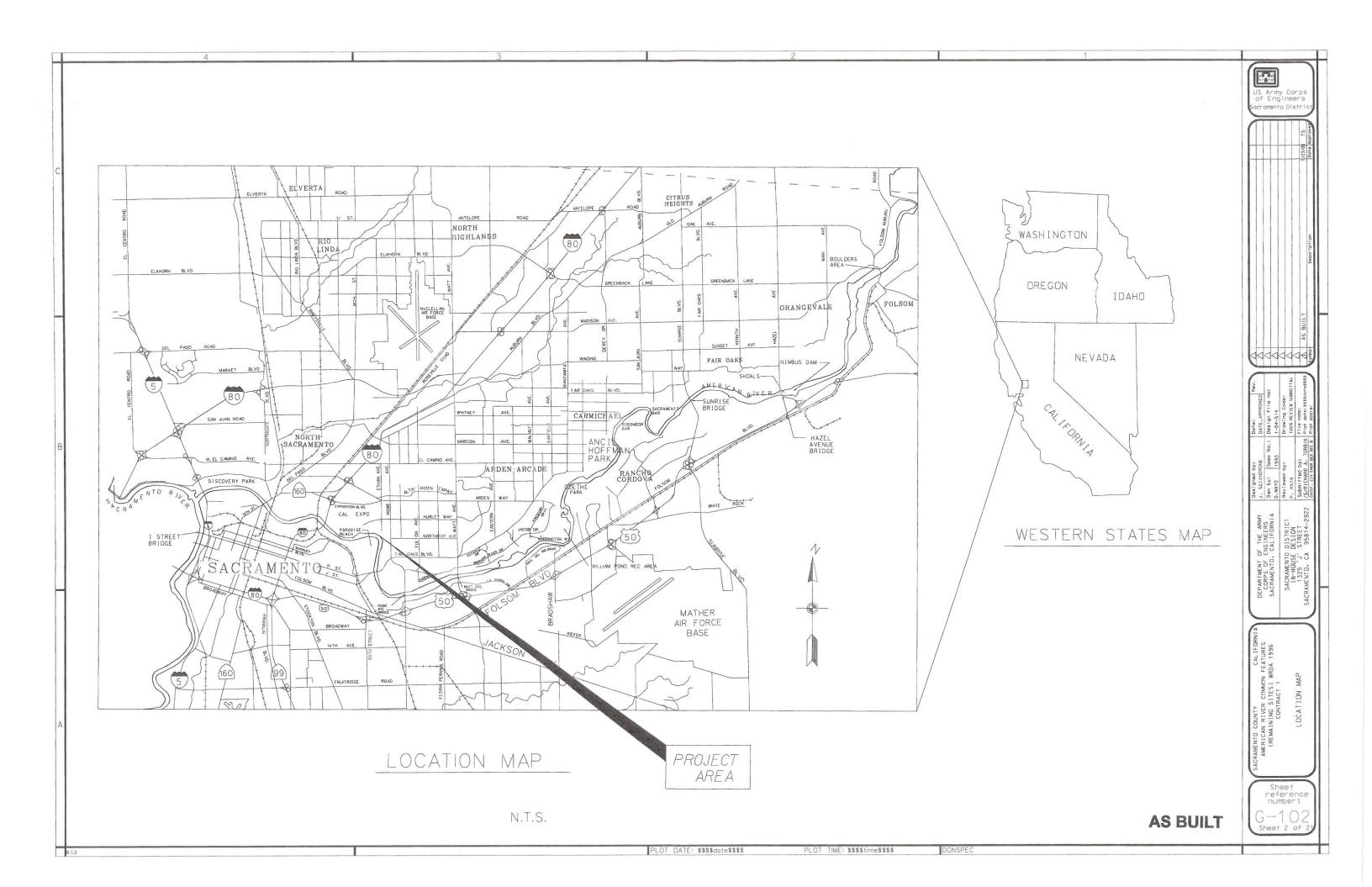
CONTR. NO: W91238-09-D-0020

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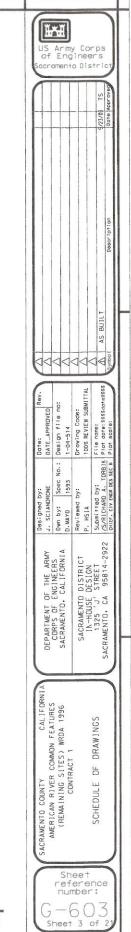
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8.1.1



SCHEDULE OF DRAWINGS

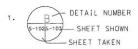
		SCHEDULE OF DRAWINGS
	GENE	RAL , CIVIL PLANS, BORING LOGS AND STRUCTURAL PLANS
SHEET NO.	SHEET REFER NO.	SHEET TITLE
1	G-001	TITLE COVER SHEET
2	G-102	LOCATION MAP
3	G-603	SCHEDULE OF DRAWINGS
4	G-004	GENERAL NOTES, CONSTRUCTION NOTES, LEGENDS AND ABBREVIATIONS
5	G-105	ACCESS ROUTE, HAULING AND BIKE DETOUR PLAN
6	B-701	LOG OF EXPLORATION, 2F-03-15
7	B-702	LOG OF EXPLORATION, 2F-03-16
8	B-703	LOG OF EXPLORATION, 2F-97-13
9	B-704	LOG OF EXPLORATION, 2F-03-20
10	C-100	SITE PLAN
1 1	C-101	TOPOGRAHICAL SURVEY AND BIKE TRAIL DETAILS
12	C-102	GRADING PLAN AND PROFILE
13	C-103	STAFF GAUGE
14	C-104	TEMPORARY SHORING AND EXCAVATION PLAN
15	S-100	PLAN AND PROFILE
16	S-101	DEMOLITION PLAN
17	S-102	PLAN AND SECTION
18	S-103	SECTION AND DETAILS
19	S-104	SECTION AND DETAILS
20	S-105	IMPERVIOUS BLANKET AND CONNECTION DETAILS
21	S-106	GENERAL STRUCTURAL NOTES AND DETAILS



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GENERAL NOTES



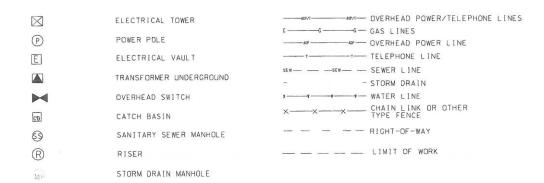
- 2. GENERAL NOTES ARE APPLICABLE TO ALL WORK, UNLESS NOTED OTHERWISE ON OTHER SHEETS.
- 3. BENCH MARK USED: SEE SHEET C-101

LEGENDS

SOIL LOG LOCATION

SPOT ELEVATION

FINISH ELEVATION



ABBREVIATIONS

1

+11.8

ASPHALT CONCRETE AGGREGATE BASE ABOVE GROUND POWER BARBED WIRE B.W. CHAIN LINK
CHAIN LINK
CONSTRUCTION JOINT CONTROLLED LOW STRENGTH MATERIAL CENTIMETER CONC. CONCRETE
DBL. DOUBLE
DEMO. DEMOLISH EL. ELEVATION EVT EVENT POINT EXIST. EXISTING FLOW LINE FEET INCHES LENGTH METER METER
MIN MINIMUM
N.T.S. NOT TO SCALE
POB POINT OF BEGINNING
PCC POINT OF COMPOUND CURVE
PC POINT OF CURVE
POE POINT OF ENDING
PI POINT OF INTERSECTION
PT POINT OF TANGENT
PVI POINT OF VERTICAL INTERSECTION
R/W RIGHT-OF-WAY
RFT STEEL REINFORCEMENT
SD STORM DRAIN
SLP SLOPE
STA. STATION
TYP TYPICAL
TOC TOP OF CONCRETE TYPICAL
TOP OF CONCRETE
TOP OF GROUTED COBBLE 3H:1V SLOPE OF 3 HORIZONTAL TO 1 VERTICAL

CONSTRUCTION NOTES

- 1. THE LOCATION OF EXISTING UTILITIES ARE SHOWN IN AN APPROXIMATE WAY ONLY AND HAVE NOT BEEN INDEPENDENTLY VERIFIED BY THE OWNER OR ITS REPRESENTATIVE. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL UTILITIES WITHIN THE CONSTRUCTION ZONE, ALONG THE CONSTRUCTION ACCESS ROUTE AND IN THE STAGING AREAS BEFORE COMMERCING WORK. THE CONTRACTOR SHALL BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE CAUSED BY THE CONTRACTOR'S FAILURE TO EXACTLY LOCATE AND PRESERVE ANY AND ALL UTILITIES.
- 2. CONTRACTOR SHALL NOTIFY THE CORPS OF ENGINEERS CONTRACTING OFFICER OF HIS CONSTRUCTION SCHEDULE IN ADVANCE OF CONSTRUCTION.
- 3. THE CONTRACTOR WILL BE RESPONSIBLE FOR: a) DISPOSAL OF EXCESS CONSTRUCTION AND EXCAVATION MATERIAL AND COMPLIANCE WITH ALL APPLICABLE FEDERAL, STATE, AND LOCAL REGULATIONS: SUCH AS THE CLEAN WATER ACT AND THE NATIONAL HISTORIC PRESERVATION ACT. b) PREVENT ANY MATERIALS FROM ENTERING THE RIVER AT WATERSIDE DURING CONSTRUCTION. C) NO REFUELING OF MOBILE EQUIPMENT IS PERMITTED ON THE WATERSIDE OF THE LEVEE.
- 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR REPAIRING ANY DAMAGE CAUSED BY THE CONTRACTOR TO THE RECREATION BIKE TRAIL.
 STAGING AREAS, PAVEMENT, ROADS, FENCES, FLOOD CONTROL STRUCTURES, INCLUDING LEVEES, LEVEE RAMP, AND EXISTING BANK PROTECTION
 IMPROVEMENTS, VEGETATION, AND ALL OTHER UTILITIES AND IMPROVEMENT NOT DESIGNATED FOR REMOVAL.
- 5. EXISTING ROADWAYS SHALL BE KEPT CLEAR OF MUD AND DEBRIS AT ALL TIMES.
- 6. CONTRACTOR SHALL PROTECT IN PLACE ALL UTILITIES TO REMAIN WITHIN PROJECT LIMITS, AND WILL BE RESPONSIBLE FOR ANY DAMAGE TO THESE UTILITIES BY CONTRACT WORK. UTILITY SERVICES AND THEIR LOCATIONS MAY NOT BE COMPLETE AS SHOWN ON DRAWING PLANS. THE CONTRACTOR SHALL TAKE NECESSARY MEASURES TO LOCATE AND PROTECT SERVICES DURING CONSTRUCTION. CONTRACTOR SHALL NOT INTERRUPT ANY UTILITY SERVICES.
- 7. ACCESS TO THE LEVEE CROWN AND ACCESS RAMPS ARE INDICATED ON THE DRAWINGS UNLESS DTHERWISE APPROVED BY THE CONTRACTING OFFICER.
- 8. ELEVATIONS REFER TO NATIONAL GEODITIC VERTICAL DATUM (NGVD)1929.
- 9. GRID COORDINATES REFER TO CALIFORNIA COORDINATE SYSTEM ZONE II NAD 83/91 AND ARE BASE ON THE CALFORNIA H.A.R.N. PUBLISHED BY CAL-TRANS.

POINTS OF CONTACT

CONTACT INFORMATION IS PROVIDED FOR THE CONVENIENCE OF THE CONTRACTOR WHERE DIRECT CONTACT IS OR MAY BE REQUIRED. THE PRIMARY POINT OF CONTACT IS THE CONTRACTING OFFICER OR HIS/HER DESIGNATED REPRESENTATIVE. ONLY THE CONTRACTING OFFICER HAS THE AUTHORITY TO ALTER OR INTERPRET THIS CONTRACT SHOULD ANY INTERPRETATION BE NECESSARY. IN THE EVENT ANY CONTACT, WHETHER LISTED HERE OR NOT REQUESTS ADDITIONAL WORK OR AN ALTERATION OF THIS CONTRACT, THE CONTRACTOR SHALL INFORM THE CONTRACTING OFFICER AS SOON AS POSSIBLE, WITH WRITTEN FOLLOW-UP, AND BEFORE PLANNING ANY SUCH WORK.

