Meeting of the Central Valley Flood Protection Board August 24, 2018

Permit Staff Report

Merced County Black Rascal Creek Bridge, Merced County

1.0 - ITEM

Consider approval of Permit No. 19312 (Attachment B).

2.0 - APPLICANT

Merced County (County).

3.0 - PROJECT LOCATION

The project is located south of Yosemite Avenue, approximately 3,900 feet north of the intersection of Bradely Lateral Road and East Olive Avenue in Merced County (Black Rascal Creek, Merced County, see Attachment A).

4.0 - PROJECT DESCRIPTION

The County proposes to remove an existing private bridge and to construct three new bridges for a new roadway crossing Black Rascal Creek. The northbound bridge will be single span, 38.5 feet long and 39 feet wide, and the southbound bridge will be single span, 38.5 feet long and 53.5 feet wide. The third bridge will be a private bridge to replace the existing private bridge and will be constructed just downstream of the proposed two new bridges. The new private bridge will be single span, 38.5 feet long and 17 feet wide.

The three new bridges will be clear span bridges over the creek. Bents and abutments will be located outside the channel and banks (see Attachment C).

5.0 – AUTHORITY OF THE BOARD

California Water Code § 8534, 8590 – 8610.5, and 8700 – 8710

California Code of Regulations Title 23, Division 1 (Title 23)

- § 6, Need for a Permit
- § 13.3, Consent Calendar
- § 112, Streams Regulated and Nonpermissible Work Periods
- § 121, Erosion Control
- § 128, Bridges

6.0 - PROJECT ANALYSIS

Black Rascal Creek is listed as a regulated stream in Title 23, Article 8, Section 112, Table 8.1. There are no levees along Black Rascal Creek in the project area. The proposed project will be in compliance with all Title 23 standards. The three new bridge structures will span over Black Rascal Creek. Two of the bridges will carry northbound and southbound Campus Parkway traffic. The third bridge will replace an existing private bridge that connects farms bisected by the creek.

6.1 - Hydraulic Analysis

The design flow for Black Rascal Creek is 320 cubic feet per second (cfs). HEC-RAS, a one-dimensional hydraulic model developed by the United States Army Corps of Engineers (USACE), was used to analyze the potential hydraulic impacts due to the project. The design flow was evaluated for existing and proposed conditions.

The hydraulic analysis shows that the lowest point of the proposed new bridges will be approximately 3.8 feet for the northbound bridge and 3.65 feet for the southbound bridge above the design water surface elevation (DWSE) at the design flow. In addition, the hydraulic analysis shows 0.06-foot increase for the northbound bridge and 0.03-foot increase for the southbound bridge in DWSE. It also shows a 0.07 foot per second decrease for the northbound bridge, 0.12 foot per second increase for the southbound bridge in velocity at the design flow.

The existing private bridge is approximately 2.33 feet below the DWSE at the design flow. The lowest point of the proposed new private bridge will be approximately 2 feet above the DWSE at the design flow, an improvement of more than 4.33 feet. In addition, the hydraulic analysis shows no change in DWSE and 0.19 foot per second increase for the new private bridge in velocity at the design flow (see Attachment D).

6.2 - Geotechnical Analysis

There are no levees associated with this project; therefore, a geotechnical analysis was not required.

7.0 - AGENCY COMMENTS AND ENDORSEMENTS

The comments and endorsements associated with this project, from all pertinent agencies are shown below:

- There are no Local Maintaining Agencies in the area for the proposed bridge project.
- The USACE Sacramento District Engineer has no comments or recommendations regarding flood control because the proposed work does not affect a federally constructed project.

8.0 - CEQA ANALYSIS

Board staff has prepared the following California Environmental Quality Act (CEQA) determination:

The Board, acting as a responsible agency under CEQA, has independently reviewed the Draft and Final Environmental Impact Reports (SCH No. 2000121003, April 2005, DEIR, February 2013, FEIR, November 2005), Mitigation Monitoring and Reporting Plan (MMRP) and the Addendum (June 2018) for the Campus Parkway Project, prepared by the CEQA lead agency, Merced County. These documents, including project design, may be viewed or downloaded from the Board website at http://www.cvfpb.ca.gov/event/August-2018-board-meeting.cfm under a link for this agenda item, and are also available for review in hard copy at the Board and Merced County offices.

Merced County determined that the project, as described in the DEIR and FEIR, would have a significant effect on the environment and filed a Notice of Determination with the Merced County Clerk on December 20, 2006, and with the State Clearinghouse on December 26, 2006. Merced County completed an Addendum (June 2018) that re-validated the DEIR and FEIR and concluded only minor technical changes or additions to the previous document were necessary and that it need not be circulated for public review per CEQA Guidelines Section 15164.

The County incorporated mandatory mitigation measures into the project plans to avoid or mitigate impacts. These mitigation measures, included in the County's Final EIR and MMRP, address impacts to aesthetics, biological resources, geology, hazards and hazardous materials, hydrology and noise. These mitigation measures are within the responsibility and jurisdiction of the County and have been adopted by the County. The Draft and Final EIR found less than significant impacts under hydrology for flood related impacts associated project.

In accordance with CEQA Guidelines Section 15096(e), Board staff independently reviewed the County's DEIR, FEIR, and Addendum, and finds these environmental documents prepared by the lead agency adequately address hydrology impacts, including potential flood risk, for the Board's approval of Permit 19312 to authorize work to construct the bridges over the regulated stream, which is within the Board's jurisdiction as it relates to maintenance of the State's flood control system. The Board, as a responsible agency, is responsible for mitigating and avoiding only the direct and indirect environmental effects of those parts of the project which it decides to carry out, finance, or approve (CEQA Guidelines Section 15096(g); Public Resources Code § 21002.1(d)).

Here, the Board's action is limited to approving an encroachment permit for work to construct and operate the replacement bridges, and the Board's jurisdiction is limited to imposing conditions or mitigation related to maintaining the State Plan of Flood Control. The mitigation measures in the County's EIR and MMRP do not address issues over which the Board has jurisdiction, therefore, no specific findings under CEQA Guidelines section 15906, subdivision (h) are required; these mitigation measures are within the jurisdiction of the County, and have been adopted by the County.

The Draft and Final EIR and Addendum identified less than significant impacts related to flood risk, which is the one resource area within the Board's jurisdiction as a responsible agency. The EIR conclusions related to flood risk are further supported by the USACE hydraulic model analysis relied upon by Board staff, which confirm the proposed bridges will result in less than significant hydraulic impacts. Based on staff's review of the environmental documents, the hydrologic analysis, and the entirety of the record, staff finds there is no substantial evidence to support a fair argument that the project may result in significant impacts related to flood risk within the Board's jurisdiction. Because the Board's approval of the encroachment permit for the proposed bridges results in less than significant impacts related to flood risk, which is the only resource area within the Board jurisdiction to address, no

findings under CEQA Guidelines section 15906, subdivision (h) or consideration of alternatives is required.

The documents and other materials which constitute the record of the Board's proceedings in this matter are in the custody of the Executive Officer, Central Valley Flood Protection Board, 3310 El Camino Ave., Suite 170, Sacramento, California 95821.

9.0 - CALIFORNIA WATER CODE SECTION 8610.5 CONSIDERATIONS

- 1. Evidence that the Board admits into its record from any party, federal, State or local public agency, or nongovernmental organization with expertise in flood or flood plain management:
 - The Board has considered all the evidence presented in this matter, including the application for Permit No. 19312, and all supporting hydraulic and other technical documentation provided by the County.
- 2. The best available science that relate to the scientific issues presented by the Executive Officer, legal counsel, the Department of Water Resources or other parties that raise credible scientific issues:
 - The accepted industry standards for the work proposed under this permit as regulated by Title 23 have been applied to the review of this permit. On the issue of hydraulic impacts, the County developed and applied a HEC-RAS hydraulic model. This model is considered one of the best available scientific tools for the purpose of evaluating DWSE changes due to the proposed project.
- Effects of the decision on the facilities of the State Plan of Flood Control (SPFC), and consistency of the proposed project with the Central Valley Flood Protection Plan Update (CVFPP) as adopted by Board Resolution 2017-10 on August 25, 2017:
 - The proposed project is located approximately 1.6 miles downstream of any SPFC facilities and will improve existing hydraulic conditions by increasing the conveyance area under the bridges. The proposed project is consistent with the adopted 2017 CVFPP as it reduces the chance of flooding by increasing the conveyance area.
- 4. Effects of reasonable projected future events, including, but not limited to, changes in hydrology, climate, and development within the applicable watershed:

The proposed project will be constructed with at least 2 feet of clearance above the DWSE. The new bridges will better accommodate changes in hydrology due to climate change. Therefore, there are no expected adverse effects to the proposed project from reasonable projected future events.

10.0 - STAFF RECOMMENDATION

Board staff recommends that the Board:

Adopt:

CEQA finding: The Board, acting as a responsible agency under CEQA, has
independently reviewed and considered the environmental documents
prepared for the project. Approving the Permit 19312 would not result in any
significant adverse impacts related to flood risk and no additional mitigation
measures within the Board's jurisdiction are required.

Approve:

 Encroachment Permit No. 19312 in substantially the form provided in Attachment B; and

Direct:

 The Executive Officer to take the necessary actions to execute the permit and file a Notice of Determination pursuant to CEQA with the State Clearinghouse.

11.0 – LIST OF ATTACHMENTS

A. Location Maps and Photos

B. Draft Permit No. 19312

C. Project Drawings

D. Hydraulic Profile Information

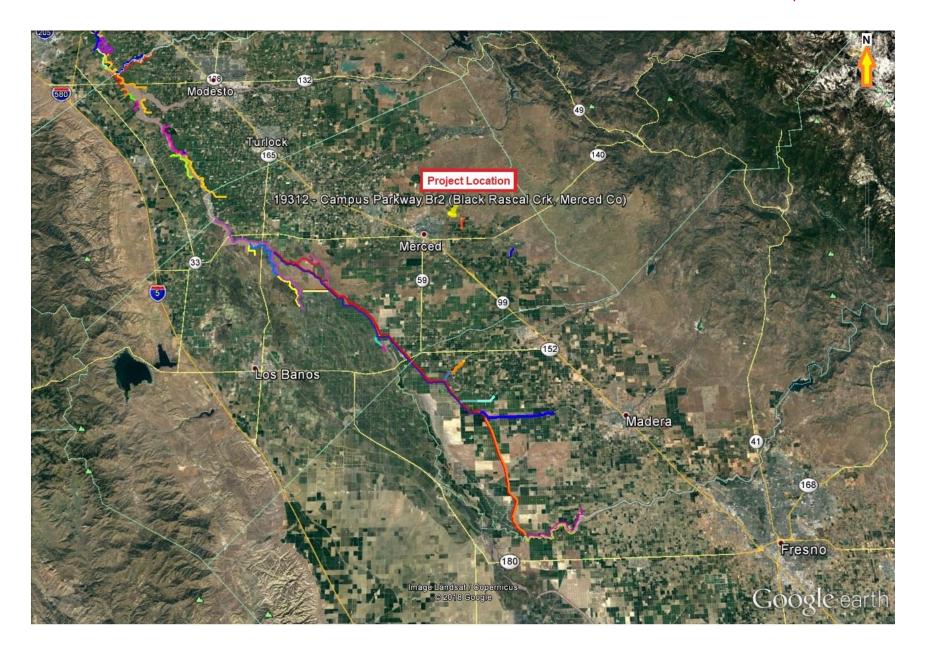
Design Review: Sungho Lee, PE, Engineer, Water Resources, Permitting Section