USA (UNDERGROUND SERVICE ALERT) 1 800 227 2600 Al Sanders inspector 916 869 0510

SACRAMENTO COUNTY TRAFFIC CONTROL (permit needed for hauling material and routing) PRIMARY: JACKIE GROOVER 827 SEVENTH STREET ROOM 105 WINDOW 11 SACRAMENTO, CA 95814 916 874 5606 SECONDARY: SCOTT URBANIK 4100 TRAFFIC WAY SCRAMENTO, CA 95827 urbaniks@saccounty.net

AMERICAN RIVER FLOOD CONTROL DISTRICT (entity which will operate and maintain levee TIM KERR, GENERAL MANAGER, 165 COMMERCE CIRCLE, SUITE D. SACRAMENTO, CA 95815 VOICE 916 929 4006 916 417 4161 916 929 4160

Sacramento County Regional Parks (managing entity of area from fence to river) 4040 Bradshaw Road, Sacramento, CA 95827 PRIMARY: STEVE FLANNERYCHIEF RANGER 916 876 5405 flannerys@saccounty.net SECONDARY: MARY MARET 916 875 4918 maretm@saccounty.net FAX 916 201 5691

SACRAMENTO COUNTY WATER RESOURCES DEPARTMENT STEPHEN KENNING (PRIMARY) 3847 BRANCH CENTER ROAD SACRAMENTO, CA 95827 916 875 7142 petersont@saccounty.net MIKE CROOKS (ALTERNATE) 9280 W. STOCKTON BLVD. #220 ELK GROVE, CA 916 874-5180 957583 crooksm@saccounty.net

SRCSD (SACRAMENTO REGIONAL COUNTY SANITATION DISTRICT) PRIMARY: STEVE NORRIS 10545 ARMSTRONG AVE. SUITE 101 MATHER, CA 95655 916 876 6053 COUNTY SANITATION DISTRICT 1

SACRAMENTO METRO FIRE DISTRICT EMERGENCY ALWAYS CALL 911 PRIMARY Michael Keehn. Fire Prevention Bureau Supervising Inspector office: 942-3326 cell: 616-4679 keehn.michael@smfd.co.gov SECONDARY Deputy Chief of Operations Geoff Miller: 566-4303 miller.geoffrey@smfd.ca.gov TERTIARYAssistant Chief of Operations Mike Johnson: johnson.mike@smfd.ca.gov

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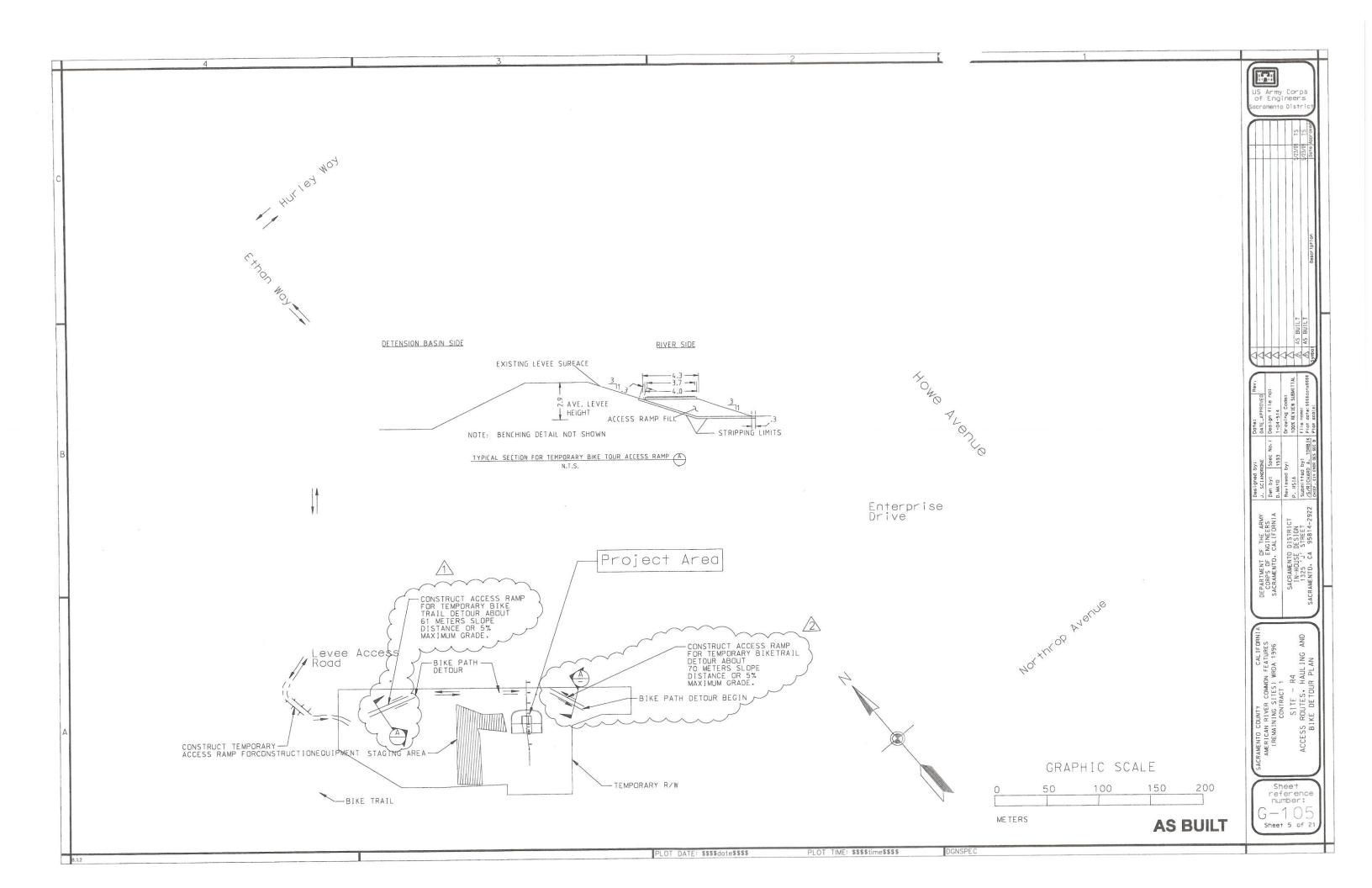
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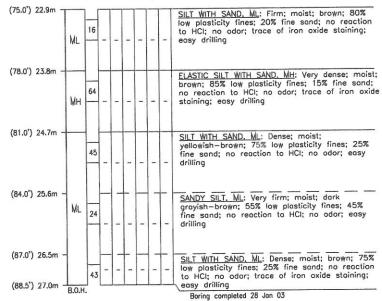
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	2 F - 0 3 - 1 5		2 F - 0 3 - 1 5	2 F - 0 3 - 1 5	
DEPTH N GR SA F	SITE R4 LL PI WC	(38.0') 11.6m	(CONTINUED FROM PREVIOUS COLUMN) POORLY GRADED SAND WITH SILT, SP-SM: Firm; moist; dusky red; 90% fine to medium, subangular sand; 10% nonplastic fines; no reaction to HCl; no odor	(CONTINUED FROM PREVIOUS COLUMN) (75.0') 22.9m 19 NL: Firm; Wet; dark yellowish-brown; 75% low plasticity fines; 25% fine sand; trace of gravel maximum particle size 2.5cm (1"); no reaction to HCl; no odor; loss	
	No samples taken between 0 to 2.4m (8 feet) depths; easy drilling	₩ SM SM CP	POORLY GRADED GRAVEL WITH SAND, GP: Dense; wet; dark brown; 60% fine to medium,	of drilling mud circulation; easy drilling (78.0') 23.8m LEAN CLAY WITH SAND. CL: Hard; wet; yellowish-brown; 75% medium plasticity fines;	
		(42.0') 12.8m 38	subangular to subrounded gravel; 40% fine to coarse, subangular sand; trace of nonplastic fines; no reaction to HCl; no ador; rough drilling At 12.8m (42.0'), driller state top of cobbles; begin air rotary/Odex System on	CL CL 25% fine sond; no reaction to HCl; drilling mud circulation returns; no odor; easy drilling	
(8.0°) 2.4rn	SANDY SILT, ML; Dense; moist; reddish-brown; 65% low plasticity fines; 35% fine sand; no		21Jan2003; pushed 6" dia. casing to 11.6m (38') depth; rough drilling At 13.4m (44.0'), very rough drilling	SANDY SILT. ML: Dense; wet; yellowish—brown; 60% low plasticity fines; 40% fine sand; no reaction to HCl; no odor; slight loss of drilling ML ML	
31	65% low plasticity fines; 35% fine sand; no reaction to HCl; no odor; easy drilling	OBBLES		(84.0') 25.6m - SANDY SILT. ML: Very firm; wet; brown; 60% low plasticity fines; 40% fine sand; no reaction to HCI; no odor; easy drilling (85.5') 26.1m Boring completed 28 Jan 03	
(11.0') 3.4m – 23	SANDY SILT. ML: Very firm; moist; very dark grayish-brown; 70% low plasticity fines; 30% fine sand; no reaction to HCl; no odor; easy drilling	GRAVEL AND CO C C C C C C C C C C C C C C C C C C	At 14.9m (49.0'), wet, sand and gravel fragments in returns; rough drilling	L E G E N D : GR GRAYEL, PERCENT BY WEIGHT PASSING THE 76mm (3-INCH) SIEVE AND RETAINED ON THE NO. 4 SIEVE.	
(14.0') 4.3m –	SANDY SILT, ML: as above at 3.4m (11.0') depth	(53.0') 16.2m -	At 16.2m (53.0'), wet, gravel fragments in	RETAINED ON THE NO. 4 SIEVE. SA SAND, PERCENT BY WEIGHT PASSING THE NO. 4 SIEVE AND RETAINED ON THE NO. 200 SIEVE. FI FINES, PERCENT BY WEIGHT PASSING THE NO. 200 SIEVE. LL LIQUID LIMIT.	
(17.0°) 5.2m –	SANDY SILT. ML: as above at 3.4m (11.0') depth	(66 A)\ (7 (-	returns; moderate to rough drilling At 17.1m (56.0'), Wet returns; moderate drilling At 17.4m (57.0'), driller states bottom of	PI PLASTICITY INDEX (LIQUID LIMIT MINUS PLASTIC LIMIT). MC LABORATORY DETERMINED MOISTURE CONTENT IN PERCENT OF DRY WEIGHT. SM COMBINED FIELD VISUAL IDENTIFICATION AND/OR LABORATORY CLASSIFICATION. NP NONPLASTIC.	
ML 22	- - - -	(57.0') 17.4m	- cobbles; wet, fine to medium sand; casing advances steadily Begin mud rotary on 24Jan03; <u>SILTY SAND, SM</u> : Firm; wet; brown; 80% fine to medium sand; 20% nanplastic fines; no reaction to HCl; no	N NUMBER OF BLOWS WITH THE STANDARD PENETROMETER. R REFUSAL WITH THE STANDARD PENETROMETER (SEE NOTE 10). A ATTEMPT WITH THE STANDARD PENETROMETER (SEE NOTE 11). WATER LEVEL.	Tote: R
21	SANDY SILT, ML; Very firm; moist; dark brown; 70% low plasticity fines; 30% fine sand; no reaction to HCi; no odor; carbon rootlets present; easy drilling	(60.0') 18.3m - SP	odor; loss of drilling mud circulation POORLY GRADED SAND WITH SILT, SP-SM: Very dense; wet; brown; 90% fine to medium sand;	B.O.H. BOTTOM OF HOLE.	bys D
(23.0') 7.0m – 8 – –	SANDY SILT. ML: as above at 6.1m (20.0') except loose; dark grayish-brown	(60.8') 18.5m - SM 52 ML	10% nonplastic fines; no reaction to HCl; no odor; loss of drilling mud circulation SILT. ML: Very dense; moist; light olive—brown; 95% low plasticity fines; 5% fine sand; no reaction to HCl; no odor; trace of iron oxide	THIS DRAWING IS TO BE USED ONLY FOR APPROXIMATE DESCRIPTIONS OF SUBSURFACE CONDITIONS. SOIL CLASSIFICATIONS AND DESCRIPTIONS ARE BASED ON FIELD LOG DESCRIPTIONS IN ACCORDANCE. THE DESCRIPTION AND DESCRIPTION OF SOUR VISION AND PROCEDURES AND ADDRESS OF SOUR VISION AND PROCEDURES.	Designed
(26.0') 7,9m -	SANDY SILT, ML: as above at 6.1m (20.0') except firm	(63.0') 19.2m 44	staining; loss of drilling mud circulation SILTY SAND. SM: Dense; moist; dark yellowish-brown; 60% fine sand; 40% low plasticity fines; no reaction to HCI; no odor; loss of drilling mud circulation	THE COBBLE LAYER. BORING 2F-03-15 WAS DRILLED WITH AN IRA DRILL RIG USING A ORDEX	
(29.0') 8.8m -	SANDY SILT. ML; Firm; moist; very dark grayish-brown; 65% low plasticity fines; 35% fine sand; no reaction to HCl; no odor; carbon rootlets present; easy drilling	(66.0') 20.1m	SILTY SAND. SM: Very firm; moist; brown; 75% fine to coarse, subangular to subrounded sand; 15% nonplastic fines; 10% fine to coarse, subangular to subrounded gravel; no reaction to HCl; no odor; moderate drilling WELL-GRADED SAND. SW: Very firm; moist; dark	SYSTEM WITH A 152mm (6") CASING AT DEPTHS WITHIN THE COBBLE LAYER. BORING 2F-03-15 WAS DRILLED WITH A MUD ROTARY SYSTEM AT DEPTHS BELOW THE COBBLES LAYER. BORING 2F-03-15 WAS DRILLED BETWEEN 27 AND 28 JANUARY 2003. 6 WHERE COBBLES WERE ENCOUNTERED, LABORATORY GRADATION DATA REPRESENT MATERIAL PASSING.	
(29.6') 9.0m 18	SILTY SAND, SM: Firm; moist; dark brown; 75% fine to medium, subangular sand; 25% nonplastic fines; no reaction to HCl; no odor	(69.0') 21.0m - SW	brown; 95% fine to medium, subangular to subrounded sand; 5% nonplastic fines; no reaction to HCI; no odor WELL-GRADED SAND. SW: as above except excessive loss of drilling mud circulation;	THE 76mm (3") SIEVE. PARTICLES LARGER THAN THE INSIDE DIAMETER OF THE SAMPLER WERE NOT RECOVERED; HENCE, ALL GRAVEL AND COBBLE LAYERS REFERENCED IN SOIL BORINGS CONTAINED LARGER PARTICLES OF GRAVEL AND COBBLES THAT ARE NOT REFLECTED IN THE LABORATORY GRADATION TEST RESULTS SHOWN IN THE LOGS OF EXPLORATIONS. 7. STANDARD PENETROMETER DATA WERE OBTAINED IN ACCORDANCE WITH ASTM D 1586 UTILIZING A 610mm LONG STANDARD 51mm O.D. BY 35mm I.D. SPLIT BARREL SAMPLER. THE SPLIT—BARREL PENETROMETER WAS DRIVEN USING AN AUTOMATIC—TRIP, 63.5 KG SAFETY HAMMER. ALL SPT "N"	AiNAC
(32.0°) 9.8m - 16	SILTY SAND. SM: as above at 9.0m (29.6') except trace of carbon rootlets	(72.0') 21.9m (72.2') 22.0m	moderate drilling SILTY SAND, SM: Very firm; moist; dark	VALUES ARE "FIELD", I.E. UNCORRECTED FOR DEPTH. 8. GROUNDWATER WAS ENCOUNTERED AT THE TIME OF EXPLORATIONS; GROUNDWATER LEVELS CAN BE EXPECTED TO FLUCTUATE IN RESPONSE TO RAINFALL VARIATIONS, PARTICULARLY IN THE VICINITY OF SITE DRAINAGE FEATURES. 9. DEPENDING ON THE SOIL MOISTURE AT THE TIME OF CONSTRUCTION, THE SOIL ENCOUNTERED MAY	TY CALIF
(35.0') 10.7m -	SILTY SAND. SM: Firm; moist; very dark grayish-brown; 85% fine to medium, subangular sand; 15% nonplastic fines; no reaction to HCl; no odor	SM 26	yellowish-brown; 65% fine sand; 35% low plasticity fines; hard nodules of sand and silt present; no reaction to HCl; no odor; loss of drilling mud circulation; easy drilling SILTY SAND, SM: Very firm; moist; dark yellowish-brown; 55% fine sand; 45% low	BE UNSTABLE OR POTENTIALLY UNSTABLE. THE PROBABILITY OF UNSTABLE CONDITIONS IS HIGHEST WHEN THE SOIL MOISTURE IS GREATEST. 10. REFUSAL WITH THE STANDARD PENETROMETER IS DEFINED AS ONE OF THE FOLLOWING: A. 10 BLOWS FOR NO APPARENT ADVANCEMENT OF THE SAMPLER; OR B. 50 BLOWS FOR LESS THAN 152mm (6") ADVANCEMENT OF THE SAMPLER; OR	NTO COUNTY
(38.0°) 11.6m	(CONTINUED IN THE NEXT COLUMN)	(75.0') 22.9m	plasticity fines; no reaction to HCl; no odor; loss of drilling mud circulation; easy drilling (CONTINUED IN THE NEXT COLUMN)	C. 100 BLOWS FOR 152mm (6") TO 457mm (18") ADVANCEMENT OF THE SAMPLER. 11. ATTEMPT WITH THE STANDARD PENETROMETER IS DEFINED AS REFUSAL WITHIN THE FIRST 152mm (6") OF SEATING PENETRATION. 12. SEE SHEET C-100 FOR LOCATION OF EXPLORATIONS. 13. SEE SHEET B-703 FOR STANDARD PENETROMETER DESCRIPTIVE DATA.	SACRAME
				GRAPHIC SCALE 0 0.5 1 1.5 2 2.5 SCALE 1:25 METERS AS BUILT	

	DEPTH								2 F - 0 3 - 1 6 SITE R4								(CO	NTIN	2 F - 0 3 - 1 6 JUED FROM PREVIOUS COLUMN)	
	EL, 13.65m (44.8') 0 -		N G	R S/	FI		PI	MC	Hole is located at top of levee, centerline of crown. About 2.5cm (1") of bituminous course and AB material present. No samples taken between 0 to 2.4m (8 feet depths; easy drilling)	(38.0') 11.6r	SP SM		-	- -	- -	-	-	POORLY GRADED SAND WITH SILT, SP-SM: Loose; moist; very dark grayish-brown; 90% fine to medium, subangular sand; 10% nonplastic fines; no reaction with HCI	(75.0') 22.9m
С										Ž	<u>₹</u> (41.0') 12.5i	n Sh	17	-	1	- -	-	-	SILTY SAND. SM: Firm; wet; very dark grayish—brown; 85% medium, subangular sand; 15% nonplostic fines; 1 piece of fractured gravel maximum particle size 2.5cm (1"); no odors present; no reaction with HCl At 12.5m (41.1"), lense of black orgains 3cm (1.2") thick	(78.0') 23.8m
	(8.0°) 2.4m -										(44.0') 13.4	GI	21		- -	- -	-	-	POORLY GRADED GRAVEL WITH SAND. GP: Very firm; wet; dark brown; 50% coarse, angular to subangular gravel maximum particle size 5.1cm (2"); 45% fine to coarse, angular to subangular sand; 5% anaplastic fines; no odor; no reaction with HCI	(81.0') 24.7m
Dominion of the last		SC	20	- -	- -	-	-	-	CLAYEY SAND, SC; Firm; moist; very dark gray 60% fine, subangular sand; 40% medium plasticity fines; no reaction to HCl; no odor	7	(47.0') 14.3 (47.9') 14.6	m - SI	14	-	- -	- -	-	=	POORLY GRADED SAND WITH SILT. SP-SM: Firm; wet; very dark grayish-brown; 90% fine to coarse, subangular sand; 10% nonplastic fines; no odor present; no reaction with HCl; rock in shoe	(84.0') 25.6m -
+	(11.0°) 3.4m -		19	- -	- -		-	-	SANDY SILT, ML: Firm; dry; dark brown; 70% low plasticity fines; 30% fine, subangular sand no odor present; no reaction with HCl	;	(50.0') 15.2	m GI	A	-			-	-	POORLY GRADED GRAVEL WITH SAND. GP: Firm; wet; light brown to dark yellowish-brown; 50% coarse, subangular to surbrounded gravel; 45% fine to coarse, subangular to subrounded sand; 5% low plasticity fines; no odor; no reaction with HCI; traces of charcoal present; rough drilling	(87.0') 26.5m -
	(14.0°) 4.3m		19	-	+	-	-	-	SANDY SILT. ML: Firm; dry; dark brown; 70% low plasticity fines; 30% fine, subangular sand no odor present; no reaction with HCI	- I;	(53.0') 16.2 (53.6') 16.3	m S	VI	_		-	-	-	POORLY GRADED SAND WITH SILT, SP-SM: Very dense; wet; very dark grayish-brown; 90% fine to coarse, subangular sand; 10% nonplastic fines; no odar present; no reaction with HCI; rough drilling SILTY SAND, SM: Dense; wet; dark gray; 80% fine to coarse, subangular sand; 20%	(88.5') 27.0m B.
	(17.0') 5.2m		5	-	+		_		SILT WITH SAND, ML: Loose; moist; dark brown 85% low plasticity fines; 15% very fine sand; no reaction with HCI	 n;	(55.0') 16.8	WEL & B	ES	-	-	- -	-	-	fine to coarse, subangular sand; 20% nonplastic fines; no ador; no reaction with HCI POORLY GRADED GRAVEL WITH SAND. GP: Dense; Wet; very dark grayish-brown; 60% fine to coarse, subangular to subrounded gravel maximum particle size 41cm (1-5/8"); 35% fine to coarse, subangular sand; 5% nonplastic fines; no odor; no reaction with HCl; rough	SA FI LL
В	(20.0') 6,1m	ML	6		+	_	-		SILT WITH SAND ML: Loose; moist; dark brown 85% low plasticity fines; 15% very fine sand; no reaction with HCI	n;	(57.0') 17.4	m - E	00 4	5 _	-	+	-	-	fines; no adar; no reaction with HCl; rough drilling At 16.8m (55.0"), driller state top of cobbles; begin air rotary/Odex System on 27Jan2003; placed 6" dia. casing to 15.2m (50") depth; very little amounts of returns;	PI MC SM NP
	(23.0') 7,0m			-	+	- -	-	-	SILT WITH SAND ML: Loose; moist; dark brow 85% low plasticity fines; 15% very fine sand;		(60.0') 18.3 (60.5') 18.4		M/2	1	-	= =	-	-	moist, fine sand to gravel fragments; rocky drilling At 16.8m (55.0'), lots of water in returns At 17.1m (56.0'), dry gravel fragments At 17.4m (57.0'), wet, fine sand returns; driller states bottom of cobbles/gravels at	N R A ∑ B,O.H.
	(26.0') 7.9m		9	-	- -	- -	-		no reaction with HCI		(63.0°) 19.3 (63.4°) 19.3	2m -5V		5	-	= =	-	-	17.4m (57.0') Begin mud rotary on 28Jan03; SANDY SILT. ML: Dense; moist; dark yellowish-brown; 70% low plasticity fines; 30% fine sand; no reaction to HCl; no odor; trace of iron oxide staining	N O T E 1. THIS DR 2. SOIL CL
-			11	-	- -	- -	-	-	SANDY SILT. ML: Firm: moist; dark brown; 65: low plasticity fines; 35% very fine sand; no reaction with HCI	7 6	(66.0') 20.	lm -		1-		1	-	-	Iominations; easy drilling WELL-GRADED SAND WITH SILT. SW-SM: Very firm; wet; dark brown; 90% fine to medium, subangular to subrounded sand; 10% nonplastic fines; no reaction to HCl; no odor; easy drilling SILTY SAND, SM: Very firm; wet; dark brown;	WITH AS OR LABO ENGINEE 3. ALL COL 4. ALL SIEV
	(29.0°) 8.8m		12	-	- -	- -	-	-	SILTY SAND. SM: Firm; dry; brown; 85% fine t medium, subangular sand; 15% nonplastic fina no reaction with HCI	to es;	(69.0') 21.	0m - c	M L	3 -	-	- -	-	-	70% fine sand; 30% nonplastic fines; no reaction to HCl; no odor WELL-GRADED SAND WITH SILT, SW-SM: same as above at 18.3m (60.0') depth SILTY SAND, SM: same as above at 18.4m (60.5') depth	5. BORING THE COE SYSTEM WAS DRI 2F-03-
	(32.0') 9.8m	SM	12	-	+	- -	-	-	SILTY SAND, SM: Firm; dry; dusky red; 80% fine, subangular sand; 20% nonplastic fines; reaction with HCI	no	tro all co			4 -	-	- -	-	-	SILTY SAND, SM: Dense; wet; dark brown; 75% fine sand; 25% nonplastic fines; no reaction to HCI; no odor; loss of drilling mud circulation; easy drilling SILTY SAND, SM: Dense; wet; dark brown; 80%	6. WHERE THE 76r NOT REC TAINED GRADATII 7. STANDAR 610mm
А	(35.0') 10.7m (35.5') 10.8m	-	7	=	+	+	= =	<u>_</u>	SILTY SAND, SM; Loose; moist; very dark grayish-brown; 70% fine, subangular sand; 3(nonplastic fines; particles of charcoal present no reaction with HCl	 0% ;	(72.0') 21.	am -	4	7 -	-	-	-	-	Hine sand; 20% nonplastic fines; no reaction to HCl; no odor; easy drilling SILTY SAND, SM: Dense; moist; brown; 70% fine sand; 30% nonplastic fines; no reaction to HCl; no odor; easy drilling	PENETRO VALUES 8. GROUND EXPECTE SITE DR
	(38.0') 11.6m				- -		1-	(CON	SILTY SAND. SM: Loose; dry; dusky red; 80% fine, subangular sand; 20% nonplastic fines; no reaction which the same subangular sand; 20% nonplastic fines; no reaction with the same subangular sand; 20% nonplastic fines; no reaction with the same subangular same subangula		(75.0') 22.	9m <u> </u>					1 ((CON	TINUED IN THE NEXT COLUMN)	9. DEPENDI BE UNS WHEN TO 10. REFUSAL A. 10 B. 50
T DATE: 1 WAY 2007										0	G R A P	H I C	S 1.5	C A	L_E 2 		2.5 W	/ETEF	नड	C. 100 11. ATTEMPT (6") OF 12. SEE SHI 13. SEE SHI

2F-03-16 (CONTINUED FROM PREVIOUS COLUMN)



- GRAVEL, PERCENT BY WEIGHT PASSING THE 76mm (3-INCH) SIEVE AND RETAINED ON THE NO. 4 SIEVE.
- SAND, PERCENT BY WEIGHT PASSING THE NO. 4 SIEVE AND RETAINED ON THE NO. 200 SIEVE.
- FINES, PERCENT BY WEIGHT PASSING THE NO. 200 SIEVE.
- PLASTICITY INDEX (LIQUID LIMIT MINUS PLASTIC LIMIT).
- LABORATORY DETERMINED MOISTURE CONTENT IN PERCENT OF DRY WEIGHT.
- COMBINED FIELD VISUAL IDENTIFICATION AND/OR LABORATORY CLASSIFICATION.
- NUMBER OF BLOWS WITH THE STANDARD PENETROMETER.
- REFUSAL WITH THE STANDARD PENETROMETER (SEE NOTE 10).
- ATTEMPT WITH THE STANDARD PENETROMETER (SEE NOTE 11).
- WATER LEVEL.
- BOTTOM OF HOLE.

- DRAWING IS TO BE USED ONLY FOR APPROXIMATE DESCRIPTIONS OF SUBSURFACE CONDITIONS.
- CLASSIFICATIONS AND DESCRIPTIONS ARE BASED ON FIELD LOG DESCRIPTIONS IN ACCORDANCE ASTM D 2488 (DESCRIPTION AND IDENTIFICATION OF SOILS, VISUAL-MANUAL PROCEDURE) AND ABORATORY TEST RESULTS IN ACCORDANCE WITH ASTM D 2487 (CLASSIFICATION OF SOILS FOR EXPLAID TO THE PROPERTY OF SOILS FOR FERING PURPOSES).
- COLORS SHOWN ARE IN ACCORDANCE WITH THE MUNSELL SOIL COLOR CHART.
- SIEVE SIZES SHOWN ARE METRIC WITH U.S. STANDARD IN PARENTHESES.
- NG 2F-03-16 WAS DRILLED WITH A MOBILE B-61 DRILL RIG USING A 152mm (6") ABOVE COBBLE LAYER. BORING 2F-03-16 WAS DRILLED WITH AN IRA DRILL RIG USING A ORDEX EM WITH A 152mm (6") CASING AT DEPTHS WITHIN THE COBBLE LAYER. BORING 2F-03-16 DRILLED WITH A MUD ROTARY SYSTEM AT DEPTHS BELOW THE COBBLES LAYER. BORING 3-16 WAS DRILLED ON 28 JANUARY 2003.
- E COBBLES WERE ENCOUNTERED, LABORATORY GRADATION DATA REPRESENT MATERIAL PASSING 75mm (3") SIEVE. PARTICLES LARGER THAN THE INSIDE DIAMETER OF THE SAMPLER WERE RECOVERED; HENCE, ALL GRAVEL AND COBBLE LAYERS REFERENCED IN SOIL BORINGS COND LARGER PARTICLES OF GRAVEL AND COBBLES THAT ARE NOT REFLECTED IN THE LABORATORY OF THE PROPERTY TION TEST RESULTS SHOWN IN THE LOGS OF EXPLORATIONS.
- DARD PENETROMETER DATA WERE OBTAINED IN ACCORDANCE WITH ASTM D 1586 UTILIZING A nm LONG STANDARD 51mm O.D. BY 35mm LD. SPLIT BARREL SAMPLER. THE SPLIT-BARREL TROMETER WAS DRIVEN USING AN AUTOMATIC-TRIP, 63.5 KG SAFETY HAMMER. ALL SPT "N" ES ARE "FIELD", I.E. UNCORRECTED FOR DEPTH.
- NDWATER WAS ENCOUNTERED AT THE TIME OF EXPLORATIONS; GROUNDWATER LEVELS CAN BE TIED TO FLUCTUATE IN RESPONSE TO RAINFALL VARIATIONS, PARTICULARLY IN THE VICINITY OF DRAINAGE FEATURES.
- NDING ON THE SOIL MOISTURE AT THE TIME OF CONSTRUCTION, THE SOIL ENCOUNTERED MAY NSTABLE OR POTENTIALLY UNSTABLE. THE PROBABILITY OF UNSTABLE CONDITIONS IS HIGHEST THE SOIL MOISTURE IS GREATEST.
- SAL WITH THE STANDARD PENETROMETER IS DEFINED AS ONE OF THE FOLLOWING:
- O BLOWS FOR NO APPARENT ADVANCEMENT OF THE SAMPLER; OR O BLOWS FOR LESS THAN 152mm (6") ADVANCEMENT OF THE SAMPLER; OR
- 00 BLOWS FOR 152mm (6") TO 457mm (18") ADVANCEMENT OF THE SAMPLER. MPT WITH THE STANDARD PENETROMETER IS DEFINED AS REFUSAL WITHIN THE FIRST 152mm OF SEATING PENETRATION.
- SHEET C-100 FOR LOCATION OF EXPLORATIONS.
- 13. SEE SHEET B-703 FOR STANDARD PENETROMETER DESCRIPTIVE DATA.