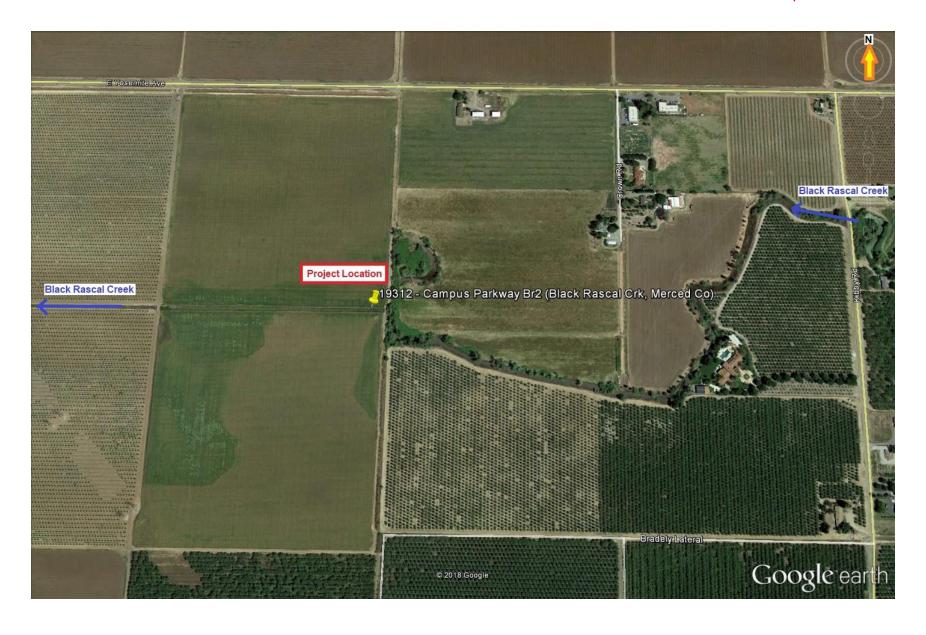
Environmental Review: James Herota, Senior Environmental Scientist

Document Review: Gary Lemon, PE, Senior Engineer, Permitting Section Chief

Kelly Soule, PE, Acting Operations Branch Chief Michael C. Wright, PE, Acting Chief Engineer

Legal Review: Christina Morkner Brown, Deputy Attorney General





No. 19312 - Attachment A - Location Maps and Photos

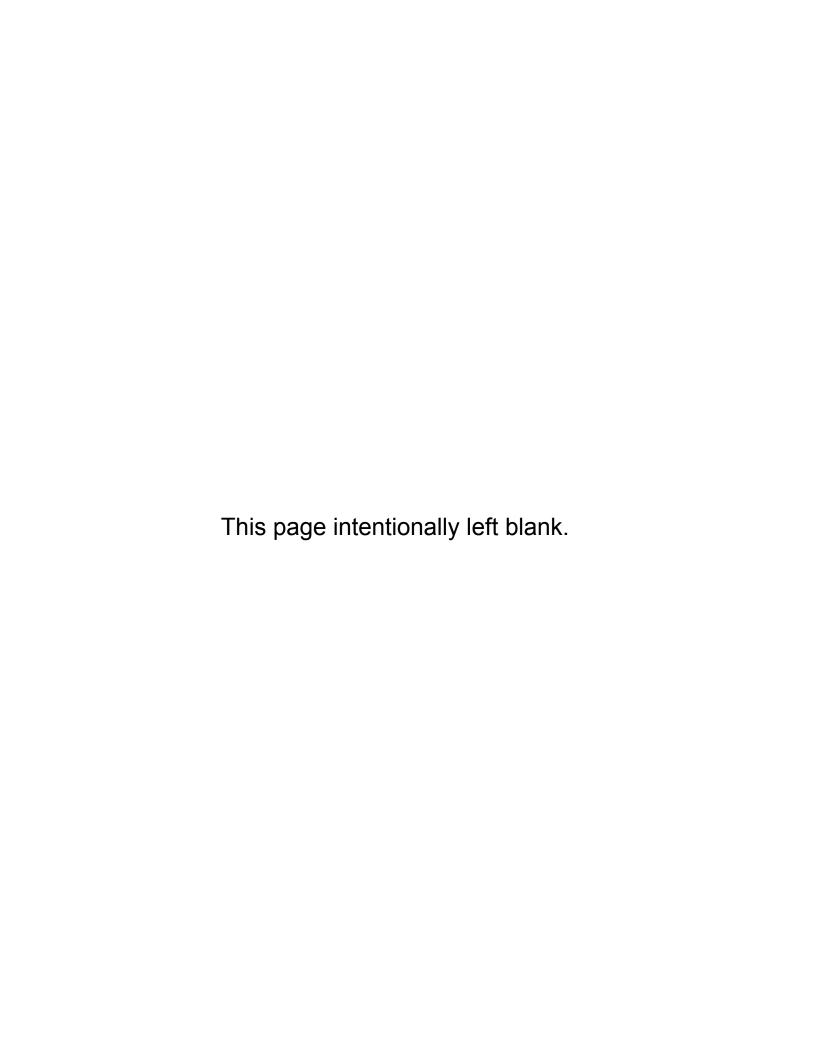




Photo 1: View of Existing Structure crossing Black Rascal Creek, looking South.



Photo 2: View from Existing Structure of Black Rascal Creek, looking East.



DRAFT

STATE OF CALIFORNIA THE RESOURCES AGENCY

THE CENTRAL VALLEY FLOOD PROTECTION BOARD

PERMIT NO. 19312 BD

This Permit is issued to:

Merced County 345 W. 7th Street Merced, California 95340

To construct three new bridges for a new roadway crossing Black Rascal Creek. The left bridge for southbound lane will be single span, 38.5 feet long, and 53.5 feet wide and the right bridge for northbound will be single span, 38.5 feet long, and 39 feet wide. A third, private bridge will replace an existing private bridge. It will be constructed just downstream of proposed two new bridges. The new private bridge will be single span, 38.5 feet long, and 17 feet wide. Three new bridges will be clear span bridge over the creek. Bents and abutments will be located outside the channel and banks.

The project is located south of Yosemite Avenue, approximately 3,900 feet north of the intersection of Bradely Lateral and E. Olive Avenue and crossing Black Rascal Creek in Merced County, at 37.32785°N 120.42382°W, Black Rascal Creek, Merced County.

NOTE: Special Conditions have been incorporated herein which may place limitations on and/or require modification of your proposed project

as described above.

(SEAL)	
Dated:	

GENERAL CONDITIONS:

ONE: This permit is issued under the provisions of Sections 8700 – 8723 of the Water Code.

TWO: Only work described in the subject application is authorized hereby.

THREE: This permit does not grant a right to use or construct works on land owned by the Sacramento and San Joaquin Drainage District or on any other land.

FOUR: The approved work shall be accomplished under the direction and supervision of the State Department of Water Resources, and the permittee shall conform to all requirements of the Department and The Central Valley Flood Protection Board.

FIVE: Unless the work herein contemplated shall have been commenced within one year after issuance of this permit, the Board reserves the right to change any conditions in this permit as may be consistent with current flood control standards and policies of The Central Valley Flood Protection Board.

SIX: This permit shall remain in effect until revoked. In the event any conditions in this permit are not complied with, it may be revoked on 15 days' notice.

SEVEN: It is understood and agreed to by the permittee that the start of any work under this permit shall constitute an acceptance of the conditions in this permit and an agreement to perform work in accordance therewith.

EIGHT: This permit does not establish any precedent with respect to any other application received by The Central Valley Flood Protection Board.

NINE: The permittee shall, when required by law, secure the written order or consent from all other public agencies having jurisdiction.

TEN: The permittee is responsible for all personal liability and property damage which may arise out of failure on the permittee's part to perform the obligations under this permit. If any claim of liability is made against the State of California, or any departments thereof, the United States of America, a local district or other maintaining agencies and the officers, agents or employees thereof, the permittee shall defend and shall hold each of them harmless from each claim.

ELEVEN: The permittee shall exercise reasonable care to operate and maintain any work authorized herein to preclude injury to or damage to any works necessary to any plan of flood control adopted by the Board or the Legislature, or interfere with the successful execution, functioning or operation of any plan of flood control adopted by the Board or the Legislature.

TWELVE: Should any of the work not conform to the conditions of this permit, the permittee, upon order of The Central Valley Flood Protection Board, shall in the manner prescribed by the Board be responsible for the cost and expense to remove, alter, relocate, or reconstruct all or any part of the work herein approved.

SPECIAL CONDITIONS FOR PERMIT NO. 19312 BD

LIABILITY AND INDEMNIFICATION

THIRTEEN: The permittee shall defend, indemnify, and hold the Central Valley Flood Protection Board (Board) and the State of California, including its agencies, departments, boards, commissions, and their respective officers, agents, employees, successors and assigns (collectively, the "State"), safe and harmless, of and from all claims and damages related to the Board's approval of this permit, including but not limited to claims filed pursuant to the California Environmental Quality Act. The State expressly reserves the right to supplement or take over its defense, in its sole discretion.

FOURTEEN: The permittee is responsible for all liability associated with construction, operation, and maintenance of the permitted facilities and shall defend, indemnify, and hold the Board and the State, safe and harmless, of and from all claims and damages arising from the project undertaken pursuant to this permit, all to the extent allowed by law. The State expressly reserves the right to supplement or take over its defense, in its sole discretion.

FIFTEEN: The Board and Department of Water Resources shall not be held liable for any damages to the permitted encroachment(s) resulting from releases of water from reservoirs, flood fight, operation, maintenance, inspection, or emergency repair.

AGENCY CONDITIONS

SIXTEEN: All work approved by this permit shall be in accordance with the submitted drawings and specifications dated December 2, 2011 except as modified by special permit conditions herein. No further work, other than that approved by this permit, shall be done in the area without prior approval of the Board.

SEVENTEEN: Correspondence was received from the Department of the Army (U.S. Army Corps of Engineers, Sacramento District) dated June 26, 2018, signifying that the District Engineer has no comments or recommendations regarding flood control because the proposed project does not affect a federally constructed project.

EIGHTEEN: Permittee shall pay to the Board, an inspection fee(s) to cover inspection cost(s), including staff and/or consultant time and expenses, for any inspections before, during, post-construction, and regularly thereafter as deemed necessary by the Board.

NINETEEN: The permittee shall be responsible for the repair of any damages to the channel, banks, floodway, or other flood control facilities due to construction, operation, or maintenance of the proposed project.