AS BUILT



	-	description (as property)	The second named in column 2 is not a local division of the local division in column 2 is not a local	ı
MENT OF THE ARMY	Designed by:	75 TONE	Date:	
PS OF ENGINEERS WENTO, CALIFORNIA	Dwn bys K. PHAN	Ckd by: R, IMASA	Design file not X-XX-XXX	
AMENTO DISTRICT	Reviewed by	У.	SPEC. No.: XXXX	1 3
125 'J' STREET 170, CA 95814-2922	Submitted by: /S/ ED, KETO-NV OHEF, SOL DES, SEC,	bys ETCHUM	File name: Plot date: 1 MAY Plot scale: 1:1	1 A

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reference number: B - 702Sheet 7 of 21

STANDARD PENETROMETER DESCRIPTIVE DATA 2 F - 9 7 - 1 3 US Army Corps of Engineers Socramento District 2F-97-13 COHESIVE COHESIONLESS (Continued from Previous Column) DEPTH BLOWS* RELATIVE DENSITY BLOWS* CONSISTENCY N GRSA FILL PIMC N GRSA FILL PIMO (40.0°) 12.2m -VERY SOFT EL. 13.65m (44.8')= 07 VERY LOOSE At surface, asphaltic concrete about 51mm (2") thick; aggregate base about 51mm (2") thick SILTY SAND. SM: Loose; moist; brown; about 75% fine to medium sand; about 25% low plasticity fines SOFT 5-10 LOOSE 11-20 FIRM (41.5') 12.6m STIFF VERY FIRM SANDY LEAN CLAY, CL: Very stiff; moist; brown; medium plasticity fines; fine to medium, mostly fine sand; gravel maximum particle size 19mm (3/4") 21-30 16-30 VERY STIFF 31-50 SM 9 VERY DENSE 31+ HARD 51+ SILTY SAND, SM: Loose; moist; brown; about 85% fine to medium sand; about 15% low plasticity fines *BLOWS PER 0.3m OF PENETRATION OF A 51mm O.D. AND 35mm I.D. SAMPLER DRIVEN BY A 63.5kg HAMMER, WITH At 1.1m (3.5'), as above except stiff (45.0') 13.7m (5.0') 1.5m LEGEND: 85 10 POORLY GRADED SAND WITH SILT. SP-SM: Very loose; moist; dark grayish-brown; fit to medium, mostly fine sand; nonplastic GRAVEL, PERCENT BY WEIGHT PASSING THE 76mm (3-INCH) SIEVE AND RETAINED ON THE NO. 4 SIEVE. GR (46.5') 14.2m - SP (6.5') 2.0m SAND, PERCENT BY WEIGHT PASSING THE NO. 4 SIEVE AND RETAINED ON THE NO. 200 SIEVE. fines; subrounded gravel maximum particle size 38mm (1-1/2"); rough drilling (7.0') 2.1m SANDY SILT, ML: Firm; moist; dark grayish-brown; about 65% low plasticity fines; about FINES, PERCENT BY WEIGHT PASSING THE NO. 200 SIEVE. brown; about 65% 10% piece 35% fine to medium sand (48.5') 14.8m LIQUID LIMIT. CLAYEY GRAVEL WITH SAND, GC: Dense; moist; dark brown; about 40% fine to coarse, (9.0°) 2.7m At 2.7m (9.0'), no penetration with sam-pler; no recovery; speculate pipe present PLASTICITY INDEX (LIQUID LIMIT MINUS PLASTIC LIMIT). moist; dark orini; abut 40% inter to coolse; subangular to subrounded gravel maximum particle size 32mm (1-1/4"); about 35% fine to coorse, subangular to subrounded sand; about 25% low to medium plasticity fines; moderate drilling LABORATORY DETERMINED MOISTURE CONTENT IN PERCENT OF DRY WEIGHT. MC COMBINED FIELD VISUAL IDENTIFICATION AND/OR LABORATORY CLASSIFICATION. SM SILTY SAND. SM: Very firm; moist; very dark grayish-brown; about 55% fine to medium sand; about 45% low plasticity fines (11.0') 3.4m - SM At 15.8m (52.0'), water level

From 15.8m (52.0') to bottom of hole, samples were obtained with a 64mm (2-1/2")
1.D. sampler NUMBER OF BLOWS WITH THE STANDARD PENETROMETER. ¥ (52.0') 15.8m -NUMBER OF BLOWS WITH SAMPLERS LARGER THAN THE STANDARD PENETROMETER. 31 30 39 26 8 8 REFUSAL WITH THE STANDARD PENETROMETER (SEE NOTE 10). (13.0') 4.0m CLAYEY GRAVEL WITH SAND. CC: Very firm; moist; fine to coarse, subangular to sub-rounded gravel maximum particle size 38mm (1-1/2"); fine to coarse, mostly fine, subangular to subrounded sand; low plasticity fines (53.5') 16.3m -ATTEMPT WITH THE STANDARD PENETROMETER (SEE NOTE 11). SANDY SILT, ML: Very firm; moist; dark grayish-brown; about 70% low plasticity fines; about 30% fine to medium sand ∇ WATER LEVEL. B.O.H. BOTTOM OF HOLE. (15.0') 4.6m COBBLES AND GRAVEL (44444444) (55.5') 16.9m SANDY SILT. ML: Very firm; moist; brown; nonplastic fines; fine to medium, mostly fine sand; easy drilling 0 38 62 COBBLES WITH MATRIX MATERIAL (GRAYEL, SAND AND FINES); MATRIX MATERIAL IS CLASSIFIED IN ACCORDANCE WITH ASTM STANDARDS. CLAYEY GRAVEL WITH SAND. GC: Very dense; wet; dark brown; about 40% subrounded gravel maximum particle size 51mm (2"); about 30% fine sond; about 30% low plasticity fines; slow drilling (16.5') 5.0m (17.0') 5.2m SILT WITH SAND. ML: Very firm; moist; dark brown; about 80% low plasticity fines; about 20% fine sand NOTES: GRAYEL AND COBBLES with a matrix of SILTY SAND WITH GRAYEL SM: Dense; wet; dark brown; nonplastic fines; fine to coarse sand; coarse, subrounded grayel maximum particle size 38mm (1-1/2"); slow drilling ₩ XXX (59.0°) 18.0m 1. THIS DRAWING IS TO BE USED ONLY FOR APPROXIMATE DESCRIPTIONS OF SUBSURFACE CONDITIONS. SOIL CLASSIFICATIONS AND DESCRIPTIONS ARE BASED ON FIELD LOG DESCRIPTIONS IN ACCORDANCE WITH ASTM D 2488 (DESCRIPTION AND IDENTIFICATION OF SOILS, VISUAL—MANUAL PROCEDURE) AND/OR LABORATORY TEST RESULTS IN ACCORDANCE WITH ASTM D 2487 (CLASSIFICATION OF SOILS FOR ENGINEERING PURPOSES). (60.5') 18.4m (20.5') 6.2m At 18.6m (61.0'), as above except easier drilling; gravel and cobbles present ED, KETCHUM SOL DES, SEC. 3. ALL COLORS SHOWN ARE IN ACCORDANCE WITH THE MUNSELL SOIL COLOR CHART. 20 8. X SANDY SILT. ML: Very firm; moist; brown about 70% low plasticity fines; about 30% 4. ALL SIEVE SIZES SHOWN ARE METRIC WITH U.S. STANDARD IN PARENTHESES. GRAYEL AND COBBLES with a matrix of SILTY SAND, SM: Very dense; wet; dark brown; fine to coorse, mostly fine to medium sand; grayel moximum particle size 76mm (3"); nonplastic fines; refusal on rock particle; BORING 2F-97-13 WAS DRILLED WITH A CME 75 DRILL RIG USING A 203mm (8") HOLLOW STEM AUGER BETWEEN 8 AND 11 AUGUST 1997. (62.5') 19.1m 6, WHERE COBBLES WERE ENCOUNTERED, LABORATORY GRADATION DATA REPRESENT MATERIAL PASSING THE 76mm (3") SIEVE. RECOVERY OF PARTICLES LARGER THAN THE INSIDE DIAMETER OF THE SAMPLER WERE NOT RECOVERED; HENCE, ALL GRAVEL AND COBBLE LAYERS REFERENCED IN SOIL BORINGS CON-At 7.3m (24.0'), as above except dark (64.0°) 19.5m gravels are fragments from larger rock ma-terial TAINED LARGER PARTICLES OF GRAVEL AND COBBLES THAT ARE NOT REFLECTED IN THE LABORATORY GRADATION TEST RESULTS SHOWN IN THE LOGS OF EXPLORATIONS. OF THE ARMY ENGINEERS CALIFORNIA At 7.5m (24.5'), as above except loose At 64.5', as above except easy drilling UNLESS OTHERWISE NOTED, STANDARD PENETROMETER DATA WERE OBTAINED IN ACCORDANCE WITH ASTM D 1586 USING A STANDARD 457mm (18") LONG BY 51mm (2") O.D. BY 38mm (1-1/2") I.D. SAMPLER. A 64mm (2-1/2") O.D. BY 61mm (2") I.D. SAMPLER OR A 76mm (3") O.D. BY 64mm (2-1/2") I.D. SAMPLER WAS USED IN SOME CASES WHERE COARSE GRAINED MATERIAL WAS ENCOUNTERED. CHANGES IN SAMPLERS ARE DESIGNATED WITH AN ASTERISK (*) AND ARE STATED WITHIN THE DESCRIPTION. THE SPLIT-BARREL PENETROMETER WAS DRIVEN USING AN AUTOMATIC-TRIP, 63.5KG (140-LB). SAFETY HAMMER. THE FIELD SPT "N" VALUES HAVE NOT BEEN CORRECTED FOR SPOON SIZE OR DEPTH. At 20.1m (66.0'), refusal on rock particle; (66,0') 20,1m ACRAMENTO IN-HOUSE 1325 'J. S' MENTO, CA DEPARTMENT O CORPS OF E SACRAMENTO, At 20.6m (67.5'), as above except hard drilling; difficulty with extracting sample rod (27.5') 8.4n (68.5') 20.9m -POORLY GRADED SAND WITH SILT. SP-SM: Dense; dark brown to block; fine to coorse, mostly fine to medium sand; nonplastic fines; grovel maximum particle size 19mm (3/4") 8. GROUNDWATER WAS ENCOUNTERED AT THE TIME OF EXPLORATIONS; GROUNDWATER LEVELS CAN BE EXPECTED TO FLUCTUATE IN RESPONSE TO RAINFALL VARIATIONS, PARTICULARLY IN THE VICINITY OF SITE DRAINAGE FEATURES. SILTY SAND, SM: Firm; moist; brown; fine to medium, mostly fine sand; nonplastic fines; no reaction with HCI SP-SM 78 1 88 11 (29.0') 8.8m SM (70.0') 21.3m -9. DEPENDING ON THE SOIL MOISTURE AT THE TIME OF CONSTRUCTION, THE SOIL ENCOUNTERED MAY BE UNSTABLE OR POTENTIALLY UNSTABLE. THE PROBABILITY OF UNSTABLE CONDITIONS IS HIGHEST WHEN THE SOIL MOISTURE IS GREATEST. B.O.H. (31,0°) 9,4m 10. REFUSAL WITH THE STANDARD PENETROMETER IS DEFINED AS ONE OF THE FOLLOWING: COUNTY
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S OF EXPLORATION

ZF-97-13 A. 10 BLOWS FOR NO APPARENT ADVANCEMENT OF THE SAMPLER; OR POORLY GRADED SAND WITH SILT. SP—SM: Firm; moist; olive—brown; about 90% fine to medium sand; about 10% nonplastic fines B. 50 BLOWS FOR LESS THAN 152mm (6") ADVANCEMENT OF THE SAMPLER; OR C. 100 BLOWS FOR 152mm (6") TO 457mm (18") ADVANCEMENT OF THE SAMPLER. 11. ATTEMPT WITH THE STANDARD PENETROMETER IS DEFINED AS REFUSAL WITHIN THE FIRST 152mm (6") OF SEATING PENETRATION. From 10.4m (34.0') to 10.5m (34.5'), no recovery; catcher was not installed 12. SEE SHEET C-100 FOR LOCATION OF EXPLORATIONS. SP- 27 At 10.5m (34.5'), as above except firm reference (40.0') 12.2m number: (Continued in Next Column) GRAPHIC SCALE B - 7030.5 1 1.5

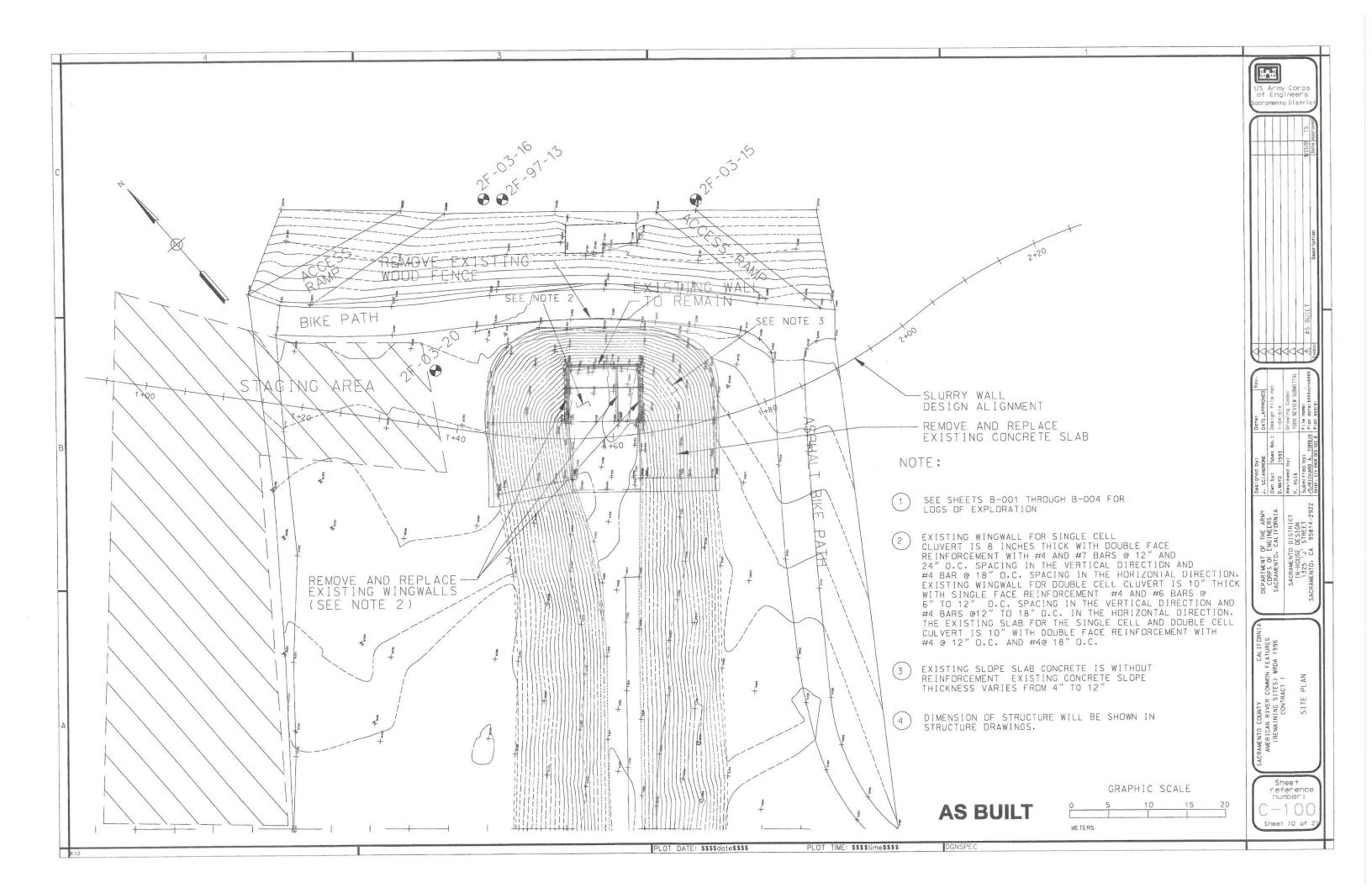
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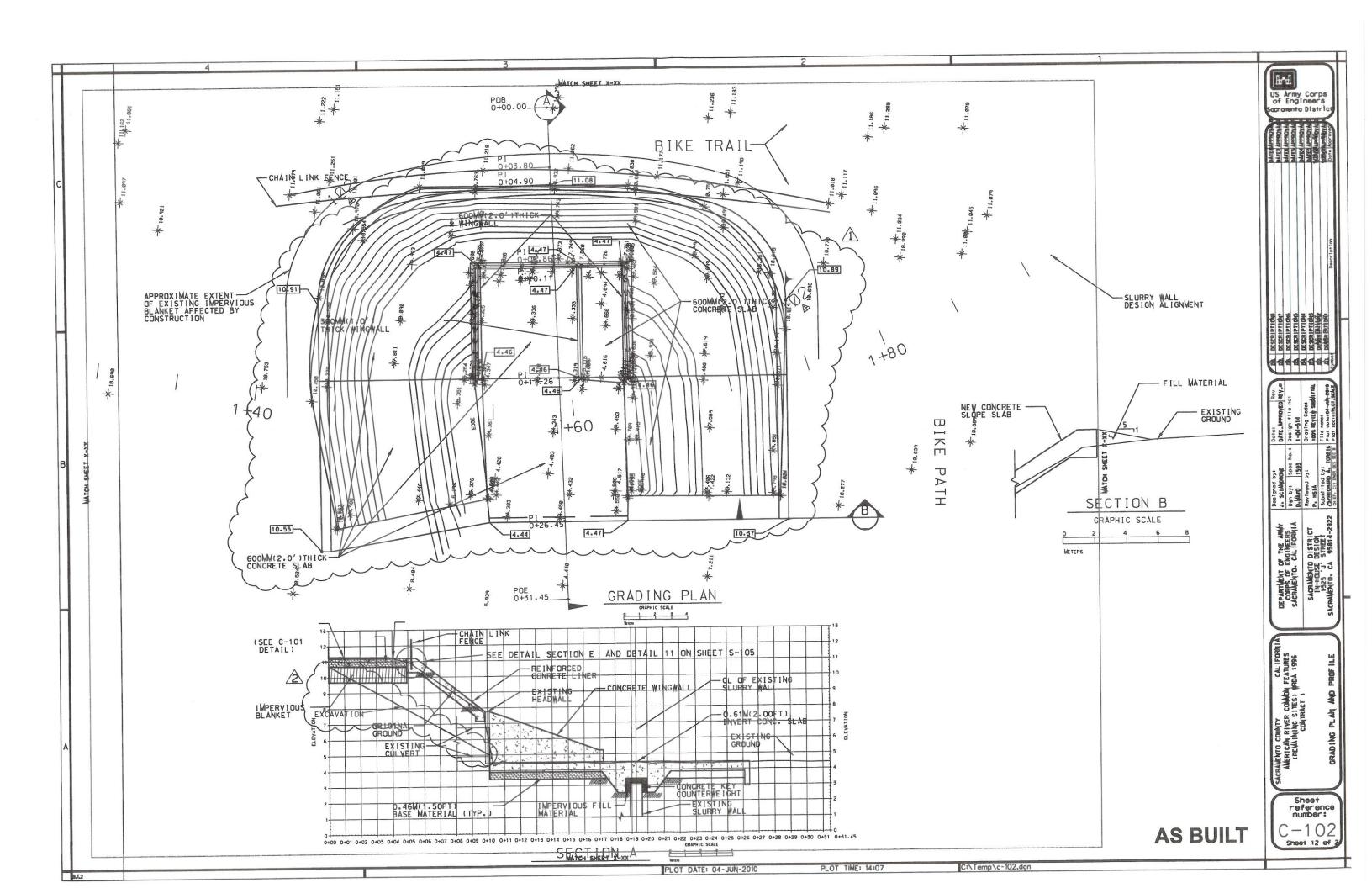
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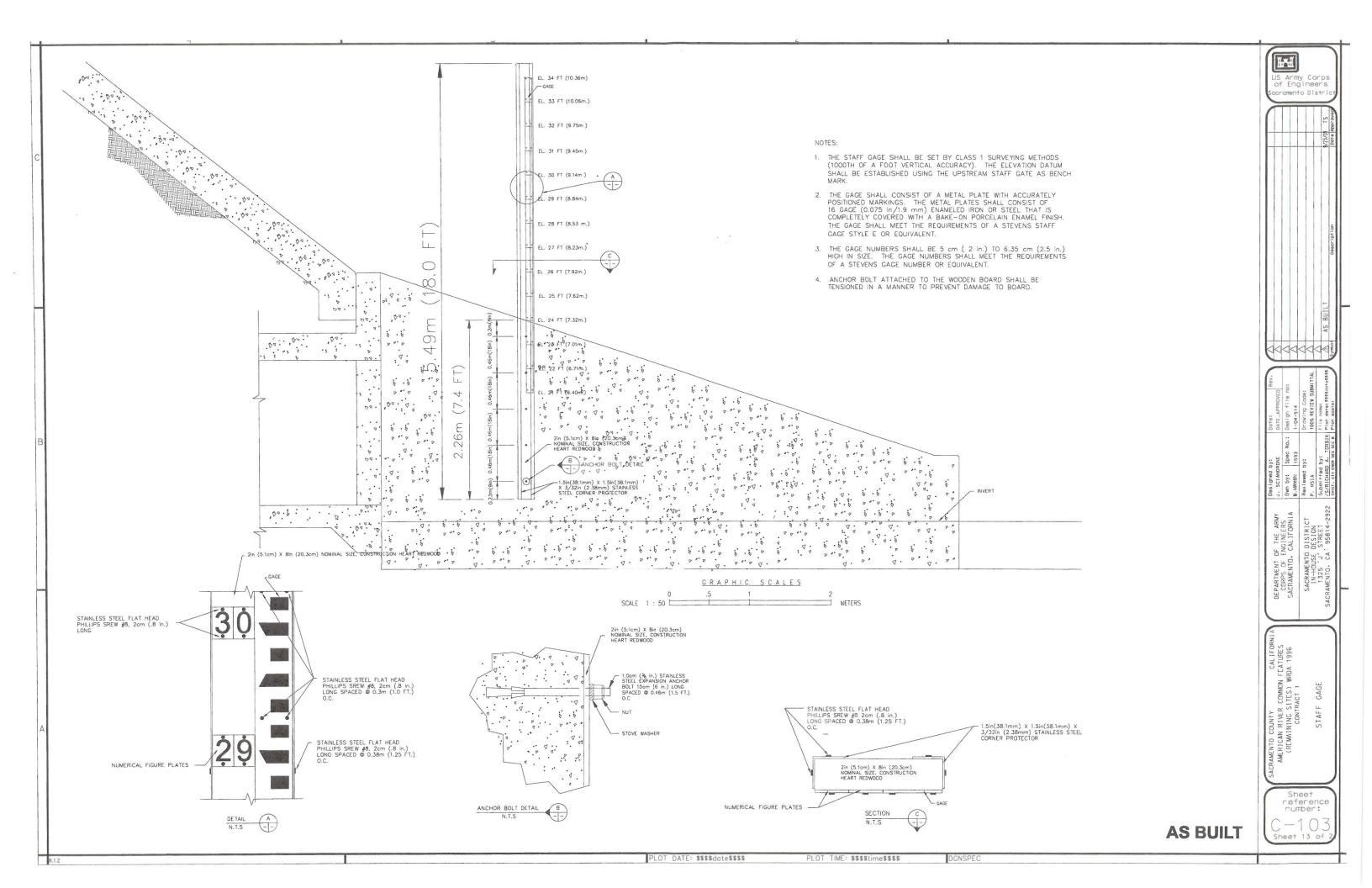
Sheet 8 of 21