TWENTY: In the event that levee or bank erosion injurious to the adopted plan of flood control occurs at or adjacent to the permitted encroachment(s), the permittee shall repair the eroded area and propose measures, to be approved by the Board, to prevent further erosion.

TWENTY-ONE: The Board reserves the right to add additional, or modify existing, conditions when there is a change in ownership and/or maintenance responsibility of the work authorized under this permit.

TWENTY-TWO: The permittee agrees to notify new property/encroachment owner(s) that they are required to submit a permit Name Change request form to the Board upon completion of the sale. The new owner(s) will be required to comply with all permit conditions. Name Change forms are available at http://cvfpb.ca.gov/

PRE-CONSTRUCTION

TWENTY-THREE: Upon receipt of a signed copy of the issued permit the permittee shall contact the Board by telephone at (916) 574-0609, and submit the enclosed postcard, to schedule a preconstruction conference with the inspector that is assigned to your project. Failure to do so at least 10 working days prior to start of work may result in a delay of the project.

CONSTRUCTION

TWENTY-FOUR: No construction work of any kind shall be done during the flood season from November 1 to April 15 without prior approval of the Board. Failure to submit a Time Variance Request to the Board at least 10 working days prior to November 1 may result in a delay of the project.

TWENTY-FIVE: Piers, bents, and abutments being dismantled shall be removed to at least 1 foot below the natural ground line and at least 3 feet below the bottom of the low-water channel.

TWENTY-SIX: Backfill material for excavations within the bank section and within 10 feet of bridge supports within the floodway shall be placed in 4- to 6-inch layers and compacted to a minimum of 90 percent relative compaction as measured by the current ASTM D1557 standard and above optimum moisture content.

TWENTY-SEVEN: No material stockpiles, temporary buildings, or equipment shall remain in the Black Rascal Creek floodway during the flood season from November 1 to April 15.

POST-CONSTRUCTION

TWENTY-EIGHT: All debris generated by this project shall be disposed outside of the Black Rascal Creek floodway.

TWENTY-NINE: Cleared trees and brush shall be completely burned or removed from the Black Rascal Creek floodway, and downed trees or brush shall not remain in the Black Rascal Creek floodway during the flood season from November 1 to April 15.

THIRTY: The work area shall be restored to at least the condition that existed prior to commencement of work.

OPERATIONS AND MAINTENANCE

THIRTY-ONE: The permittee shall maintain the permitted encroachment(s) in the manner required and as requested by the authorized representative of the Board, the Department of Water Resources, or any other agency responsible for maintenance and shall, at all times, allow officials from these agencies to access any adjacent areas as necessary for flood control.

THIRTY-TWO: The permitted encroachment(s) shall not interfere with operation and maintenance of the flood control project. If the permitted encroachment(s) are determined by any agency responsible for operation or maintenance of the flood control project to interfere, the permittee shall be required, at permittee's cost and expense, to modify or remove the permitted encroachment(s) under direction of the Board or Department of Water Resources. If the permittee does not comply, the Board may modify or remove the encroachment(s) at the permittee's expense.

THIRTY-THREE: After each period of high water, debris that accumulates at the site shall be completely removed from the Black Rascal Creek floodway.

THIRTY-FOUR: If erosion occurs adjacent to the permitted encroachment(s), the permittee shall repair the eroded areas and place adequate revetment on the affected areas to prevent further erosion.

THIRTY-FIVE: If the bridge is damaged to the extent that it may impair the flow capacity in Black

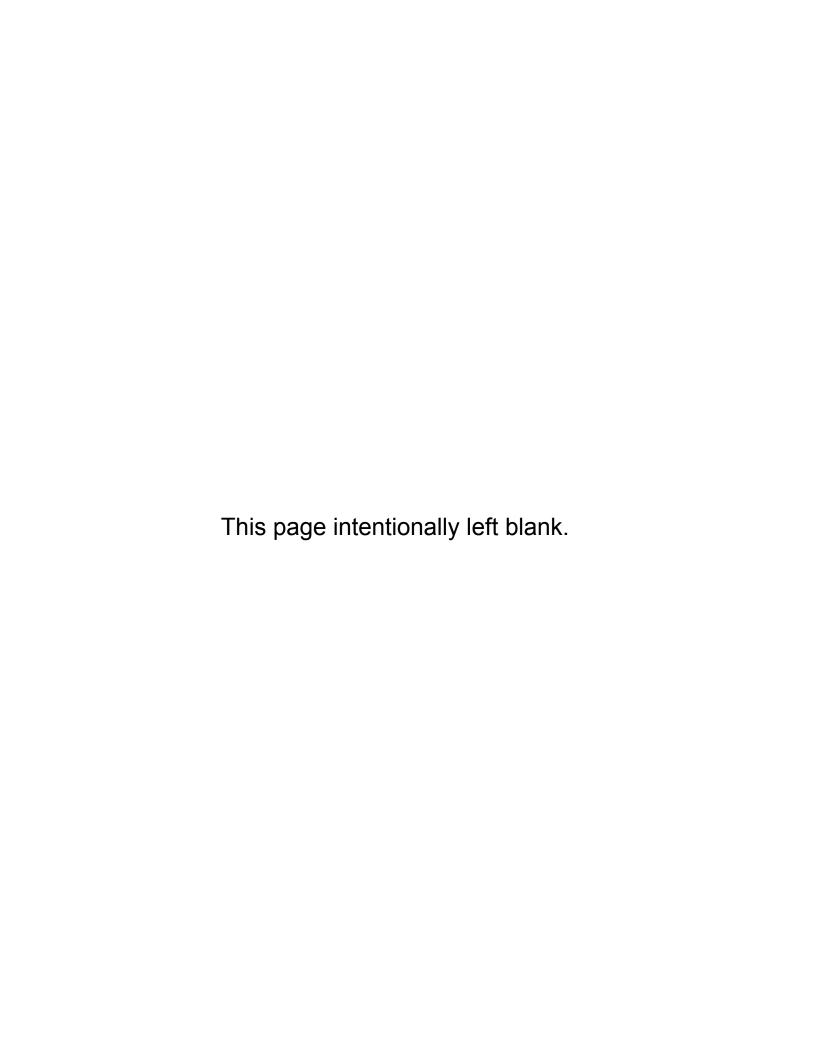
Rascal Creek, it shall be repaired or removed prior to the next flood season.

PROJECT ABANDONMENT, CHANGE IN PLAN OF FLOOD CONTROL

THIRTY-SIX: If the project, or any portion thereof, is to be abandoned in the future, the permittee or successor shall abandon the project under direction of the Board at the permittee's or successor's cost and expense.

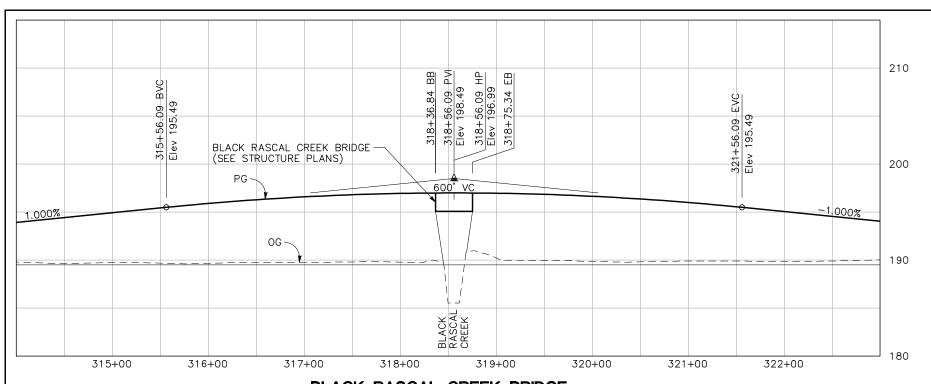
THIRTY-SEVEN: The permittee may be required, at permittee's cost and expense, to remove, alter, relocate, or reconstruct all or any part of the permitted encroachment(s) if in the discretion of the Board the removal, alteration, relocation, or reconstruction is necessary as part of or in conjunction with any present or future flood control plan or project or if the project is not maintained or is damaged by any cause. If the permittee does not comply, or in the event of an emergency, the Board may remove the encroachment(s) at the permittee's expense.

END OF CONDITIONS



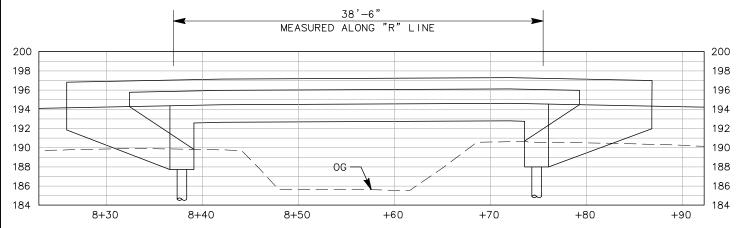
DGN FILE => 56-0211B-Phase III-T1.dwg

No. 19312 - Attachment C - Project Drawings



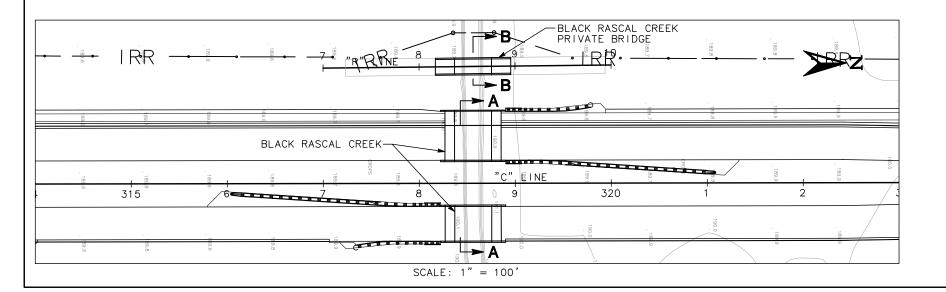
BLACK RASCAL CREEK BRIDGE

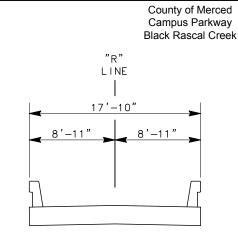
SCALE: VERTICAL 1" = 10' SCALE: HORIZONTAL 1" = 100'

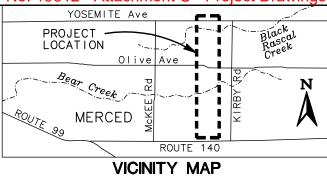


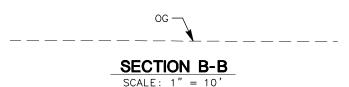
BLACK RASCAL CREEK PRIVATE BRIDGE

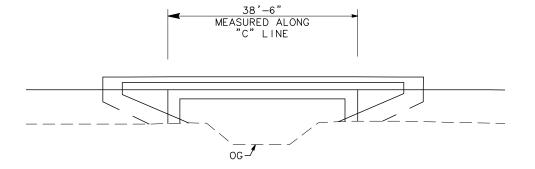
SCALE: VERTICAL 1" = 10' SCALE: HORIZONTAL 1" = 10'