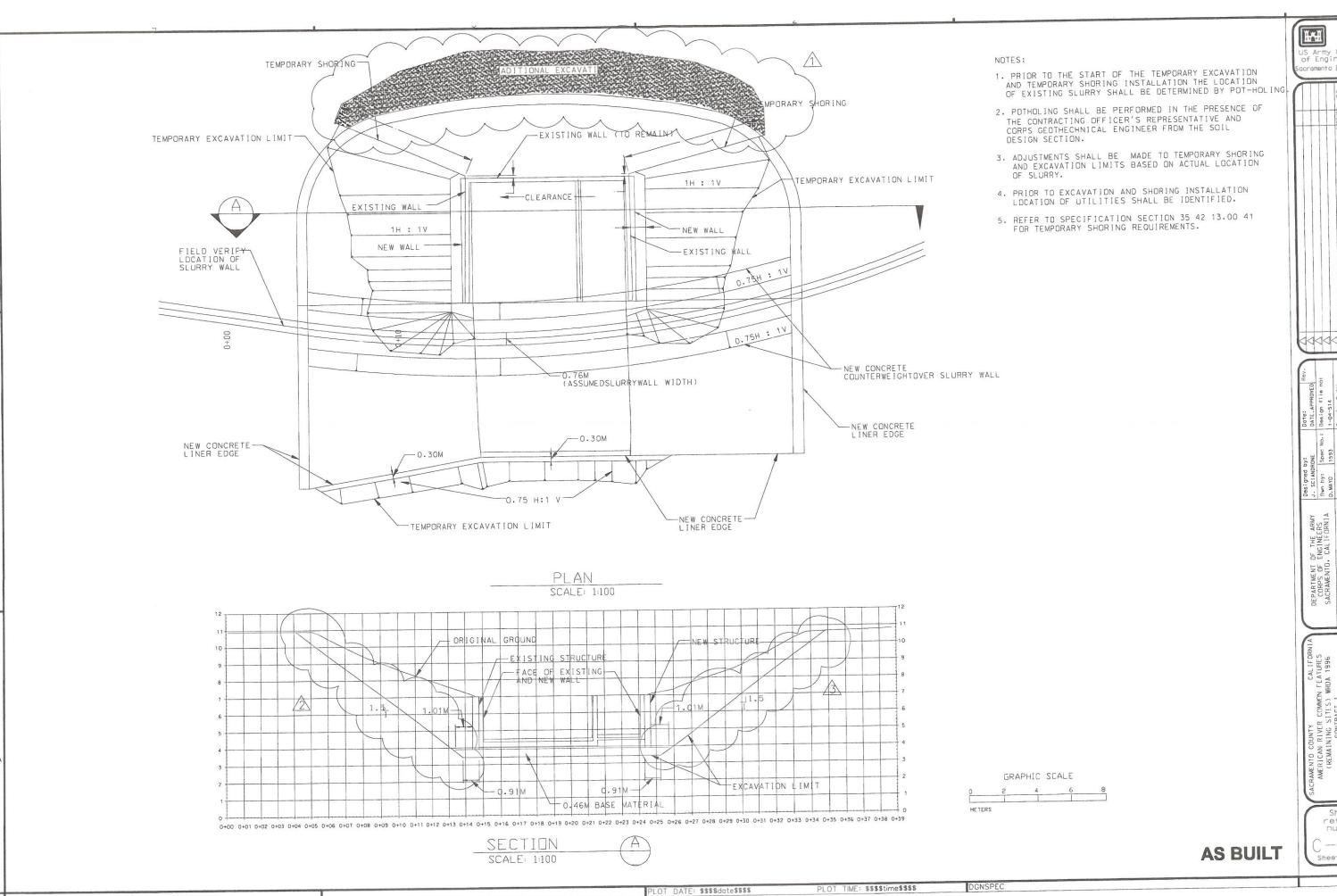
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2 F - 0 3 - 2 0 2 F - 0 3 - 2 0 US Army Corps of Engineers Socramento District 2F-03-20 (CONTINUED FROM PREVIOUS COLUMN)
PI MC (CONTINUED FROM PREVIOUS COLUMN) SITE R4 DEPTH N GR SA FI LL Boring drilled on waterside of levee N GR SA FI LL N GR SA FI LL PI MC (77.0') 23.5m (39°) 11.9m -<u>SILTY SAND. SM</u>: Very dense; wet; gray-brown; fine grained sand EL. 10.92m (35.8')± 0 -At 24.5m (80.5') grades from fine to medium grained sand; Weak cementation; trace of clay SANDY SILT, ML: Firm; moist; brown; fine sand (83.2') 25,4m SILT WITH SAND, ML: Very dense; moist; gray with mottled orange; fine grained sand At 1.8m (6.0') becomes firm (47.0°) 14.3m CLAYEY SILT. ML: Very stiff; moist; tan At 14.6m (48.0') changed drill rigs to Mobile At 2.6m (8.5') grades to dark brown (50.0°) 15.2m SILTY SAND, SM: Dense; wet; brown; fine to medium grained sand; abundant mica At 4.0m (13.0°) grades to light reddish-brown SM - - - - - to medium grained sand (90.4') 27.5m (91.0') 27.7m -Boring drilled 25-26 Feb. 03 B.O.H. At 16.1m (53.0') grades to very firm LEGEND: At 4.7m (15.5') grades to brown; fine to medium grained GRAVEL, PERCENT BY WEIGHT PASSING THE 76mm (3-INCH) SIEVE AND RETAINED ON THE NO. 4 SIEVE. (56.0') 17.1m SAND, PERCENT BY WEIGHT PASSING THE NO. 4 SIEVE AND RETAINED ON THE NO. 200 SIEVE. POORLY GRADED SAND WITH SILT, SP—SM: Dense; Wet; gray; fine to medium grained sand; abundant mica; decreased silt content FINES, PERCENT BY WEIGHT PASSING THE NO. 200 SIEVE. (18.0') 5.5m SILTY SAND. SM: Firm; moist; gray-brown to red-brown; fine to (predominantly) medium 11 0 85 15 PLASTICITY INDEX (LIQUID LIMIT MINUS PLASTIC LIMIT). LABORATORY DETERMINED MOISTURE CONTENT IN PERCENT OF DRY WEIGHT. COMBINED FIELD VISUAL IDENTIFICATION AND/OR LABORATORY CLASSIFICATION. (59.5') 18.1m SANDY SILT. ML: Very dense; moist; brown; fine grained sand NP At (21.5') 6.5m grades to loose; brown NUMBER OF BLOWS WITH THE STANDARD PENETROMETER. REFUSAL WITH THE STANDARD PENETROMETER (SEE NOTE 9). ATTEMPT WITH THE STANDARD PENETROMETER (SEE NOTE 10). SILTY SAND. SM: Firm; wet; gray-brown; fine to medium grained sand WATER LEVEL. BOTTOM OF HOLE. B.O.H. (63.5') 19.3m SANDY SILT, ML: Firm; moist; red-brown; fine grained sand OF THE ARMY ENGINEERS S, CALIFORNIA From 7.5m (24.5') grades moist to wet NOTES: DESIGN TREET 95814-1. THIS DRAWING IS TO BE USED ONLY FOR APPROXIMATE DESCRIPTIONS OF SUBSURFACE CONDITIONS. SOIL CLASSIFICATIONS AND DESCRIPTIONS ARE BASED ON FIELD LOG DESCRIPTIONS IN ACCORDANCE WITH ASTM D 2488 (DESCRIPTION AND IDENTIFICATION OF SOILS, VISUAL—MANUAL PROCEDURE) AND/ OR LABORATORY TEST RESULTS IN ACCORDANCE WITH ASTM D 2487 (CLASSIFICATION OF SOILS FOR ENGINEERING PURPOSES). At 20.3m (66.5') grades to mottled light gray with orange SACRAMENTO IN-HOUSE E 1325 'J' ST RAMENTO, CA DEPARTMENT O CORPS OF E SACRAMENTO, 3. ALL COLORS SHOWN ARE IN ACCORDANCE WITH THE MUNSELL SOIL COLOR CHART. 4. ALL SIEVE SIZES SHOWN ARE METRIC WITH U.S. STANDARD IN PARENTHESES. BORING 2F-03-20 WAS DRILLED WITH A MOBILE B-61 DRILL RIG USING A 203mm (8") HOLLOW STEM AUGER BETWEEN 25 AND 26 FEBRUARY 2003. At 20.7m (68.0') very dense; grades to POORLY GRADED SAND WITH SILT. SP-SM: Loose; moist; brown; fine to medium grained sand (30.0°) 9.1m 6. STANDARD PENETROMETER DATA WERE OBTAINED IN ACCORDANCE WITH ASTM D 1586 UTILIZING A 610mm LONG STANDARD 61mm O.D. BY 35mm I.D. SPLIT BARREL SAMPLER, THE SPLIT-BARREL PENETROMETER WAS DRIVEN USING AN AUTOMATIC-TRIP, 63.5 KG SAFETY HAMMER. ALL SPT "N" VALUES ARE "FIELD", I.E. UNCORRECTED FOR DEPTH. ¥ (31.0') 9,4m At 9.1m (30.0') Wet; trace of coarse sand RVEN TO CANHON FEATURES
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OF EXPLORATION
2F-03-20 GROUNDWATER WAS ENCOUNTERED AT THE TIME OF EXPLORATIONS; GROUNDWATER LEVELS CAN BE EXPECTED TO FLUCTUATE IN RESPONSE TO RAINFALL VARIATIONS, PARTICULARLY IN THE VICINITY OF SITE DRAINAGE FEATURES. At 21.8m (71.5') firm (33.0') 10,1m 8. DEPENDING ON THE SOIL MOISTURE AT THE TIME OF CONSTRUCTION, THE SOIL ENCOUNTERED MAY BE UNSTABLE OR POTENTIALLY UNSTABLE. THE PROBABILITY OF UNSTABLE CONDITIONS IS HIGHEST WHEN THE SOIL MOISTURE IS GREATEST. REFUSAL WITH THE STANDARD PENETROMETER IS DEFINED AS ONE OF THE FOLLOWING:
 A. 10 BLOWS FOR NO APPARENT ADVANCEMENT OF THE SAMPLER; OR At 23.0m (75.5') Dense; grades to mottled gray; weak cementation B. 50 BLOWS FOR LESS THAN 152mm (6") ADVANCEMENT OF THE SAMPLER; OR C. 100 BLOWS FOR 152mm (6") TO 457mm (18") ADVANCEMENT OF THE SAMPLER. 200 ATTEMPT WITH THE STANDARD PENETROMETER IS DEFINED AS REFUSAL WITHIN THE FIRST 152mm (6") OF SEATING PENETRATION. R At 11.3m (37.0') changed drill rigs to ODEX 11. SEE SHEET C-100 FOR LOCATION OF EXPLORATIONS. (77.0') 23.5m 12. SEE SHEET B-703 FOR STANDARD PENETROMETER DESCRIPTIVE DATA. (CONTINUED IN THE NEXT COLUMN) (39') 11,9m (CONTINUED IN THE NEXT COLUMN) reference number; GRAPHIC SCALE B - 704SCALE 1: 25 1 1.5 METERS AS BUILT Sheet 9 of 21