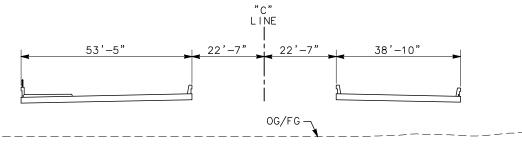






BLACK RASCAL CREEK BRIDGE

SCALE: 1" = 30'



SECTION A-A SCALE: 1" = 30'

PROPOSED CONSTRUCTION OF BLACK RASCAL CREEK

MERCED COUNTY	SHEET 1 OF 1	
345 WEST 7th ST	SHEELLOFI	
MERCED, CA 95340	CONTACT:	
DAY TIME PHONE (209) 385-7601	STEVEN ROUGH	
DATE: 02-17-12	MERCED COUNTY	

MARK THOMAS & COMPANY, INC. 1960 ZANKER ROAD

SAN JOSE, CA 95112

© Abut 2 N89'29'20"W © Abut 1 N89.29.20"W **LEGEND** Indicates Bottom of Abutment Elevation WWLOL N00'30'40"E, Typ **DATUM** WWLOL North American Vertical datum of 1988(NAVD88) based on NGS Benchmark D 1420 (PID: HS4524) 189.0 having an elevation of 182.92'. 189.0 WWLOL WWLOL "C" 318+37.75 "C" 318+74.75 22, 318+00 319+00 "C" Line N00'30'40"E WWLOL WWLOL 189.0 189.0 WWLOL N00'30'40"E, Typ **PLAN** $1^{"} = 10'$

HORZ.DATUM NAD 83

ALIGNMENT TIES

DRAFTED

CHECKED

V. SHERBY

G. BOYKO

' P. VULLIE1

DETAILS

CALE: AS SHOWN VERT.DATUM NAVD 88

OTOGRAMMETRY AS OF:

JRVEYED

OUNDATION PLAN SHEET (ENGLISH) (REV. 7/16/10)

BENCH MARKS

County of Merced Campus Parkway Black Rascal Creek

Benchmark No. 1 (Point No. 4005) Set 3/4" iron pipe with plastic plug stamped "MTCo Control" at centerline of dirt farm road north of the irrigation Control. Elevation 191.00' (NAVD88)

Benchmark No. 2 (Point No. 4006) Set 3/4" iron pipe with plastic plug stamped "MTCo Control" 12' Southeast of 12" standpipe. Elevation 190.69' (NAVD88)

320 343 REGISTERED CIVIL ENGINEER DATE PO-KANG CHEN No.___S3112__ PLANS APPROVAL DATE \Exp. 09/30/13 THE COUNTY OF MERCED OR ITS OFFICERS OR AGENTS SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF SCANNED COPIES OF THIS PLAN SHEET. STRUCTURE OF CALE OF COUNTY OF MERCED DEPARTMENT OF PUBLIC WORKS 345 W. 7TH STREET MERCED, CA 95340

HYDROLOGIC SUMMARY

Drainage Area <u>x.x Square Mile</u>

Indicates Pile (not all piles shown)

	Design Flood	Base Flood	Overtopping Flood
Frequency (Years)			
Discharge (Cubic Foot per Sec)			
Nater Surface (Elevation at Bridge)			

Flood plain data are based upon information available when the plans were prepared and are shown to meet federal requirements. The accuracy of said information is not warranted by the State and interested or affected parties should make their own investigation

PILE DATA TABLE - LEFT



Location	Pile Type	Nominal Resistance (kip)		Design Tip		
Location	Compression Tension Elevations (ft)		Elevations (ft)	Resistance (kip)		
Abut 1	Class 90 Alt "W"	190	0	146.0	146.0	190
Abut 2	Class 90 Alt "W"	190	0	142.0	142.0	190

PILE DATA TABLE - RIGHT



Location	Pile Type	Nominal Resistance (kip) Design Tip			Nominal Driving	
200011011	1116 1966	Compression	Tension	sion Elevations (Ft)	Elevations (Ft)	Resistance (kip)
Abut 1	Class 90 Alt "W"	170	0	149.0	149.0	170
Abut 2	Class 90 Alt "W"	170	0	149.0	149.0	170



Z. SIVIGLIA

Z. SIVIGLIA

V. SHERBY

ORIGINAL SCALE IN INCHES FOR REDUCED PLANS

Structural Concrete, Bridge

CONCRETE TYPE LIMITS

NO SCALE

			CAMPUS PARKWAY BRIDGE
PREPARED FOR THE COUNTY OF MERCED	J. PASSALACQUA	BRIDGE NO. 39C0383R/L	OVER BLACK RASCAL CREEK
DEPARTMENT OF PUBLIC WORKS	PROJECT ENGINEER		FOUNDATION PLAN
1 1 2 3	UNIT:	DISREGARD PRINTS EARLIER REVISION	S BEARING REVISION DATES (PRELIMINARY STAGE ONLY) SHEET OF 0 DATES 7/15/08 10/19/11 12/02/11 3 10

FILE => 03 Black Rascal Creek FP.dwg

PROJECT NUMBER & PHASE:

PROJECT ID:

→ 7<u>/15/08</u> 1<u>0</u>/12/11 12/02/11

County of Merced Campus Parkway Black Rascal Creek

326 343

REGISTERED CIVIL ENGINEER DATE GARY PARIKHE No. G.E. 666 PLANS APPROVAL DATE *\Exp.12/31/13 THE COUNTY OF MERCED OR ITS OFFICERS OR AGENTS SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF SCANNED COPIES OF THIS PLAN SHEET. GEOTECHNICAL

COUNTY OF MERCED PARIKH CONSULTANTS, INC. 2360 QUME DRIVE, SUITE A DEPARTMENT OF PUBLIC WORKS 345 W. 7TH STREET MERCED, CA 95340 SAN JOSE, CA 95131

CONSISTENCY OF COHESIVE SOILS					
Unconfined Compressive Strength (tsf)	Pocket Penetrometer Measurement (tsf)	Torvane Measurement (tsf)	Field Approximation		
< 0.25	< 0.25	< 0.12	Easily penetrated several inches by fist		
0.25 to 0.50	0.25 to 0.50	0.12 to 0.25	Easily penetrated several inches by thumb		
0.50 to 1.0	0.50 to 1.0	0.25 to 0.50	Penetrated several inches by thumb with moderate effort		
1 to 2	1 to 2	0.50 to 1.0	Readily indented by thumb but penetrated only with great effort		
2 to 4	2 to 4	1.0 to 2.0	Readily indented by thumbnail		
> 4.0	> 4.0	> 2.0	Indented by thumbnoil with difficulty		
	Compressive Strength (tsf) < 0.25 0.25 to 0.50 0.50 to 1.0 1 to 2 2 to 4	Unconfined Compressive Strength (tsf) Penetrometer Measurement (tsf) < 0.25 < 0.25 to 0.50 0.25 to 0.50 0.50 to 1.0 1 to 2 1 to 2 2 to 4 2 to 4	Unconfined Compressive Strength (tsf) Pocket Penetrometer Measurement (tsf) Co.25 Co.2		

	PLASTICITY OF FINE-GRAINED SOILS			
Description	Criterio			
Nonplastic	A 1/8-inch thread cannot be rolled at any water content.			
Low	The thread can barely be rolled and the lump cannot be formed when drier than the plastic limit.			
Medium	The thread is easy to roll and not much time is required to reach the plastic limit. The thread cannot be rerolled after reaching the plastic limit. The lump crumbles when drier than the plastic limit.			
High	It takes considerable time rolling and kneading to reach the plastic limit. The thread can be rerolled several times after reaching the plastic limit. The lump can be formed without crumbling when drier than the plastic limit.			

	BOREHOLE IDENTIFICATION				
Symbol	Hole Type	Description			
Size	Α	Auger Boring			
Size	R P	Rotary drilled boring Rotary percussion boring (air)			
ag g	R	Rotary drilled diamond core			
Size	HD HA	Hand driven (1—inch soil tube) Hand Auger			
•	D	Dynamic Cone Penetration Boring			
●	CPT	Cone Penetration Test (ASTM D 5778-95)			
	0	Other			
	Note: Size in inches.				

CAMPUS PARKWAY BRIDGE

OVER BLACK RASCAL CREEK

LOG OF TEST BORINGS

FIELD AND LABORATORY TESTING

- C Consolidation (ASTM D 2435)
- (CL) Collapse Potential (ASTM D 5333)
- (CP) Compaction Curve (CTM 216)
- Corrosivity Testing (CTM 643, CTM 422, CTM 417)
- Consolidated Undrained Triaxial (ASTM D 4767) (CI)
- (DS) Direct Shear (ASTM D 3080)
- (EI) Expansion Index (ASTM D 4829)
- M) Moisture Content (ASTM D 2216)
- (OC) Organic Content-% (ASTM D 2974)
- P Permeability (CTM 220)
- (PA) Particle Size Analysis (ASTM D 422)
- Plasticity Index (AASHTO T 90) Liquid Limit (AASHTO T 89)
- (PL) Point Load Index (ASTM D 5731)
- PM Pressure Meter
- (PP) Pocket Penetrometer
- (R) R-Value (CTM 301)
- SE) Sand Equivalent (CTM 217)
- (SG) Specific Gravity (AASHTO T 100)
- (SL) Shrinkage Limit (ASTM D 427)
- (SW) Swell Potential (ASTM D 4546)
- (TV) Pocket Torvane
- Unconfined Compression-Soil (ASTM D 2166) Unconfined Compression-Rock (ASTM D 2938)
- Unconsolidated Undrained Triaxial (ASTM D 2850)
- (UW) Unit Weight (ASTM D 4767)
- (VS) Vane Shear (AASHTO T 223)

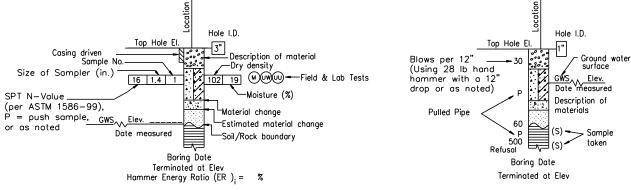
SPT N ₆₀ (Blows / 12 inches)
0 - 4
5 - 10
11 – 30
31 – 50
> 50

MOISTURE			
Description	Criteria		
Dry	Absence of moisture, dusty, dry to the touch		
Moist	Damp but no visible water		
Wet	Visible free water, usually soil is below water table		

PERCENT OR PROPORTION OF SOILS				
Description	Criteria			
Trace	Particles are present but estimated to be less than 5%			
Few	5 to 10%			
Little	15 to 25%			
Some	30 to 45%			
Mostly	50 to 100%			

	PARTICLE SIZE						
Des	cription	Size					
Boulder		> 12"					
Cobble		3" to 12"					
Gravel	Coarse	3/4" to 3"					
Gravei	Fine	No. 4 to 3/4"					
	Coarse	No. 10 to No. 4					
Sand	Medium	No. 40 to No. 10					
	Fine	No. 200 to No. 40					

	CEMENTATION							
Description	Criteria							
Weak	Crumbles or breaks with handling or little finger pressure.							
Moderate	Crumbles or breaks with considerable finger pressure.							
Strong	Will not crumble or break with finger pressure.							



TRAN

G. PARIKH

CHECKED BY

REFERENCE: CALTRANS SOIL & ROCK LOGGING, CLASSIFICATION, AND PRESENTATION MANUAL (JUNE 2007)

CL

CL-ML

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ОН

_ean CLAY

SILTY CLAY

SILT with SAND

SILT with GRAVEL SANDY SILT SANDY SILT with GRAVEL

ORGANIC lean CLAY

Fot CLAY

Elastic SIL7 Elastic SILT with SAND

GRAVELLY SILT GRAVELLY SILT with SAND

ORGANIC SILT ORGANIC SILT with SAND ORGANIC SILT with GRAVEL

Fat CLAY with SAND Fat CLAY with GRAVEL

ORGANIC lean CLAY with SAND ORGANIC lean CLAY with GRAVEL

SANDY ORGANIC SILT SANDY ORGANIC SILT with GRAVEL

GRAVELLY ORGANIC SILT
GRAVELLY ORGANIC SILT with SAND

SANDY fot CLAY
SANDY fot CLAY with GRAVEL
GRAVELLY fot CLAY

GRAVELLY fat CLAY with SAND

SANDY elostic SILT with GRAVEL
SANDY elostic SILT
SANDY elostic SILT with GRAVEL

GRAVELLY elastic SILT
GRAVELLY elastic SILT with SAND

ORGANIC fot CLAY
ORGANIC fot CLAY with SAND
ORGANIC fot CLAY with GRAVEL

ORGANIC elastic SILT
ORGANIC elastic SILT with SAND
ORGANIC elastic SILT with GRAVEL
SANDY ORGANIC elastic SILT

SANDY ORGANIC SOIL WITH GRAVEL
SANDY ORGANIC SOIL WITH GRAVEL
GRAVELLY ORGANIC SOIL

GRAVELLY ORGANIC SOIL with SAND

ORGANIC SOIL
ORGANIC SOIL with SAND
ORGANIC SOIL with GRAVEL

SANDY ORGANIC fot CLAY
SANDY ORGANIC fot CLAY
With GRAVEL
GRAVELLY ORGANIC fot CLAY

GRAVELLY ORGANIC fot CLAY with SAND

SANDY ORGANIC CHOSTIC SILT SANDY ORGANIC CHOSTIC SILT with GRAVEL GRAVELLY ORGANIC CHOSTIC SILT with SAND

SANDY ORGANIC lean CLAY SANDY ORGANIC lean CLAY with GRAVEL GRAVELLY ORGANIC lean CLAY

GRAVELLY ORGANIC lean CLAY with SAND

Lean CLAY with SAND

ean CLAY with GRAVEL

SANDY lean CLAY with GRAVEL
SANDY lean CLAY with GRAVEL
GRAVELLY lean CLAY
GRAVELLY lean CLAY with SAND

SILTY CLAY
SILTY CLAY with SAND
SILTY CLAY with GRAVEL
SANDY SILTY CLAY
SANDY SILTY CLAY with GRAVEL
GRAVELLY SILTY CLAY with SAND
GRAVELLY SILTY CLAY with SAND

Graphic/Symbol

Group Names

GROUP SYMBOLS AND NAMES

Group Names

Well-graded GRAVEL

Poorly graded GRAVEL

Well-graded GRAVEL with SAND

Poorly graded GRAVEL with SAND

Well-graded GRAVEL with SILT and SAND

Well-graded GRAVEL with CLAY and SAND or SILTY CLAY and SAND

Poorly graded GRAVEL with SILT and SAND

Well-graded GRAVEL with SILT

Well-graded GRAVEL with CLAY (or SILTY CLAY)

Poorly graded GRAVEL with SILT

Poorly graded GRAVEL with CLAY

SILTY GRAVEL

CLAYEY GRAVEL

SILTY GRAVEL with SAND

CLAYEY GRAVEL with SAND

SILTY, CLAYEY GRAVEL with SAND

Well-graded SAND with GRAVEL

Poorly graded SAND with GRAVEL

Well-graded SAND with SILT and GRAVEL

Well-graded SAND with CLAY and GRAVEL (or SILTY CLAY and GRAVEL)

Poorly graded SAND with SILT and GRAVEL

Well-graded SAND with SILT

Well-groded SAND with CLAY (or SILTY CLAY)

Poorly graded SAND with SILT

Poorly graded SAND with CLAY (or SILTY CLAY)

SILTY SAND with GRAVEL

CLAYEY SAND with GRAVEL

COBBLES and BOULDERS
BOULDERS

SILTY, CLAYEY SAND with GRAVEL

SILTY, CLAYEY SAND

SILTY SAND

CLAYEY SAND

PEAT

GS GEOTECHNICAL LOG OF TEST BORINGS SHEET (ENGLISH) (REV. 7/16/10)

Poorly graded SAND with CLAY and GRAVEL (or SILTY CLAY and GRAVEL

SILTY, CLAYEY GRAVEL

Well-graded SAND

Poorly graded SAND

Poorly graded GRAVEL with CLAY and SAND (or SILTY CLAY and SAND)

Graphic/Symbol

GW

GP

GP-GN

GM

GC

GC-GN

SW

SW-SC

SP-SM

SP-SC

SM

SC

22 22

00

DESIGN OVERSIGHT

SIGN OFF DATE

4

, e e .

8 4

Hole I.D Top Hole El. Hole I.D. Top Hole El. Pressure measured No count recorded __ GWS Elev. along sleeve friction Pressure measured 2 Pushed element (34,88 in Date on tip element area) divided by Driving rate in (2.33 in ²area) pressure measured 20 Friction Ratio (%) Tip Bearing (MPa) 100 Boring Date Borina Date

DYNAMIC CONE PENETRATION BORING

..S. BHANGOO FIELD INVESTIGATION BY DATE:

PREPARED FOR THE COUNTY OF MERCED DEPARTMENT OF PUBLIC WORKS

PROJECT NUMBER & PHASE: X

FILE => \$REQUEST

CONTRACT NO.: X

SHEET OF

9

seconds per 12" (using a Stanley MB 156 percussion hammer and a 2.2" cone, or as noted) **ROTARY BORING** HAND BORING CONE PENETRATION TEST (CPT) SOUNDING

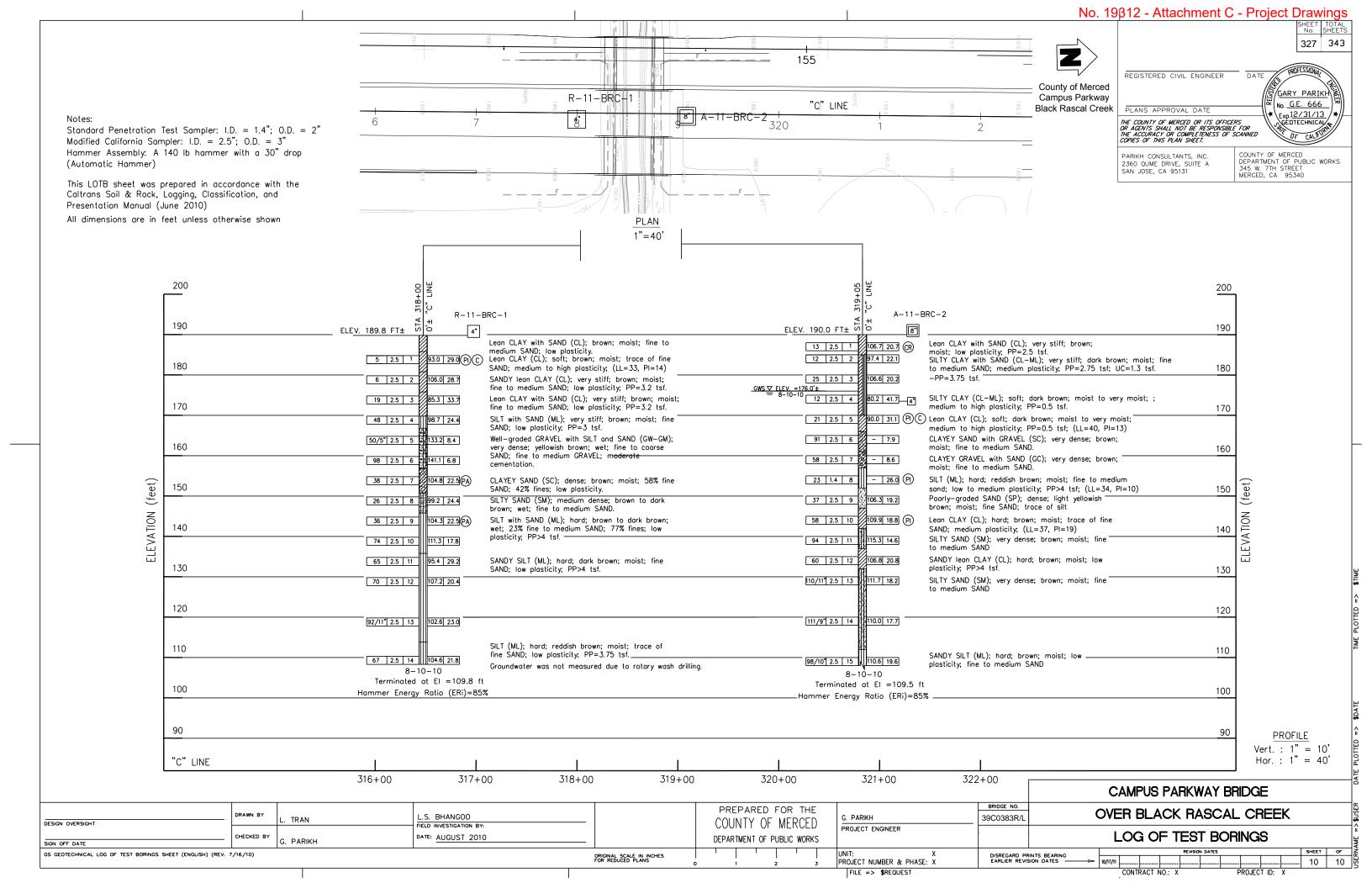
ORIGINAL SCALE IN INCHES FOR REDUCED PLANS

G. PARIKH 39C0383R/L PROJECT ENGINEER

DISREGARD PRINTS BEARING EARLIER REVISION DATES

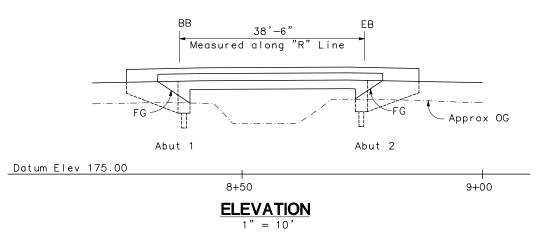
BRIDGE NO.