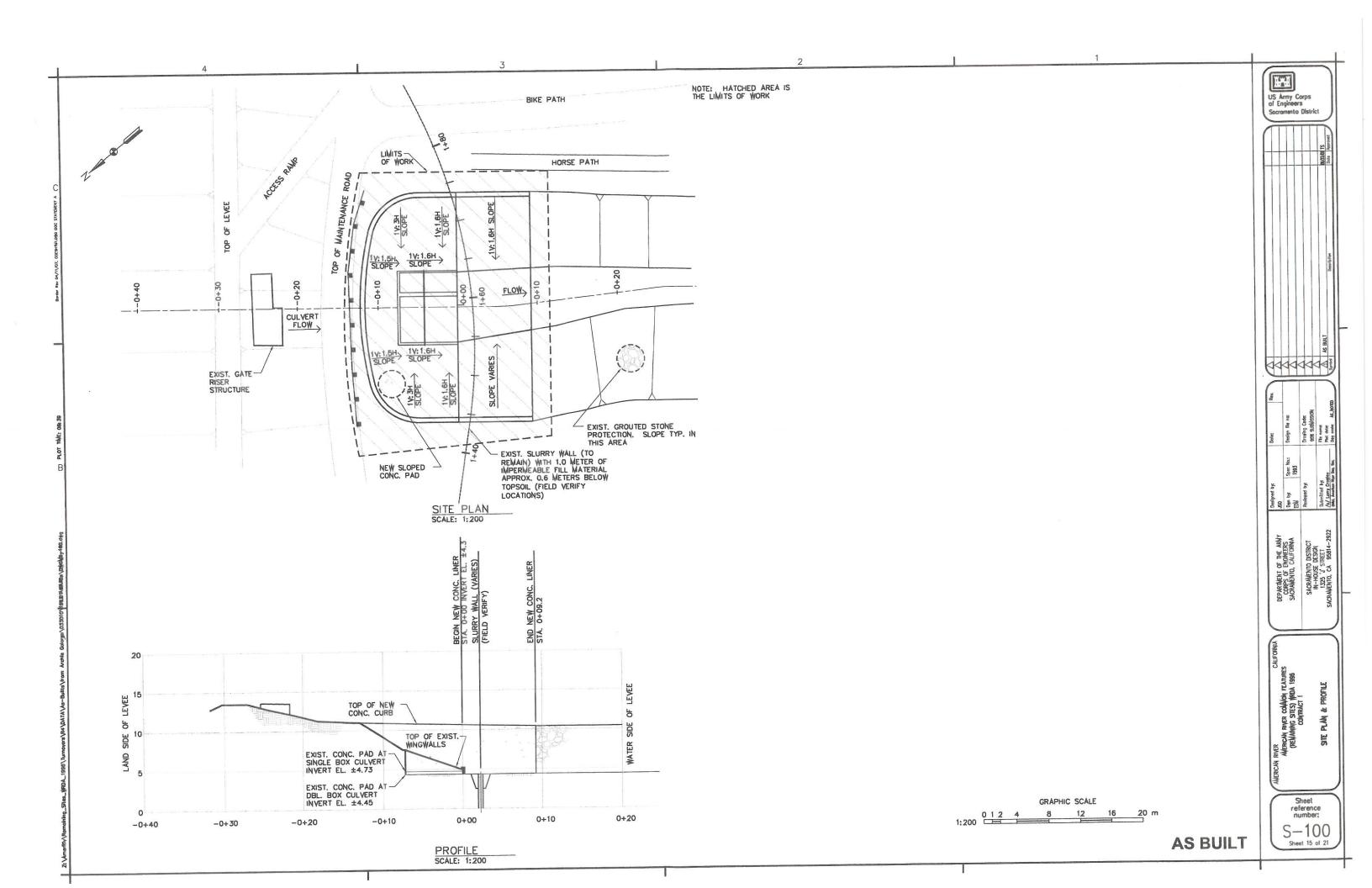


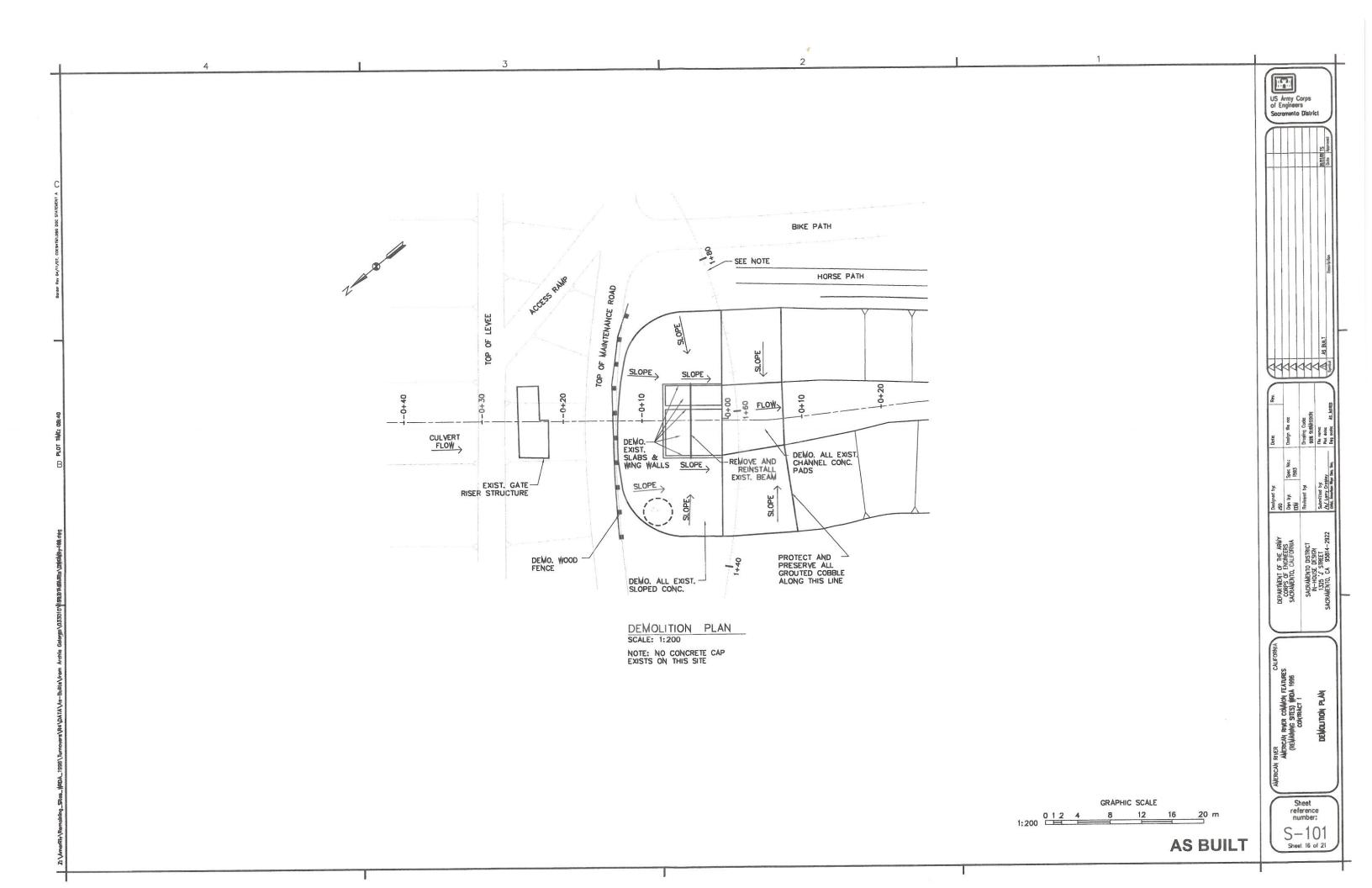


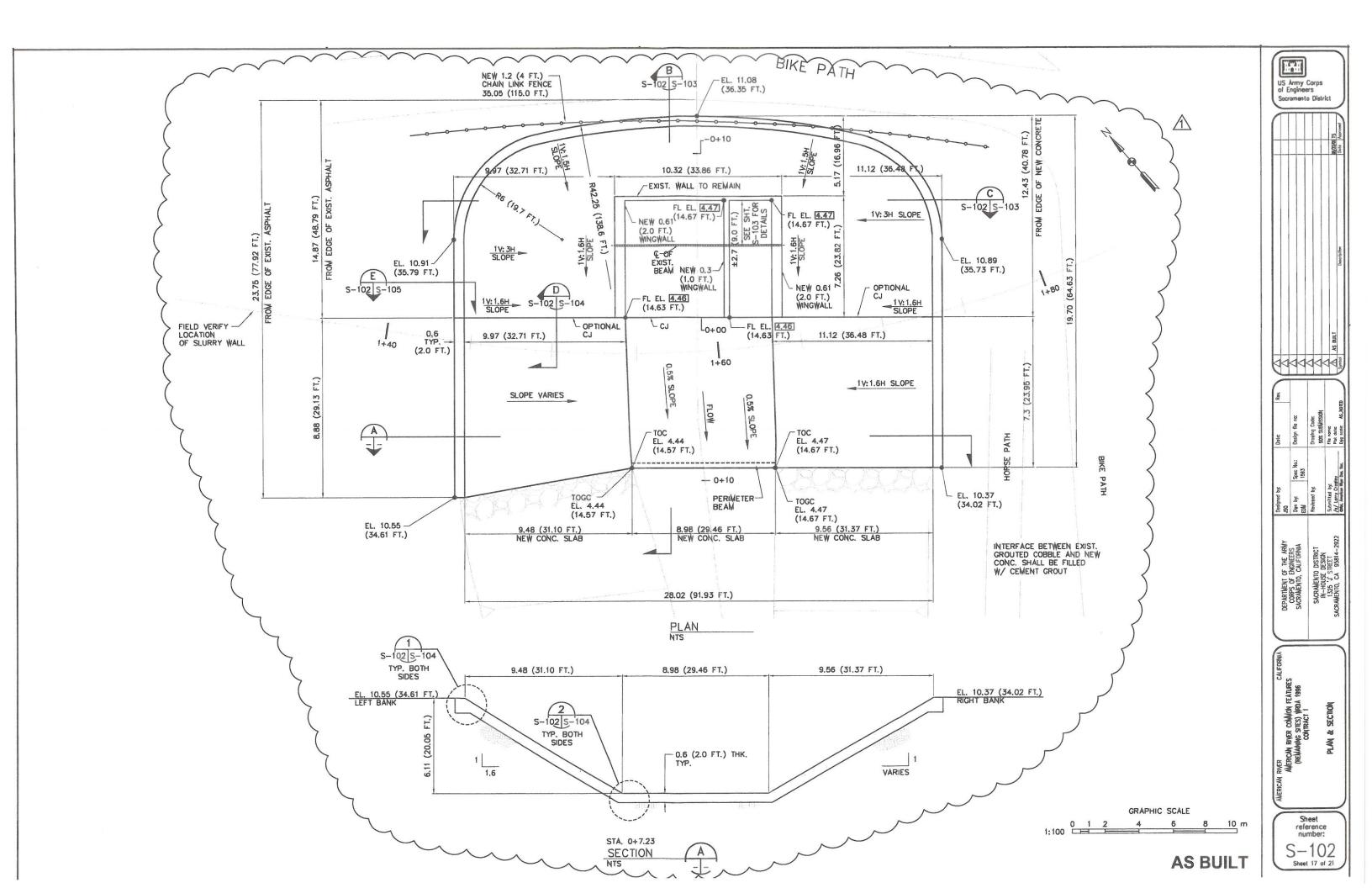


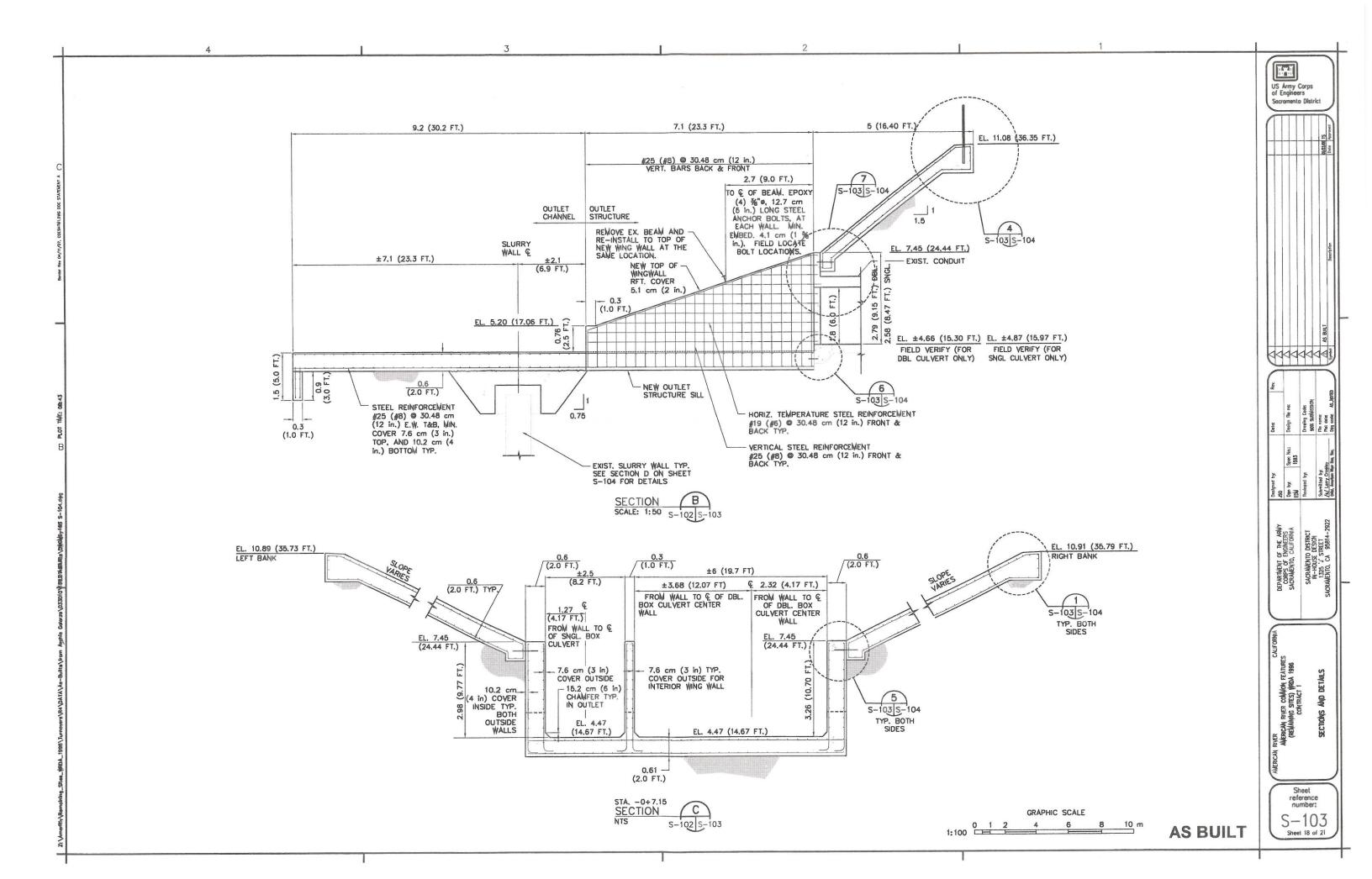
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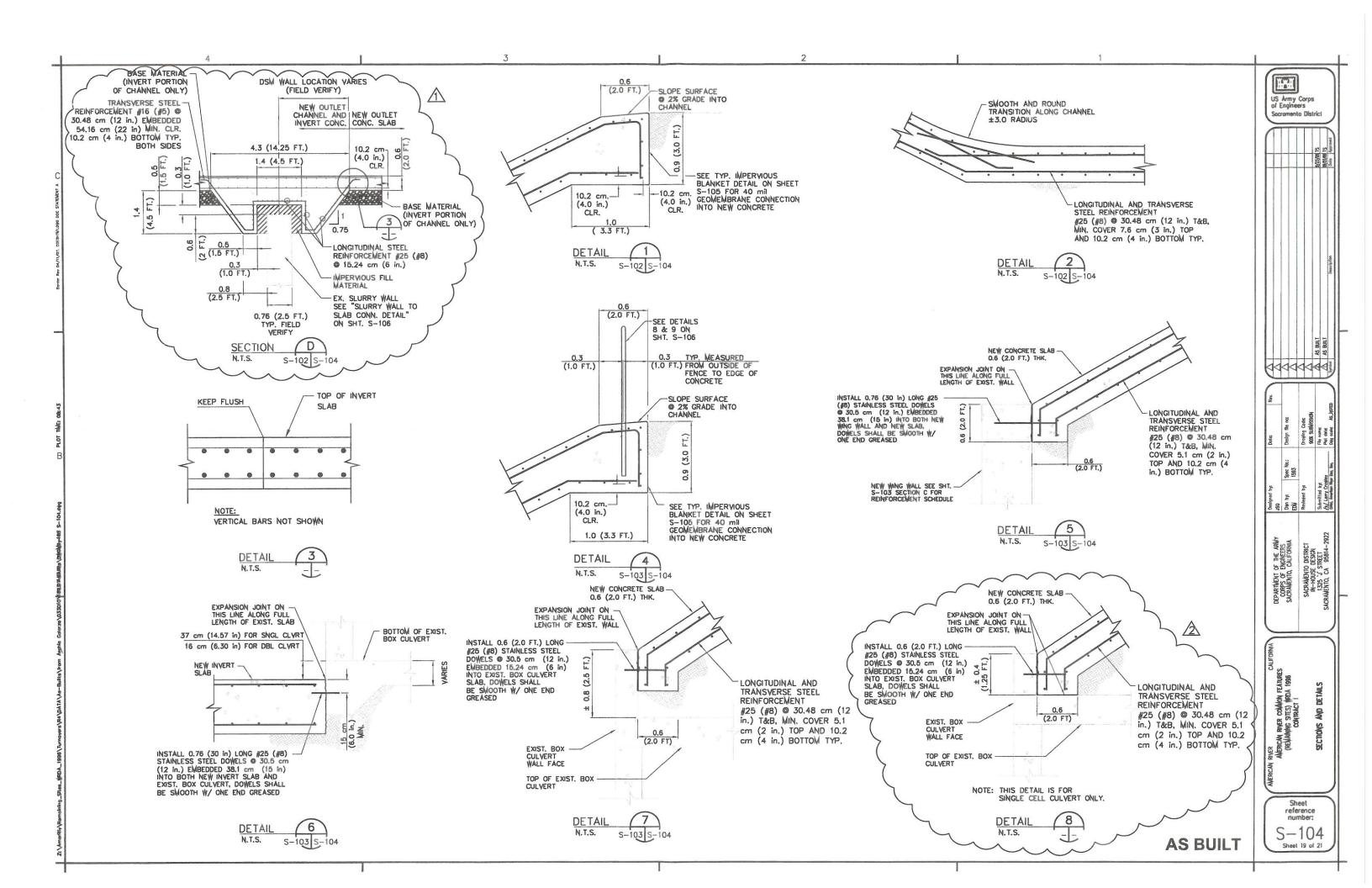
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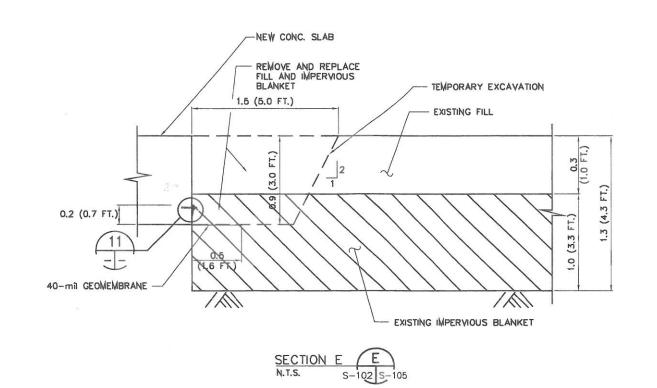