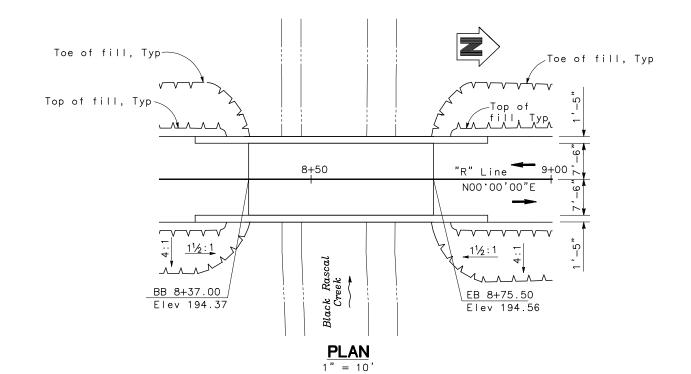
PROJECT ID: X



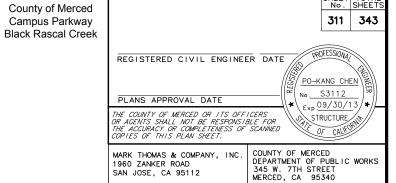


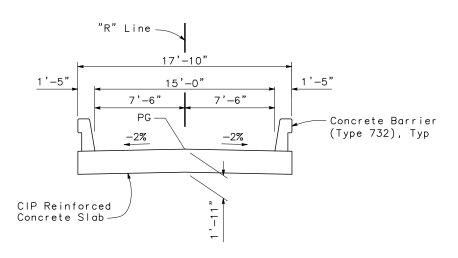
PROFILE GRADE NO SCALE





QUANTITIES





<u>TYPICAL SECTION</u> 1/4" = 1'-0"

INDEX TO PLANS

SHEET No.	<u>TITLE</u>
1	GENERAL PLAN
2	DECK CONTOURS
3	FOUNDATION PLAN
4	ABUTMENT LAYOUT
5	SLAB REINFORCEMENT DETAILS
6	LOG OF TEST BORINGS 1 OF 2
7	LOG OF TEST BORINGS 2 OF 2

<u>NOTES</u>

For "General Notes", see "Deck Contours" sheet.

For "Pile Data Table", see "Foundation Plan" sheet.

For "Hydrologic Summary", see "Foundation Plan" sheet.

												PRIVATE ACCESS BRIDGE		o
DESIGN	V. SHERBY	Z. SIVIGLIA	LRFD DESIGN	LIVE LOADING: HL93 w/ AND PERI	'LOW BOY' NIT DESIGN VEHICLE		EPARED FOR		J. PASSALACQUA		0	VER BLACK RASCAL CREEK	K	Sboyk
DETAILS	G. BOYKO	Z. SIVIGLIA	LAYOUT	J. NETTLETON	Z. SIVIGLIA	COL	UNTY OF M	ERCED	PROJECT ENGINEER	·				-
QUANTIT	IES P. VULLIET	V. SHERBY	SPECIFICATIONS	J. PASSALACQUA	COMPARED P. CHEN	DEPA	rtment of Publi	IC WORKS				GENERAL PLAN		AME
DESIGN GENERAL PLAN SHEET (ENGLISH) (REV.7/16/10) V:\MERCED COUNTY-56-0211B-CAMPUS PKWY PHASE \CADD\STRUCTURE	S\BLACK RASCAL CREEK PA BRIDGE\01 BI	LACK RASCAL CREEK PA_ GP.DWG 12/2/	2011 4:06:00 PM		ORIGINAL SCALE IN INCHES FOR REDUCED PLANS	1	1 2	3	UNIT: PROJECT NUMBER & PHASE:	DISREGARD PRIN' EARLIER REVISIO		REVISION DATES (PRELIMINARY STAGE ONLY) 11/14/70 19/147/11 10/21/11	1 7	L USERN

FILE => 01 Black Rascal Creek PA_ GP.dwg

NTRACT NO.:

PROJECT ID:

ty-56-0211B-Campus Pkwy Phase II\CADD\Struct

DATE

REGISTERED CIVIL ENGINEER

THE COUNTY OF MERCED OR ITS OFFICERS OR AGENTS SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF SCANNED COPIES OF THIS PLAN SHEET.

PLANS APPROVAL DATE

MARK THOMAS & COMPANY, INC. 1960 ZANKER ROAD

Abut 2

CONTRACT NO.:

PROJECT ID:

SAN JOSE, CA 95112

313 338

PO-KANG CHEN No. S3112

\Exp. 09/30/13

STRUCTURE OF CALEOR

COUNTY OF MERCED DEPARTMENT OF PUBLIC WORKS 345 W. 7TH STREET MERCED, CA 95340

BENCH MARKS

County of Merced Campus Parkway Black Rascal Creek

Set 3/4" iron pipe with plastic plug stamped "MTCo Control" at centerline of dirt farm road north of the irrigation Control. Elevation 191.00' (NAVD88)

Benchmark No. 2 (Point No. 4006) Set 3/4" iron pipe with plastic plug stamped "MTCo Control" 12' Southeast of 12" standpipe. Elevation 190.69' (NAVD88)

DATUM

North American Vertical datum of 1988(NAVD88) based on NGS Benchmark D 1420 (PID: HS4524) having an elevation of 182.92'.

LEGEND

Indicates Bottom of Footing Elevation

Indicates Pile (not all piles shown)

HYDROLOGIC SUMMARY

Drainage Area x.x Square Mile

Base Design Overtopping Flood Flood Flood Frequency (Years) Discharge (Cubic Foot per Sec) Water Surface (Elevation at Bridge) Flood plain data are based upon information available when the

plans were prepared and are shown to meet federal requirements. The accuracy of said information is not warranted by the State and interested or affected parties should make their own investigation



Abut 1

FILE => 03 Black Rascal Creek PA_FP.dwg

Structural Concrete, Bridge

CONCRETE TYPE LIMITS

NO SCALE

GEOTE										PRIVATE ACCESS BRIDGE	=	
	SCALE: AS SHOWN VERT.DATUM NAV	/D 88 HORZ.DATUM NAD 83	DESIGN	BY V. SHERBY	CHECKED Z. SIVIGLIA	PREPARED FOR THE			\cap	VER BLACK RASCAL CRE	:FK	ŀ
ı	PHOTOGRAMMETRY AS OF:	ALIGNMENT TIES	DETAILS	BY G. BOYKO	CHECKED	COUNTY OF MERCED	J. PASSALACQUA	L		VEH BEACK HAOOAL OHE	<u></u>	
ı	SURVEYED BY	DRAFTED BY	2225	 	Z. SIVIGLIA		PROJECT ENGINEER			FOUNDATION PLAN		
L	FIELD CHECKED BY	CHECKED BY	QUANTITIES	P. VULLIET	CHECKED	DEPARTMENT OF PUBLIC WORKS				I CONDATION FLAN		:
П	OUNDATION PLAN SHEET (ENGLISH) (REV. 7/16/10)				ORIGINAL SCALE IN INCHES	1 1 1 1	UNIT:	DISREGARD PRINTS	REARING	REVISION DATES (PRELIMINARY STAGE ONLY)	SHEET	OF
ı	V:\MERCED COUNTY-56-0211B-CAMPUS PKWY PHASE II\CADD\STRUCTURES\BLACK RASCAL CREEK PA BRII	DGE\03 BLACK RASCAL CREEK PA_FP.DWG 12/2/2011	:06:40 PM		ORIGINAL SCALE IN INCHES FOR REDUCED PLANS	0 1 2 3	PROJECT NUMBER & PHASE:	DISREGARD PRINTS EARLIER REVISION D	ATES -	11/12/10 10/20/11 11/21/11	3	7

County of Merced Campus Parkway 316 343

REGISTERED CIVIL ENGINEER GARY PARIKH No. G.E. 666 PLANS APPROVAL DATE Exp.12/31/13 THE COUNTY OF MERCED OR ITS OFFICERS OR AGENTS SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF SCANNED COPIES OF THIS PLAN SHEET.

COUNTY OF MERCED PARIKH CONSULTANTS, INC. 2360 QUME DRIVE, SUITE A DEPARTMENT OF PUBLIC WORKS 345 W. 7TH STREET MERCED, CA 95340 SAN JOSE, CA 95131

Indented by thumbnoil with difficulty

	CONSISTENCY OF COHESIVE SOILS							
escription	Unconfined Compressive Strength (tsf)	Pocket Penetrometer Measurement (tsf)	Torvane Measurement (tsf)	Field Approximation				
ry Soft	< 0.25	< 0.25	< 0.12	Easily penetrated several inches by fist				
oft	0.25 to 0.50	0.25 to 0.50	0.12 to 0.25	Easily penetrated several inches by thumb				
edium Stiff	0.50 to 1.0	0.50 to 1.0	0.25 to 0.50	Penetrated several inches by thumb with moderate effort				
iff	1 to 2	1 to 2	0.50 to 1.0	Readily indented by thumb but penetrated only with great effort				
ry Stiff	2 to 4	2 to 4	1.0 to 2.0	Readily indented by thumbnail				

> 2.0

	PLASTICITY OF FINE-GRAINED SOILS					
Description	Criteria					
Nonplastic	A 1/8-inch thread cannot be rolled at any water content.					
Low	The thread can barely be rolled and the lump cannot be formed when drier than the plastic limit.					
Medium	The thread is easy to roll and not much time is required to reach the plastic limit. The thread cannot be rerolled after reaching the plastic limit. The lump crumbles when driet than the plastic limit.					
High	It takes considerable time rolling and kneading to reach the plastic limit. The thread can be rerolled several times after reaching the plastic limit. The lump can be formed without crumbling when drier than the plastic limit.					

	BOREHOLE IDENTIFICATION					
Symbol	Hole Type	Description				
Size	A	Auger Boring				
Size	R P	Rotary drilled boring Rotary percussion boring (air)				
egy,	R	Rotary drilled diamond core				
HD Hand driven (1-inch soil tube)						
•	D	Dynamic Cone Penetration Boring				
	СРТ	Cone Penetration Test (ASTM D 5778-95)				
	0	Other				

PRIVATE ACCESS BRIDGE

OVER BLACK RASCAL CREEK

LOG OF TEST BORINGS

Black Rascal Creek

> 4.0

> 4.0

FIELD AND LABORATORY TESTING C Consolidation (ASTM D 2435)

CU Consolidated Undrained Triaxial (ASTM D 4767)

P Permeability (CTM 220)

(PM) Pressure Meter PP Pocket Penetrometer (R) R-Value (CTM 301)

(TV) Pocket Torvane

Ground water surface

taken

GWS Elev.