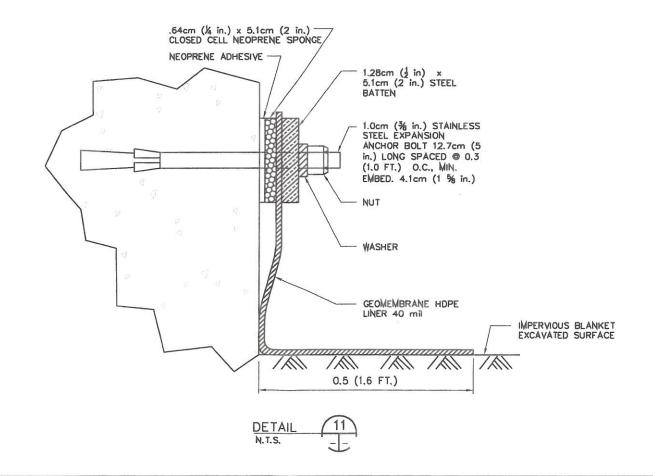


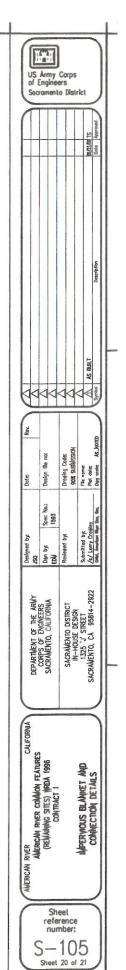




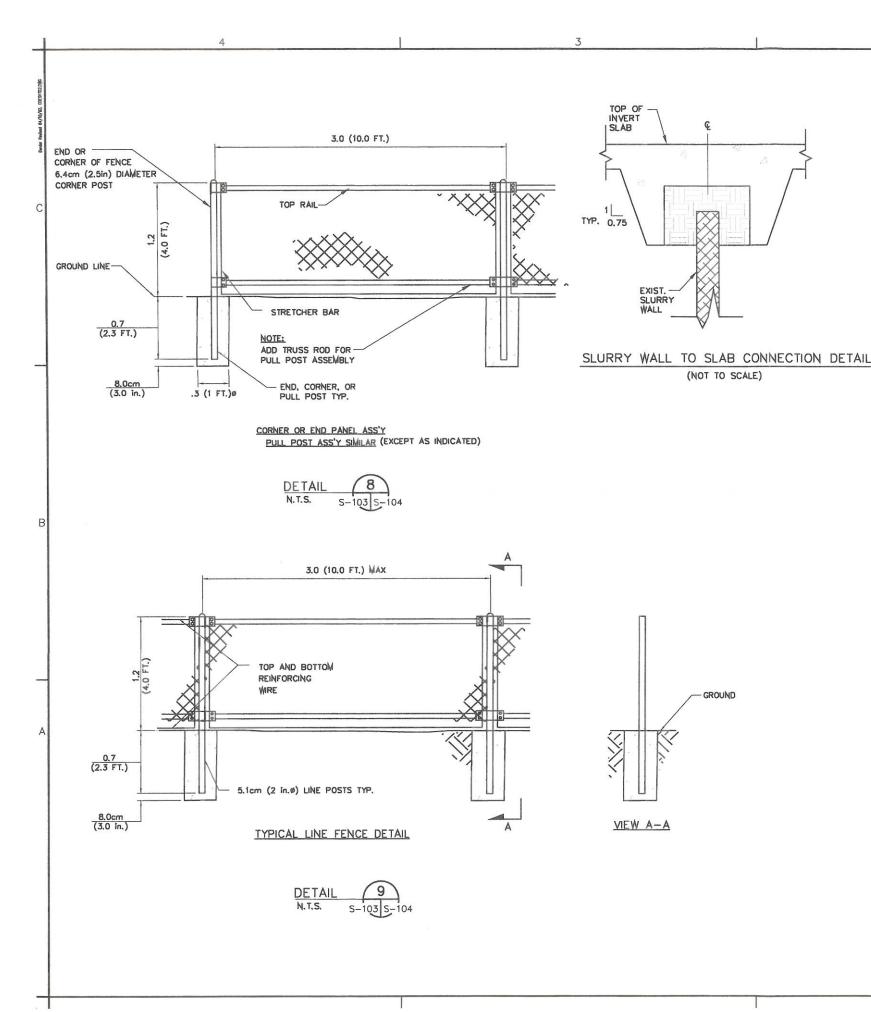


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GENERAL STRUCTURAL NOTES:

- 1. CONSTRUCTION: THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING THE JOB SITE IN A SAFE CONDITION AND EMPLOYING OSHA APPROVED CONSTRUCTION PRACTICES PERFORMED IN A WORKMANLIKE MANNER, CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE DESIGN OF SHORING, FORMWORK AND OTHER TEMPORARY CONSTRUCTION STRUCTURES. CONSTRUCTION MATERIALS SHALL BE SPREAD OUT TO AVOID OVERLOADING INDIVIDUAL MEMBERS. PERIODIC JOB SITE VISITS BY THE OWNER AND/OR ENGINEER DO NOT CONSTITUTE REVIEW OR APPROVAL OF CONSTRUCTION TECHNIQUES BUT ARE INTENDED SOLELY FOR THE PURPOSE OF CHECKING GENERAL COMPLIANCE OF THE WORK WITH THE PROJECT CONSTRUCTION DOCUMENTS.
- 2. THE DRAWINGS: THESE DRAWINGS SHALL BE READ AND COORDINATED WITH ALL OTHER CONTRACT DRAWINGS, INCLUDING VENDOR DRAWINGS AND SUBCONTRACTOR SHOP DRAWINGS. SPECIFIC DIMENSIONS, DETAILS AND NOTES SHALL TAKE PRECEDENCE OVER GENERAL NOTES, TYPICAL DETAILS, MINIMUM CODE VALUES OR SCALED INFORMATION ON PLANS OR SECTIONS. ITEMS NOT SPECIFICALLY DETAILED SHALL GENERALLY FOLLOW OTHER SIMILAR DETAILS. DISCREPANCIES, CONFLICTS AND/OR UNCLEAR CONDITIONS SHALL BE BROUGHT TO THE ATTENTION OF THE CONTRACTING OFFICER AND/OR ENGINEER IMMEDIATELY.
- THE CONTRACTOR SHALL VERIFY ALL CONTROLLING FIELD AND EXISTING CONCRETE DIMENSIONS BEFORE ORDERING OR FABRICATING ANY MATERIAL.
- ALL FORMED CONCRETE SURFACES SHALL HAVE CLASS "C" FINISH EXCEPT FORMED CONCRETE SURFACES AGAINST WHICH BACKFILL IS PLACED SHALL HAVE CLASS "D" FINISH. SEE SPEC. SECTION 03 30 04.
- 5. ALL EXPOSED EDGES OF CONCRETE SHALL BE CHAMFERED 19 mm UNLESS OTHERWISE SHOWN,
- ALL FINISHED SURFACES WHERE A FLOAT OR TROWEL FINISH ARE REQUIRED SHALL HAVE TOOLED EDGES.
- ALL REINFORCEMENT SPLICES, UNLESS OTHERWISE NOTED, SHALL CONFORM TO CLASS "B' SPLICE FOR fc'=35 MPa (5000 psi), AS SHOWN BELOW:

	DEVELOPMENT LENGTH	SCHEDULE
Bar Size	ld (Metric (English))	Splice (Metric (English))
#25 (#8)	87cm (34in)	112cm (44in)
#19 (#6)	65cm (26in)	85cm (33in)
#16 (#5)	55cm (22in)	71cm (28in)

- 8. ALL BENDS AND HOOKS SHALL BE "STANDARD BENDS AND HOOKS" AS DETAILED IN ACI 318M-05.
- UNLESS OTHERWISE SHOWN, STAGGER SPLICES IN ALTERNATE LONGITUDINAL SLAB REINFORCEMENT AT LEAST 1.2 METERS.

MATERIAL NOTES:

- 1. CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 35 MPa (5000 psi) AT 28 DAYS (ACI 318M-05).
- REINFORCING STEEL SHALL HAVE A MINIMUM YIELD STRENGTH OF 420 MPg (60 ksi) UNLESS NOTED OTHERWISE, AND SHALL CONFORM TO ASTM A 615. SEE SPECIFICATION SECTION 03 30 04.
- 3. ALL DOWELS SHALL BE STAINLESS STEEL BARS CONFORMING TO ASTM A615, UNLESS NOTED OTHERWISE.
- 4. ALL STRUCTURAL STEEL SHALL CONFORM TO ASTM SPECIFICATION A-36. UNLESS NOTED OTHERWISE.

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> reference number: S-106

Sheet 21 of 21

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January 19, 2011

Kent Zenobia
Department of Water Resources
Department of Flood Management
Project Delivery Branch - CVFPB/USACE Projects 3310 El Camino Avenue, Room 180
Sacramento, CA 95821

SUBJECT: AMERICAN RIVER COMMON FEATURES PROGRAM REMAINING SITES – SITE R4:

Dear Mr. Zenobia:

The U.S. Army Corps of Engineers recently reconstructed the outlet works of the gravity drains at the R4 site near Cal Expo. Representatives of the Sacramento Area Flood Control Agency (SAFCA) reviewed the design, visiting the site during construction and have reviewed the as-built plans and Operation and Maintenance (O&M) Manual.

We find the project to be acceptable to SAFCA and we are agreeable to accepting O&M responsibility for the site.

If you have any questions, please contact me at (916) 874-8737.

Sincerely,

Richard M. Johnson

Interim Executive Director

Sacramento Area Flood Control Agency

cc: Tim Kerr, ARFCD

Grant Kreinberg, SAFCA

Attachment F- Written Notice of Acceptance of Completed Work



DEPARTMENT OF THE ARMY

SACRAMENTO DISTRICT, CORPS OF ENGINEERS
VALLEY RESIDENT OFFICE
2021 Jefferson Boulevard
West Sacramento, California 95691

April 30, 2010

C-0006

Contract No. W91238-09-D-0020, T.O. 0001 Repair Channel Outlet Structure R4, Sacramento, California

Subject: "Beneficial Occupancy of Repair Channel Outlet Structure R4"

RDA Contracting, Inc. 2900 Main Street, Ste. 208 Alameda, CA 94501

Gentlemen,

Construction of the Repair Channel Outlet Structure R4 project was accepted by USACE on September 1, 2009 without any outstanding deficiencies. Per our letter number C-0005 (dated April 30, 2010) all SWPPP obligations have been met and are considered terminated.

This is to inform you that, in the event it should become necessary to implement any of the warranty clauses of this contract, the Government will notify you. A prompt reply is expected to any request for repairs necessary under the warranty provisions of your contract.

Sincerely,

Cathy Wise, P.E., CCM Administrative Contracting Officer

cc: Contr Div Con-ops Div PM – John Hoge Reading File File

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CENTRAL VALLEY FLOOD PROTECTION BOARD

3310 El Camino Ave., Rm. 151 SACRAMENTO, CA 95821 (916) 574-0609 FAX: (916) 574-0682 PERMITS: (916) 574-0685 FAX: (916) 574-0682



January 28, 2011

Colonel William J. Leady District Engineer Sacramento District U.S. Army Corps of Engineers 1325 J Street Sacramento, California 95814-2922

Dear Colonel Leady,

In your July 1, 2010 letter to the Central Valley Flood Protection Board (Board), the U.S. Army Corps of Engineers transferred the recently completed portion of work (R4) of the American River Common Features Project to the Board, performed under the authority of the Water Resources Development Act of 1996 (WRDA 1996). Site R4 is located approximately 250 feet south of the Strong Ranch Slough Pump Station. You advised us that the work meets the requirements of the existing Operation and Maintenance Manual, American River – Part No. 1, Levee Construction from Carmichael Bluffs Downstream 8.3 miles. We understand that the work was completed in accordance with the original plans and specifications. We acknowledge receipt of As-Built Drawings and will forward a copy to the Sacramento Area Flood Control Agency (SAFCA). We acknowledge that the Corps has provided additions and revisions to the existing Operation and Maintenance Manual and that these additions and revisions are to be incorporated into our copies of the Operation and Maintenance Manual.

The Board, on behalf of the State of California, accepted the completed portion of work at its January 28, 2011 meeting and transferred the operation, maintenance, repair, replacement, and rehabilitation of Site R4 work to SAFCA.

If you have any questions, you may contact Kent Zenobia, Project Delivery Branch Chief, at (916) 574-2639.

Sincerely,

Jay Punia Chief Executive Officer

cc: (Please see attached list of cc's)

Colonel William J. Leady January 28, 2010 Page 2 of 2

cc: Mr. Randy Olsen, Chief
Readiness Branch
Sacramento District
U.S. Army Corps of Engineers
1325 J Street
Sacramento, California 95814-2922

Mr. Richard Johnson, Interim Executive Director Sacramento Area Flood District 1007 7th Street Sacramento, California 95814

CENTRAL VALLEY FLOOD PROTECTION BOARD

3310 El Camino Ave., Rm. 151 SACRAMENTO, CA 95821 (916) 574-0609 FAX: (916) 574-0682 PERMITS: (916) 574-0685 FAX: (916) 574-0682



January 28, 2011

Mr. Richard Johnson Interim Executive Director Sacramento Area Flood Control District 1007 7th Street Sacramento, California 95814

Dear Mr. Johnson,

The U.S. Army Corps of Engineers (USACE) has recently completed the portion of work (R4) of the American River Common Features Project, performed under the authority of the Water Resources Development Act of 1996 (WRDA 1996). Richard Johnson, Interim Executive Director for the Sacramento Area Flood Control District (SAFCA) sent a letter dated January 19, 2011 to Kent Zenobia, Department of Water Resources, Project Delivery Branch Chief, which provided assurance of acceptance for the completed portion of work. The USACE has transferred the completed portion to the State of California for operation and maintenance (see enclosed letter). At its January 28, 2011 meeting, on behalf of the State of California, The Central Valley Flood Protection Board accepted the completed portion of work.

The Board hereby transfers the completed portion of work (R4) of the American River Common Features Project to SAFCA for operation, maintenance, repair, replacement, and rehabilitation. The USACE has advised the Board that the completed portion of work meets the requirements of the existing Operation and Maintenance Manual, American River – Part No. 1, Levee Construction from Carmichael Bluffs Downstream 8.3 miles. The USACE has provided additions and revisions to the existing O&M Manual and that these additions and revisions are to be incorporated into your copies of the Operation and Maintenance Manual. SAFCA will perform operation and maintenance in accordance with the amended Operation and Maintenance Manual. As-Built Drawings are enclosed.

If you have any questions, you may contact Kent Zenobia, Project Delivery Branch Chief, at (916) 574-2639.

Sincerely,

Jay S. Punia Chief Executive Officer

cc: (Please see attached list)

Mr. Richard Johnson January 28, 2011 Page 2

cc: Colonel William J. Leady, P.E.
District Engineer
Sacramento District
U.S. Army Corps of Engineers
1325 J Street
Sacramento, California 95814-2922

Mr. Randy Olsen, Chief Readiness Section Sacramento District U.S. Army Corps of Engineers 1325 J Street Sacramento, California 95814-2922