Date measured

Description of

(S) — Sample

(s) **-**

Boring Date

Terminated at Flev

HAND BORING

Refusal

.S. BHANGOO

FIELD INVESTIGATION BY

Pulled Pipe

DATE:

(ASTM D 2938) UU Unconsolidated Undrained Triaxial (ASTM D 2850) UW) Unit Weight (ASTM D 4767) (VS) Vane Shear (AASHTO T 223)

(DS) Direct Shear (ASTM D 3080)

(EI) Expansion Index (ASTM D 4829)

M Moisture Content (ASTM D 2216)

OC) Organic Content-% (ASTM D 2974)

(PA) Particle Size Analysis (ASTM D 422)

Pl Plosticity Index (AASHTO T 90) Liquid Limit (AASHTO T 89)

(PL) Point Load Index (ASTM D 5731)

SE) Sand Equivalent (CTM 217) SG Specific Gravity (AASHTO T 100) (SL) Shrinkage Limit (ASTM D 427) (SW) Swell Potential (ASTM D 4546)

> Unconfined Compression-Soil (ASTM D 2166) Unconfined Compression-Rock

> > ORIGINAL SCALE IN INCHES FOR REDUCED PLANS

(CL) Collapse Potential (ASTM D 5333) (CP) Compaction Curve (CTM 216) CR Corrosivity Testing (CTM 643, CTM 422, CTM 417)

MOISTURE					
Description	Criteria				
Dry	Absence of moisture, dusty, dry to the touch				
Moist	Damp but no visible water				
Wet	Visible free water, usually soil is				

PE	PERCENT OR PROPORTION OF SOILS							
Description	Criteria							
Trace	Particles are present but estimated to be less than 5%							
Few	5 to 10%							
Little	15 to 25%							
Some	30 to 45%							
Mostly	50 to 100%							

	PARTICLE SIZE						
Des	cription	Size					
Boulder		> 12"					
Cobble		3" to 12"					
Gravel	Coarse	3/4" to 3"					
Gravei	Fine	No. 4 to 3/4"					
	Coarse	No. 10 to No. 4					
Sand	Medium	No. 40 to No. 10					
	Fine	No. 200 to No. 40					

CEMENTATION							
Description	Criteria						
Weak	Crumbles or breaks with handling or little finger pressure.						
Moderate	Crumbles or breaks with considerable finger pressure.						
Strong	Will not crumble or break with finger pressure.						

APPARENT DENSITY OF COHESIONLESS SOILS							
Description	SPT N ₆₀ (Blows / 12 inches)						
Very loose	0 - 4						
Loose	5 - 10						
Medium Dense	11 – 30						
Dense	31 – 50						
Very Dense	> 50						

MOISTURE							
Description Criteria							
Dry	Absence of moisture, dusty, dry to the touch						
Moist	Damp but no visible water						
Wet	Visible free water, usually soil is below water table						

PARTICLE SIZE								
De	scription	Size						
Boulder		> 12"						
Cobble		3" to 12"						
Gravel	Coarse	3/4" to 3"						
Gruver	Fine	No. 4 to 3/4"						
	Coarse	No. 10 to No. 4						
Sand	Medium	No. 40 to No. 10						
	Fine	No. 200 to No. 40						

CEMENTATION								
Description Criteria								
Weak	Crumbles or breaks with handling or little finger pressure.							
Moderate	Crumbles or breaks with considerable finger pressure.							
Strong	Will not crumble or break with finger pressure.							

GM GM GM GM GM GM GM GM GM GM GM	SILTY GRAVEL SILTY GRAVEL with SAND CLAYEY GRAVEL CLAYEY GRAVEL with SAND		OL	ORGANIC Iean CLAY ORGANIC Iean CLAY with SAND ORGANIC Iean CLAY with GRAVEL SANDY ORGANIC Iean CLAY SANDY ORGANIC Iean CLAY with GRAVEL GRAVELLY ORGANIC Iean CLAY GRAVELLY ORGANIC Iean CLAY GRAVELLY ORGANIC Iean CLAY
GC-GM	SILTY, CLAYEY GRAVEL SILTY, CLAYEY GRAVEL with SAND		OL	ORGANIC SILT ORGANIC SILT with SAND ORGANIC SILT with SAND ORGANIC SILT with GRAVEL SANDY ORGANIC SILT
SW	Well-groded SAND Well-groded SAND with GRAVEL	$ \rangle\rangle\rangle$	OL	SANDY ORGANIC SILT with GRAVEL GRAVELLY ORGANIC SILT GRAVELLY ORGANIC SILT with SAND
SP	Poorly graded SAND Poorly graded SAND with GRAVEL		СН	Fot CLAY Fot CLAY with SAND Fot CLAY with GRAVEL SANDY for CLAY
SW-SM	Well-graded SAND with SILT Well-graded SAND with SILT and GRAVEL		СП	SANDY fot CLAY SANDY fot CLAY with GRAVEL GRAVELLY fot CLAY GRAVELLY fot CLAY GRAVELLY fot CLAY with SAND
SW-SC	Well-graded SAND with CLAY (or SILTY CLAY) Well-graded SAND with CLAY and GRAVEL (or SILTY CLAY and GRAVEL)		1411	Elostic SILT Elostic SILT with SAND Elostic SILT with GRAVEL
SP-SM	Poorly graded SAND with SILT Poorly graded SAND with SILT and GRAVEL		МН	SANDY elastic SILT SANDY elastic SILT with GRAVEL GRAVELLY elastic SILT GRAVELLY elastic SILT with SAND
SP-SC	Poorly graded SAND with CLAY (or SILTY CLAY) Poorly graded SAND with CLAY and CRAYEL (or SILTY CLAY and GRAYEL)			ORGANIC fot CLAY ORGANIC fot CLAY with SAND ORGANIC fot CLAY with GRAVEL
SM	SILTY SAND SILTY SAND with GRAVEL		ОН	SANDY ORGANIC fot CLAY SANDY ORGANIC fot CLAY with GRAVEL GRAVELLY ORGANIC fot CLAY GRAVELLY ORGANIC fot CLAY with SAND
sc	CLAYEY SAND CLAYEY SAND with GRAVEL			ORGANIC elastic SILT ORGANIC elastic SILT with SAND ORGANIC elastic SILT with GRAVEL
SC-SM	SILTY, CLAYEY SAND SILTY, CLAYEY SAND with GRAVEL		ОН	SANDY ORGANIC elostic SILT SANDY ORGANIC elostic SILT with GRAVEL GRAVELLY ORGANIC elostic SILT GRAVELLY ORGANIC elostic SILT with SAND
자 자 A 8 자 자 B1 <u>개 연</u> 3	PEAT			ORGANIC SOIL ORGANIC SOIL with SAND ORGANIC SOIL with GRAVEL
	COBBLES COBBLES and BOULDERS BOULDERS		OL/OH	SANDY ORGANIC SOIL SANDY ORGANIC SOIL with GRAVEL GRAVELLY ORGANIC SOIL GRAVELLY ORGANIC SOIL with SAND
Casing driven Sample I Size of Sampler (in.)	- Dry density		a Tooto	Top Hole EI. Blows per 12" (Using 28 lb hand hammer with a 12" Hole I.D. Gro Sur
	16 1.4 1 102 19 (M)(UV) - Fi	u. Lui	, 16363	hammer with a 12" GWS EI

TRAN

G. PARIKH

-Moisture (%)

Estimated material change

Material change

-Soil/Rock boundary

REFERENCE: CALTRANS SOIL & ROCK LOGGING, CLASSIFICATION, AND PRESENTATION MANUAL (JUNE 2007)

CL

CL-ML

Graphic/Symbol

Group Names

Lean CLAY Lean CLAY with SAND Lean CLAY with GRAVEL

SILT SILT with SAND SILT with GRAVEL

ORGANIC lean CLAY

SANDY SILT SANDY SILT with GRAVEL

GRAVELLY SILT GRAVELLY SILT with SAND

SANDY lean CLAY
SANDY lean CLAY with GRAVEL
GRAVELLY lean CLAY
GRAVELY
GRAV

SILTY CLAY
SILTY CLAY with SAND
SILTY CLAY with GRAVEL
SANDY SILTY CLAY
SANDY SILTY CLAY
SANDY SILTY CLAY
GRAVELLY SILTY CLAY
GRAVELLY SILTY CLAY
GRAVELLY SILTY CLAY
WITH SAND

GROUP SYMBOLS AND NAMES

Group Names

Well-graded GRAVEL

Poorly graded GRAVEL

Well-graded GRAVEL with SAND

Poorly graded GRAVEL with SAND

Well-graded GRAVEL with SILT and SAND

Well-graded GRAVEL with CLAY and SAND or SILTY CLAY and SAND

Poorly graded GRAVEL with SILT and SAND

Well-graded GRAVEL with SILT

Well-graded GRAVEL with CLAY (or SILTY CLAY)

Poorly graded GRAVEL with SILT

Poorly graded GRAVEL with CLAY

Poorly graded GRAVEL with CLAY and SAND (or SILTY CLAY and SAND)

Graphic/Symbol

GW

GP

GP-GM

4

. . .

8 4

 $P = push sample, GWS_{\Lambda\Lambda}$

Date measured

GS GEOTECHNICAL LOG OF TEST BORINGS SHEET (ENGLISH) (REV. 7/16/10)

Borina Date

Terminated at Elev

Hammer Energy Ratio (ER); = %

ROTARY BORING

(per ASTM 1586-99),

SPT N-Value

or as noted

DESIGN OVERSIGHT

SIGN OFF DATE

Hole I.D. Top Hole El. No count recorded __ GWS Elev. Pushed Driving rate in seconds per 12" (using a Stanley MB 156 percussion hammer and a 2.2" cone, or as noted) 100 Boring Date

Pressure measured along sleeve friction element (34,88 in area) divided by pressure measured

DYNAMIC CONE PENETRATION BORING

CONE PENETRATION TEST (CPT) SOUNDING

Boring Date

Hole I.D.

Pressure measured 2

DISREGARD PRINTS BEARING

on tip element

20 30

Tip Bearing (MPa)

(2.33 in ²area)

BRIDGE NO.

Friction Ratio (%)

Top Hole El.

PREPARED FOR THE COUNTY OF MERCED DEPARTMENT OF PUBLIC WORKS

PROJECT ENGINEER PROJECT NUMBER & PHASE: X

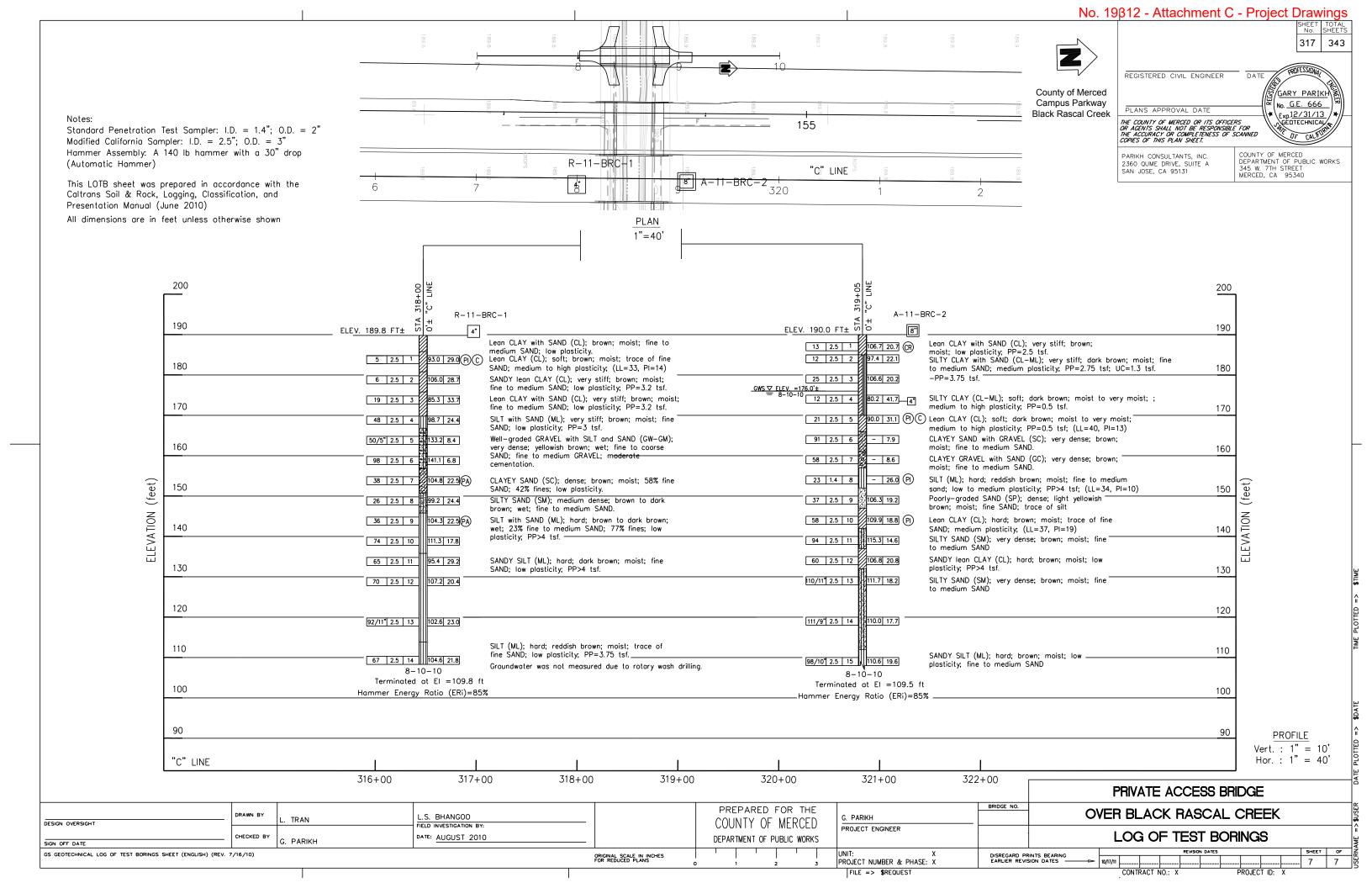
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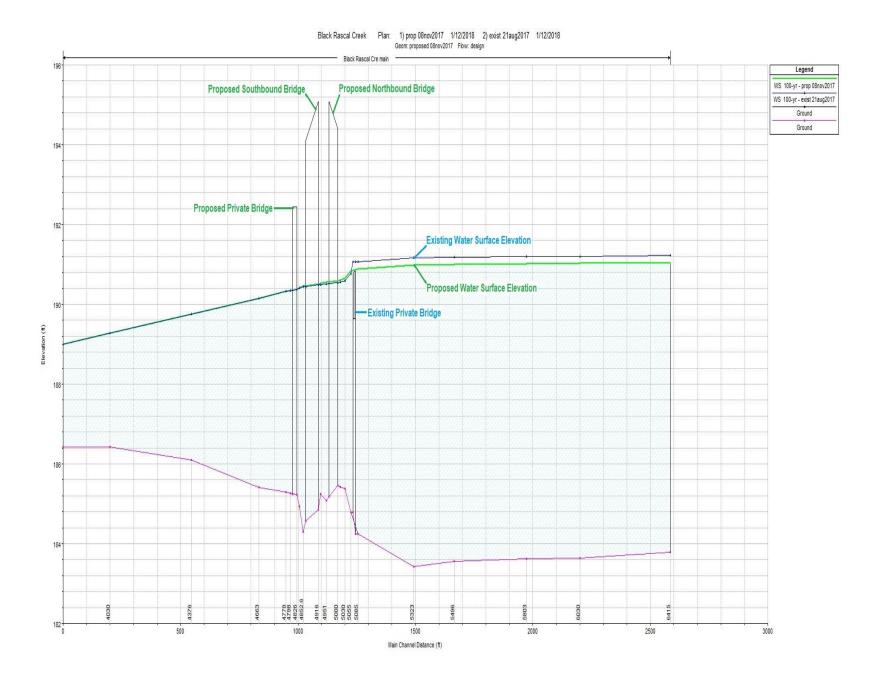
G. PARIKH

CONTRACT NO.: X

PROJECT ID: X

6 7





No. 19312 - Attachment D - Hydraulic Profile Information

			HEC-R/	AS River:	black Rasca	l Cre Rea	cn: main	Profile: 100	-yr				Reload Da
Reach	River Sta	Profile	Plan	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
				(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	
nain	6030	100-yr	prop 08nov2017	320,00	183.64	191.03		191.05	0.000072	1.06	473.78	251.33	0.08
nain	6030	100-yr	exist 21aug2017	320.00	183.64	191.20		191.22	0.000061	0.99	516.95	252.59	0.07
nin.	5803	100-yr	prop 08nov2017	320.00	183.62	191.02		101.02	0.000049	0.82	546.00	202.59	0.07
nain	- CONTRACTOR	-						1.00	0.000048		-		2000
nain	5803	100-yr	exist 21aug2017	320.00	183.62	191.20		191.20	0.000041	0.77	581.09	204.20	0.06
nain	5496	100-yr	prop 08nov2017	320,00	183.55	191.00	185.55	191.02	0.000056	0.89	365.14	96.80	0.07
nain	5496	100-yr	exist 21aug2017	320.00	183.55	191.18	185.55	191.19	0.000051	0.85	385.43	207.94	0.07
							B1240-110						
nain	5323	100-yr	prop 08nov2017	320,00	183.42	190.99	185.78	191.00		0.89	358.06	1585.00	0.08
nain	5323	100-yr	exist 21aug2017	320.00	183.42	191.17	185.78	191.18	0.000076	0.85	376.28	1864.11	0.08
nain	5085	100-yr	prop 08nov2017	320.00	184.25	190.88	187.38	190.96	0.000419	2.18	146.70	33.41	0.18
nain	5085	100-yr	exist 21aug2017	320.00	184.25	191.07	187.38		0.000368	2.09	153.79	63.08	0.17
			4										
main	5055	100-yr	prop 08nov2017	320,00	184.78	190.83	187.91	190.94	0.000868	2.62	122.00	37.78	0.26
main	5055	100-yr	exist 21aug2017	320.00	184.78	190.78	187.91	190.89	0.000893	2.66	120.10	36.99	0.26
main	5030	100-yr	prop 09ppy2017	320.00	185.38	190.65	188.61	190.89	0.001872	3.95	82.34	32.61	0.37
nain nain	5030	100-yr	prop 08nov2017 exist 21aug2017	320.00	185.38	190.65	188.61		0.001872	4.02	80.52	30.61	0.37
iliali i	3030	100 yı	CXISC ZIGGGZ017	320,00	100.00	130.33	100.01	130.01	0.001370	1.02	00.52	50.01	0.50
main	5010	100-yr	prop 08nov2017	320.00	185.43	190.61	188.59	190.85	0.001900	3.95	82.65	35.91	0.37
main	5010	100-yr	exist 21aug2017	320,00	185.43	190.55	188.59	190.80	0.002006	4.02	81.06	33.83	0.38
							·-				195		
main	5000			Bridge									
nain	5000	100-yr	exist 21aug2017	320.00	185.46	190.55	188.56	190.78	0.001877	3.89	96.11	109.97	0.37
main	4961	100-yr	exist 21aug2017	320.00	185.18	190.52	188.08	190.70	0.001357	3.49	102.21	505.36	0.32
main	4951	100-yr	prop 08nov2017	320,00	185.09	190.55	187.99	190.73	100000000000000000000000000000000000000	3.43	95.39	654.21	0.31
main	4951	100-yr	exist 21aug2017	320.00	185.09	190.51	187.99	190.68	0.001247	3.38	109.01	592.33	0.30
main	4926	100-yr	prop 08nov2017	320.00	185.24	190.52	188.04	190.70	0.001245	3.40	96.40	749.85	0.30
main	4926	100-yr	exist 21aug2017	320.00	185.24	190.49	188.04		0.001163	3.28	122.67	707.44	0.29
							i i				-5		
main	4916		-	Bridge		-							
main	4916	100-yr	exist 21aug2017	320.00	184.86	190.49	187.81	190.64	0.001008	3.13	128.14	721.00	0.27
in	4963.6	100 255	eviet 21aue 2017	220.00	104 57	100.44	107 67	100 50	0.000045	2.05	121.05	055.50	0.27
nain	4862.6	100-yr	exist 21aug2017	320.00	184.57	190.44	187.67	190.58	0.000945	3.05	131.95	956.60	0.27
main	4852.6	100-yr	prop 08nov2017	320.00	184.29	190.46	187.54	190.61	0.000959	3.11	104.57	1019.68	0.27
main	4852.6	100-yr	exist 21aug2017	320,00	184.29	190.44	187.53		0.000874	2.96	136.09	985.03	0.26
			-										
main	4836	100-yr	prop 08nov2017	320.00	184.93	190.41	187.94		0.001226	3.37	96.47	941.74	0.30
main	4836	100-yr	exist 21aug2017	320,00	184.93	190.41	187.94	190.55	0.001089	3.18	128.93	940.20	0.28
nain	4826			Bridge		1-							
nain	4826	100-yr	exist 21aug2017	320.00	185.23	190.38	188.12	190.54	0.001232	3.29	124.44	935.76	0.30
i i Gill i	1020	200 /1	CXISC E1ddg2017	320,00	100120	130.30	100.12	130.31	0.001252	5,25	12 11 11	333.70	0.50
main	4808	100-yr	exist 21aug2017	320.00	185.25	190.36	188.10	190.52	0.001231	3.28	124,40	989.58	0.30
	4700	100	00 00 00	200 00	405.55	100.51	100.00	100 5-	0.001701		65.71	000.01	
main	4798	100-yr	prop 08nov2017	320.00	185.26	190.34	188.09	-	0.001381	3.46	93.74	992.84	0.32
main	4798	100-yr	exist 21aug2017	320.00	185.26	190.35	188.09	190.51	0.001218	3.26	124.70	1016.19	0.30
main	4778	100-yr	prop 08nov2017	320.00	185.29	190.33	188.07	190 48	0.001169	3.21	128.00	969.47	0.30
nain	4778	100-yr	exist 21aug2017	320.00	185.29	190.33	188.07	-	0.001169	3.21	128.00	969.47	0.30
4	1		1		230.03	230.30		230, 10			-30,00	- 241 11	•